Riverstone East Precinct Transport Study

Department of Planning & Environment

Post Exhibition Report



Contents

			Page
1	Introd	luction	1
	1.1	Study Background	1
	1.2	Study Objectives	1
	1.3	Report Structure	2
2	Summ	nary of Proposed Development	3
	2.1	North West Growth Centre Precinct	3
	2.2	Riverstone East Precinct	3
	2.3	Draft Indicative Layout Plan	3
3	Trans	port Conditions	5
	3.1	Existing Travel Patterns	5
	3.2	Road Network	6
	3.3	Crossings of the Richmond Rail Line	10
	3.4	Traffic Volumes	12
	3.5	Heavy Vehicles	14
	3.6	Rail Services	15
	3.7	Bus Services	18
	3.8	Walking and Cycling	20
4	Precir	nct Planning Principles	22
	4.1	Transport Planning Objectives	22
	4.2	Road Classification and Capacity	22
	4.3	Road Cross Sections	23
	4.4	Heavy Vehicles	23
	4.5	Public Transport Provision	24
	4.6	Walking and Cycling	25
	4.7	Parking	28
5	Road	Network Assessment	29
	5.1	Assessment Criteria	29
	5.2	Strategic Network Modelling	31
	5.3	Traffic Generation and Trip Containment	32
	5.4	Future Traffic Volumes	32
	5.5	Traffic Distribution	35
	5.6	Road Network Hierarchy	35
	5.7	Intersection Capacity Analysis	41
	5.8	Heavy Vehicle Movements	49

6	Public	c Transport, Walking and Cycling	50
	6.1	Bus Network	50
	6.2	Rail Services	57
	6.3	Pedestrian and Cycling Network	58
7	Sumn	nary and Conclusions	61

Tables

- Table 1 Existing services along the Richmond rail line
- Table 2 Functional classification of roads
- Table 3 GCC standard road types and road corridor widths
- Table 4 Parking Rates Residential Uses
- Table 5 Parking Rates Non-Residential Uses
- Table 6 Mid-Block capacities of urban roads
- Table 7 Roadway levels of service definition
- Table 8 Intersection level of service
- Table 9 Traffic generation rates
- Table 10 2036 Traffic Volumes Summary Riverstone East
- Table 11 Sensitivity analysis Guntawong Road closure
- Table 12 Recommended Intersection Controls
- Table 13 Warrants for Traffic Signals at Intersections
- Table 14 Traffic Modelling Results

Figures

- Figure 1 North West Growth Centre
- Figure 2 North West Growth Centre Structure Plan
- Figure 3 Riverstone East precinct aerial
- Figure 4 Draft Riverstone East Stage 1 ILP
- Figure 5 Existing travel patterns, North West Growth Centre
- Figure 6 Windsor Road
- Figure 7 Schofields Road upgrade
- Figure 8 Garfield Road East
- Figure 9 Existing Road Network
- Figure 10 Proposed road network strategy for the NWGC
- Figure 11 Traffic survey locations
- Figure 12 Existing traffic volumes
- Figure 13 Existing heavy vehicle proportions
- Figure 14 Cudgegong Road station layout
- Figure 15 North West Rail Link

- Figure 16 Existing bus services
- Figure 17 Existing cycling network
- Figure 18 Separation of bicycles and motor vehicles
- Figure 19 Transport for NSW residual lands
- Figure 20 Traffic implications of Guntawong Road closure
- Figure 21 Riverstone East Road Hierarchy
- Figure 22 Stage 1 and Stage 2 Road Network
- Figure 23 Intersection configurations indicative 2036 layout
- Figure 24 Recommended traffic facilities
- Figure 25 Proposed bus network Riverstone East
- Figure 26 North West Sector bus servicing plan
- Figure 27 North West Growth Centre Bus Catchment Area
- Figure 28 Sydney's Future Bus Network
- Figure 29 Marsden Park transport corridor
- Figure 30 Proposed pedestrian / cycle network

Appendices

Appendix A

Typical Road Cross Sections

Appendix B

Traffic Model Outputs

1 Introduction

1.1 Study Background

The Riverstone East Precinct is a major future urban release area which is proposed to be developed as part of Sydney's North West Growth Centre (NWGC). Arup has been appointed by Department of Planning and Environment (DP&E) to undertake a transport assessment, one of a set of specialist studies that will inform the development of the draft Indicative Layout Plan (ILP) for the Riverstone East Precinct.

1.2 Study Objectives

The purpose of this study is to provide an assessment of the Riverstone East precinct by all modes of transport including walking, cycling, public transport and passenger vehicles. This will need to consider the development of adjacent precincts within the North West Growth Centre, as well as upcoming infrastructure works such as the North West Rail Link. The transport assessment will identify suitable facilities for Riverstone East employees and residents to walk, cycle, access to public transport or use private cars.

Specific objectives of the study will be to:

- provide a strategic overview of the existing and future transport network in the North West Growth Centre;
- assess and test the transport impacts of the proposed development of the study area as reflected in the Indicative Layout Plan (ILP), taking into consideration potential development staging;
- recommend infrastructure upgrades and other measures to address those impacts within the vicinity of Riverstone East;
- make recommendations for suitable land uses that will interface with the future NWRL stabling yard in the Riverstone East Precinct;
- prepare an agreed implementation framework, in negotiation with the NSW Government transport agencies, Blacktown Council, and DP&E, for the key infrastructure components;
- ensure all modes of transport, including private vehicle, public transport (bus and rail), walking and cycling are considered in the planning and development of each Precinct.

1.3 Report Structure

This transport assessment for the Riverstone East precinct is structured as follows:

• Section 1: Introduction

This section

• Section 2: Summary of Proposed Development

Overview of the future development of the North West Growth Centre and Riverstone East precinct

• Section 3: Transport Conditions

Summary of existing transport services in the North West Growth Centre, including roads, public transport, walking and cycling

• Section 4: Precinct Planning Principles

Identification of key criteria and objectives when planning for the development of the Riverstone East precinct

• Section 5: Road Network Assessment

Analysis of future road network conditions following the development of the Riverstone East precinct, including an analysis of intersection capacities

• Section 6: Public Transport, Walking and Cycling

Assessment of the future transport provision for non-car modes of travel, supporting the project objective of reducing car dependency for residents and employees of the Riverstone East precinct.

• Section 7: Summary and Conclusions

Summary of the key findings of this document

2 Summary of Proposed Development

2.1 North West Growth Centre Precinct

The North West Growth Centre (NWGC), comprising 16 precincts, is approximately 10,000 hectares and will contain about 70,000 new dwellings for 200,000 people. 11 of these 16 precincts have been rezoned for development, those being:

- North Kellyville
- Alex Avenue
- Riverstone
- Riverstone West
- Colebee
- Area 20

- Marsden Park Industrial
- Schofields
- Box Hill
- Box Hill Industrial
- Marsden Park

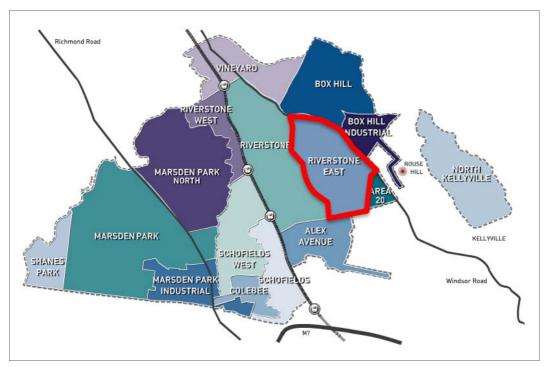


Figure 1 North West Growth Centre

The NWGC spans three local government areas (LGA) – Blacktown, Hawkesbury and The Hills Shire. The NWGC is undergoing a streamlined planning process to enable land to be rezoned in a shorter period. Figure 2 on the following page illustrates the existing North West Growth Centre Structure Plan (edition 3). It should be noted the alignment of the North West Rail Link indicated on this plan differs from the preferred route along Schofields Road.

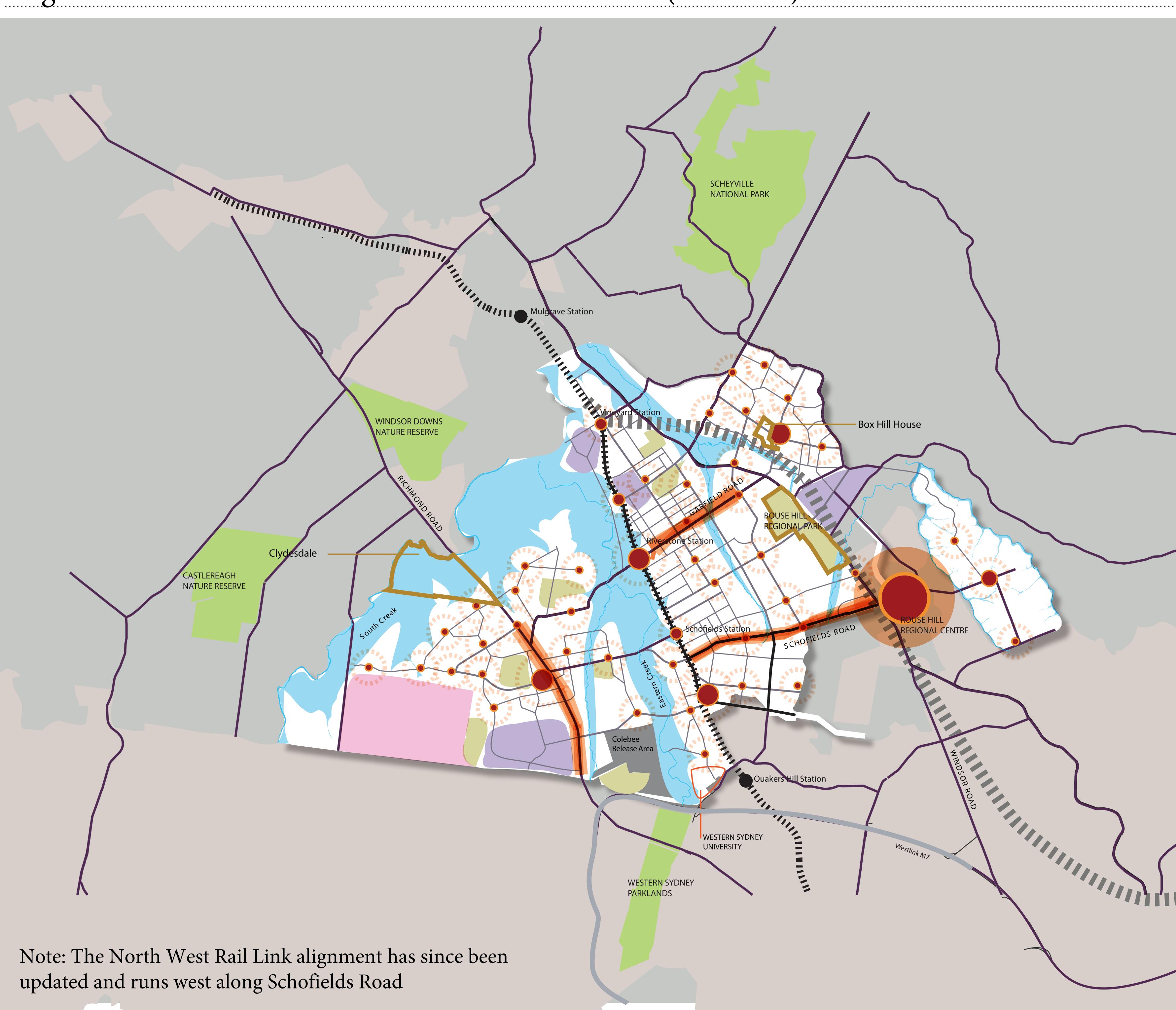


Figure 2: North West Growth Centre Structure Plan (Edition 3)



DISCLAIMER: The Structure Plan is an indicative



MAJOR CENTRES



TOWN & VILLAGE CENTRES



WALKABLE NEIGBOURHOODS

A key element of the urban form is the walkable neighbourhood which is the area within a 400 metre radius from a local shop (or group of shops) or from another community focus (eg a community centre with a bus stop). Walkable neighbourhoods are clustered around mixed use main street retail centres shown as red on the plan. These centres include housing, community facilities, shops for daily convenience etc.



NORTH WEST RAIL LINK

As part of Government's commitment to deliver better, sustainable new communities it announced that a new rail line would be constructed to serve the new communities. As part of the early planning for this project, the Government will investigate route alignment options.

COLEBEE RELEASE AREA

The Colebee Release Area Local Environmental Plan has been finalised.

FLOOD LIABLE LAND & MAJOR CREEKS

CONSERVATION / OPEN SPACE



HERITAGE CURTILAGES

INDUSTRIAL / EMPLOYMENT LANDS

TRANSITIONAL LANDS

MIXED USE EMPLOYMENT CORRIDORS





2.2 Riverstone East Precinct

The Riverstone East precinct is located in the eastern, central portion of the North West Growth Centre, wholly within the Blacktown Local Government Area (LGA). It is currently zoned General Rural under the Blacktown Local Environmental Plan 1988 with certain land at the northern end of the precinct zoned for residential purposes.

The precinct is bounded by Windsor Road to the east and north, Schofields Road to the south and First Ponds Creek to the west. In the wider area, the precinct is located approximately 50km from the Sydney CBD, 9km northwest of Blacktown and 5km west of Rouse Hill.

It is comprised of 656 hectares in total; it is immediately surrounded by Alex Avenue to the south west, Area 20 to the south east, Box Hill and Box Hill Industrial to the north east, and Riverstone to the direct west.



Figure 3 Riverstone East precinct aerial

2.3 Draft Indicative Layout Plan

The draft indicative layout plan (ILP) developed for Stage 1 and Stage 2 of the Riverstone East precinct is presented in Figure 4 on the following page.

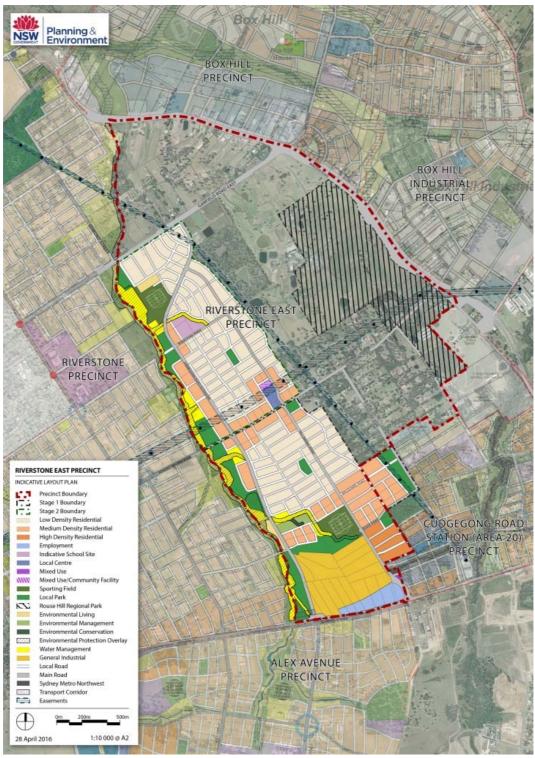


Figure 4 Draft Riverstone East Stage 1 and 2 ILP

3 Transport Conditions

3.1 Existing Travel Patterns

Existing travel characteristics of residents in the North West Growth Centre area have been identified based on 2011 Journey to Work Census data¹ and 2012 Household Travel Survey Information². The existing mode share of residents is illustrated in Figure 5.

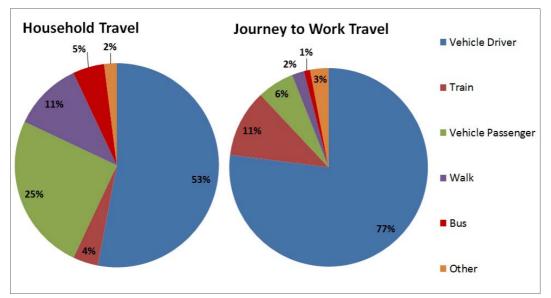


Figure 5 Existing travel patterns, North West Growth Centre

Source: Bureau of Transport Statistics, 2014

Other includes motorcycle, bicycle and mode not stated

The results indicate the significant majority of journey, irrespective of purpose, are made by private vehicle. Train travel accounts for 11% of total work trips, however this would be expected to increase following the completion of the North West Rail Link (anticipated for 2019).

Walking and cycling account for low proportion of work related trips, household travel survey data indicates these modes account for a much higher proportion of household trips.

¹ Based on travel zones within the North West Growth Centre

² Based on travel information for residents in the Blacktown LGA

3.2 Road Network

The existing road network supporting the Riverstone East precinct, as well as potential future road infrastructure upgrades, is outlined in Figure 9. Details of key roads serving the precinct are described below.

3.2.1 Windsor Road

Windsor Road forms the primary access route into the precinct, running along the eastern boundary of the site. It is classified as a sub-arterial road, with two traffic lanes in each direction with provision of a third lane for right turn vehicles at certain intersections. East of Commercial Road (at Rouse Hill town centre) Windsor Road widens to three lanes in each direction, reflecting the increasing traffic demands at this location.



Figure 6 Windsor Road

3.2.2 Schofields Road

Schofields Road, at the southern boundary of the site, is currently being upgraded between Windsor Road and Tallawong Road to provide two traffic lanes in both directions. In July 2014 traffic signals were installed at the intersections of both Cudgegong Road and Tallawong Road. Further work along this road corridor is to be carried out in the coming years which will provide for a four lane divided road corridor along the full length of Schofields Road between Windsor Road and Richmond Road.

The upgrade of Schofields Road, to become a 'transit boulevard', will provide connections for pedestrians, cyclists and buses to surrounding land uses. A wide central median will be provided to allow for a six lane corridor in the future should demand necessitate. This will meet the future transport needs of the North West Growth Centre.



Figure 7 Schofields Road upgrade

3.2.3 Garfield Road East

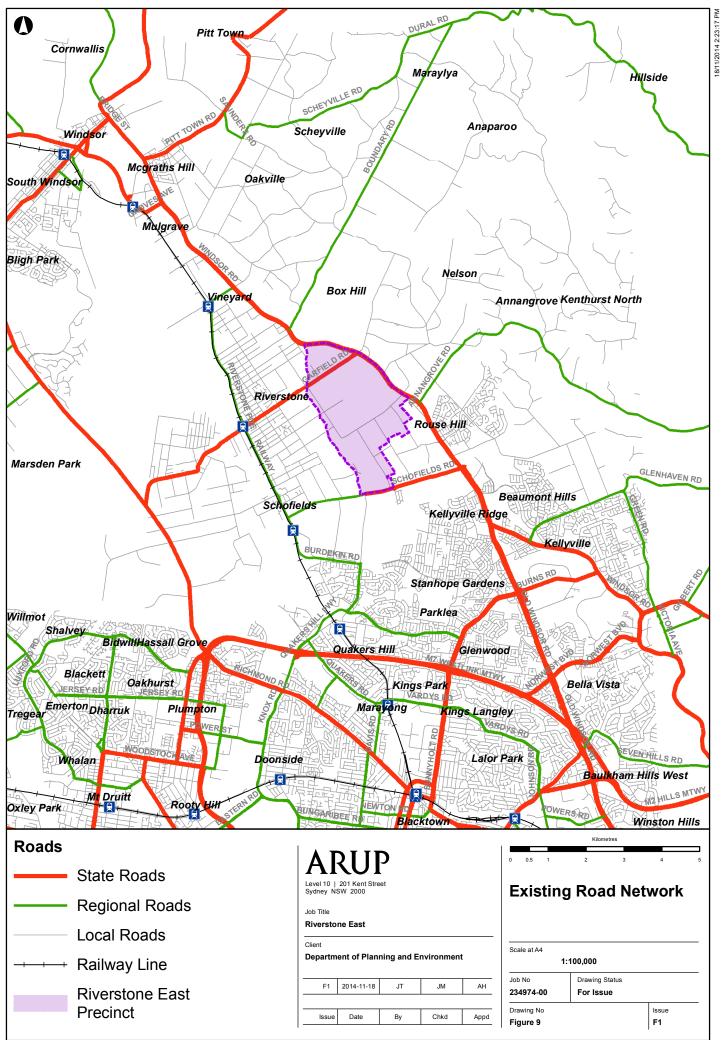
Garfield Road East currently provides the primary east-west road connection through the Riverstone East precinct. It provides a link between Windsor Road and Riverstone Parade (to Riverstone Railway Station), extending west to Richmond Road. It is currently an undivided two lane sealed road, with an 80km/h speed limit.

Garfield Road currently provides the primary east-west road connection through Riverstone. It provides a link between Windsor Road and Riverstone Parade (to Riverstone Railway Station), extending west to Richmond Road. It is currently an undivided two lane sealed road, with a 50km/hr speed limit within the study area. It currently forms an at grade intersection with the Richmond railway line.

Roads and Maritime have identified Garfield Road as a future east-west road corridor providing connectivity between Windsor Road and Garfield Road. This infrastructure would be delivered by the time the North West Growth Centre reaches approximately 75% of its population and employment development.



Figure 8 Garfield Road East



Nglobal.arup.com/australasia/SYD/Projects/234000/234974-00 Riverstone E Vineyard/Work/06 GIS/Projects/Riv E & Vineyard Transport Planning 20141118 JRT.m

3.3 Crossings of the Richmond Rail Line

The RMS has developed a strategy for the provision of grade-separated road crossings across the Richmond rail line. The road network strategy identifies the need for five grade-separated crossings of the Richmond Rail line:

- Burdekin Road, Quakers Hill
- Westminster Street bridge, Schofields (existing with limited capacity)
- Schofields Road, Schofields (in progress as part of the Schofields Road upgrade project)
- Garfield Road, Riverstone (currently a level-crossing of the rail line)
- Bandon Road, Vineyard (currently a level-crossing of the rail line).

A summary of the planned strategy is illustrated in Figure 10 below.

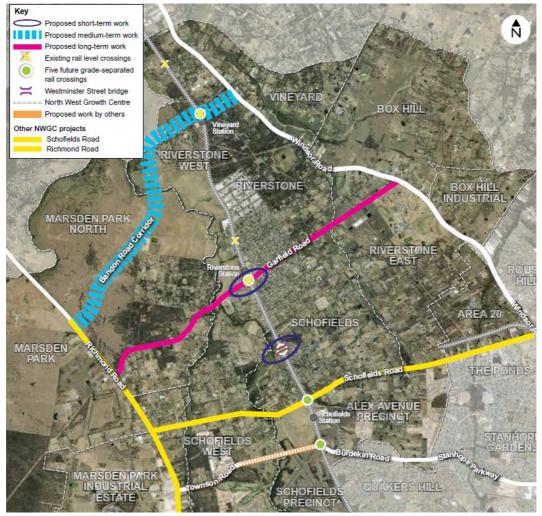


Figure 10 Proposed road network strategy for the NWGC Source: Roads and Maritime Services (2014)

The road network strategy has been divided into short, medium and long terms works as described in detail in the sections below.

3.3.1 Short Term Works

The planned short term works (to be completed within the next four years) will directly influence traffic conditions within the Riverstone town centre. These works include:

- Work with Blacktown City Council to develop local strategies to improve traffic flow within Riverstone town centre to boost capacity and minimise delays, with the existing level crossing retained. The specific location and extent of these works are currently being developed.
- Link Westminster Street with Garfield Road West to provide an alternative route for local traffic away from the Garfield Road level crossing.
- Work with the Department of Planning and Environment and Blacktown City Council to reserve a road corridor along Garfield Road between Richmond Road and Windsor Road for future widening

3.3.2 Medium Term Works

The strategy has recommended an upgrade of Bandon Road to function as a transit boulevard (including an underpass of the Richmond railway line) which would provide a high quality road connection between Windsor Road and Richmond Road. This upgrade would be completed by the time the NWGC reaches approximately 25% of its population and employment development. Based on current projections, this could occur by 2021. Works include the construction of an underpass and creation a new road connection between Richmond Road and Windsor Road. The existing level crossings at Bandon Road and Level Crossing Road would be closed at this time.

3.3.3 Long Term Works

The road network strategy has recommended the construction of a grade separated crossing at Garfield Road, replacing the existing level crossing. This infrastructure would be delivered by the time the NWGC reaches approximately 75% of its population and employment development. Based on current projections, this could occur between 2031 and 2036. Works would also include an upgrade Garfield Road between Richmond Road and Windsor Road.

3.4 Traffic Volumes

Surveys were undertaken in March 2014 to understand the existing level of traffic in the vicinity of the Riverstone East and Vineyard precincts. Intersection counts and seven day automated counts were undertaken at a total of 19 locations in the area as illustrated in Figure 11. These counts were used to calibrate and validate the traffic model developed for this study, further outlined in Section 5.

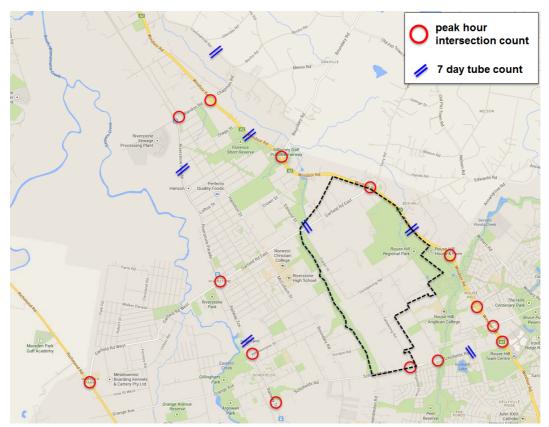


Figure 11 Traffic survey locations

The results of the surveys are shown in Figure 12 on the following page and indicate Windsor Road carries the majority of traffic in the precinct. Traffic volumes on Windsor Road progressively increase from north to south, attributable to the more densely developed areas around Rouse Hill and The Ponds.

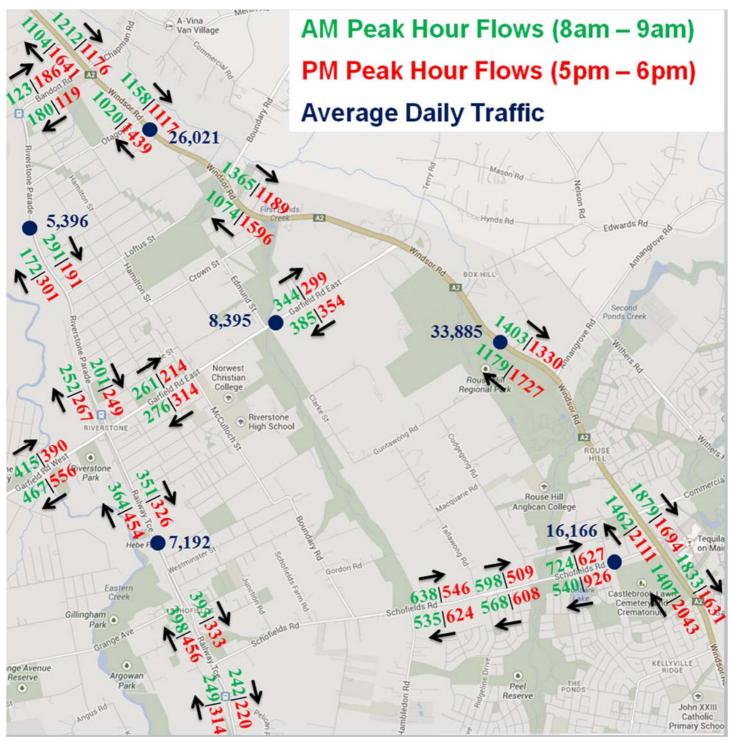


Figure 12 Existing traffic volumes

3.5 Heavy Vehicles

Traffic surveys conducted for this study identified the existing level of heavy vehicles utilising key roads supporting the study area. Key findings from the surveys, with respect to heavy vehicle traffic, were as follows:

- Heavy vehicles currently account for approximately 14% of all vehicles utilising Windsor Road adjacent to the precinct.
- 12% of traffic (approximately 1,100 vehicles per day) were identified as heavy vehicles along Garfield Road (within the Riverstone East precinct). This is a significant number and reflects the current function of Garfield Road as the predominant east-west link between Windsor Road and Richmond Road.
- Heavy vehicles only accounted for 6% of traffic on Schofields Road, lower than other surveyed locations due to the proximity of nearby residential areas.
- On Riverstone Parade to the north-west of the study area, 16% of all vehicles surveyed were identified as heavy vehicles. This reflects the number of light industrials uses in this area and relatively low number of local residents.

The outcomes of the surveys with respect to heavy vehicles are illustrated in Figure 13.

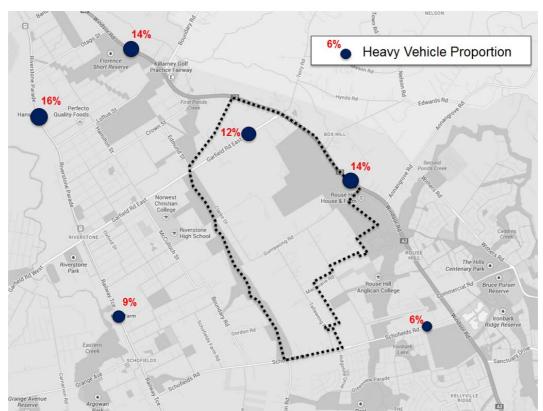


Figure 13 Existing heavy vehicle proportions

3.6 Rail Services

3.6.1 Richmond Rail Line

The Riverstone East precinct is currently served by the Richmond railway line, a branch of the main western line. The Richmond Line currently provides access to key centres located throughout Sydney via both direct links and onward connections. A summary of the existing services along the Richmond Line is shown in Table 1.

Departing			Average Frequency of Services (Weekday)		
Station		AM Peak (7am – 9am)	PM Peak (4pm – 6pm	Off Peak (10am – 3pm)	
Riverstone	Northbound	30 minutes	30 minutes	30 minutes	
	Southbound	30 minutes	30 minutes	30 minutes	
Schofields	Northbound	20 minutes	15 minutes	15 minutes	
	Southbound	12 minutes	15 minutes	15 minutes	

 Table 1 Existing services along the Richmond rail line

In 2011 a duplication of the rail line was completed between Quakers Hill and Schofields, including the opening of the new Schofields Station. This duplication has allowed for more frequent train services travelling to and from Schofields. The new station at Schofields includes 230 park and ride spaces and a new bus interchange servicing residents of the North West Growth Centre.

A second stage of the project includes a new and relocated Vineyard station and an upgrade of the existing Riverstone Station. This second stage of the project is not presently proceeding, however the planning of the Riverstone Precinct considered a new station location at Vineyard.

3.6.2 Sydney Metro Northwest

Sydney Metro Northwest (previously known as the North West Rail Link), is the first stage of Sydney Metro. The project is scheduled for completion in 2019 and will deliver eight new railway stations to Sydney's North West, providing a connection into Chatswood and the Sydney CBD. Passengers will be provided with rail services every 4 minutes during peak periods and every 10 minutes across the day.

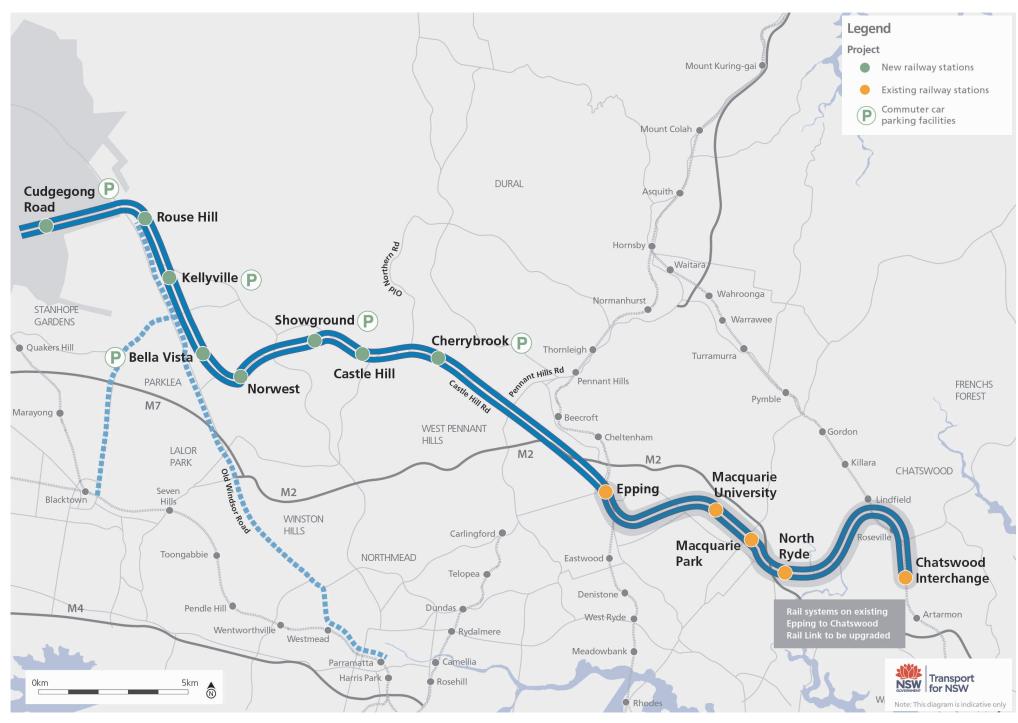
The rail line will provide connections between the NWGC and major destinations such as Norwest, Castle Hill, Macquarie Park, Chatswood, North Sydney and the Sydney CBD. The new rail line will ultimately connect with Metro City & Southwest - the second stage of the Sydney Metro network.

Future residents of the Riverstone East precinct will be provided with a high quality rail interchange via a new station at Cudgegong Road. Located between Tallawong Road and Cudgegong Road, the new station will provide for 1,000 commuter car parking spaces and space for 6 buses. The station design will include pedestrian linkages to these areas as well as secure parking and storage for up to 45 bicycles. The station layout is shown in Figure 14, and is wholly located within the Area 20 precinct.



Figure 14 Cudgegong Road station layout

An overview of the proposed rail link is shown in Figure 15 on the following page.



Source: Transport for NSW

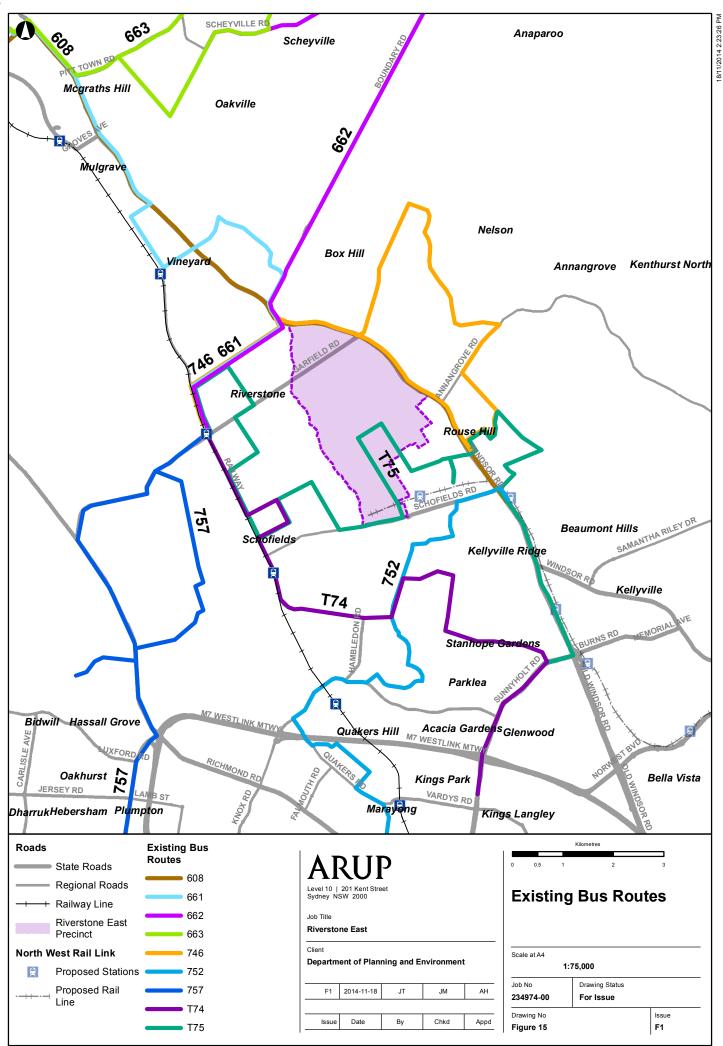
Figure 14: Sydney Metro Northwest

3.7 Bus Services

Due to the current low level of development, there are currently limited bus services within the vicinity of the Riverstone East precinct. These typically run at low frequencies throughout the day, and include the following routes:

- Route 608: Windsor to Rouse Hill (via Windsor Road)
- **Route 661**: Windsor to Riverstone via McGraths Hill (via Commercial Road and Crown Street)
- Route 662: Riverstone to Maraylya and Oakville (via Boundary Road)
- Route 663: Windsor to Wisemans Ferry via Pitt Town
- Route 746: Riverstone to Box Hill (via Crown Street and Windsor Road)
- Route 752: Blacktown to Rouse Hill via Quakers Hill & The Ponds
- **Route 757**: Mt Druitt to Riverstone via Rooty Hill Rd North & Marsden Park (via Richmond Road to Riverstone Station)
- **Route T75**: Blacktown to Rouse Hill and Riverstone (via Schofields Road, Tallawong Road and Cudgegong Road)
- **Route T74**: Blacktown to Riverstone via The Ponds (via Burdekin Road and Railway Terrace)

A summary of the existing bus routes serving the Riverstone East precinct are illustrated in Figure 16 on the following page.



global.arup.com\australasia\SYDIProjectsi234000\234974-00 Riverstone E Vineyard\Work\06 GIS\Projects/Riv E & Vineyard Transport Planning_20141118_JRT.mx

3.8 Walking and Cycling

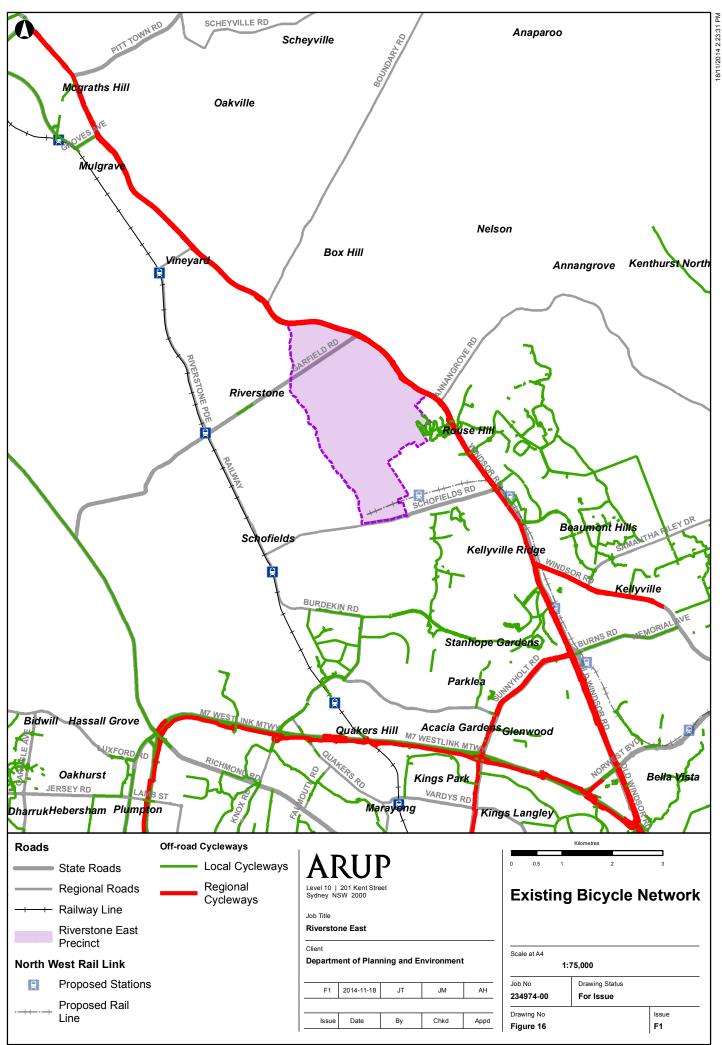
Due to the current undeveloped, primarily rural nature of Riverstone East, existing pedestrian and cycling routes and facilities within and surrounding the precinct are limited.

Pedestrian crossing facilities are provided at signalised intersections of Windsor Road, including at Garfield Road, Rouse Road and Burns Road. The majority of streets in the precinct currently contain footpaths, although these are often damaged and/or narrow in sections.

A shared pathway is provided on the western side of Windsor Road (between Rouse Road and Commercial Drive) which facilitates regional cycling movements. Regional cycling links are also provided on the surrounding network, including cycle lanes on the M7 Motorway and on-road cycle lanes on Quakers Hill Parkway.

A number of roads in the precinct are designated as on-road cycleways, however include no dedicated bicycle facility (e.g. on-road markings). These designated cycle routes often carry large volumes of traffic and are generally only appropriate for confident riders.

A summary of the existing off-road bicycle paths in the vicinity of the study area is shown in Figure 17 on the following page.



. Vglobal.arup.com/australasia/SYD/Projects/234000/234974-00 Riverstone E Vineyard/Work/06 GIS/Projects/Riv E & Vineyard Transport Planning_20141118_JRT.mxd

4 **Precinct Planning Principles**

This following section presents the overall principles and objectives that contributed to the development of the proposed transport network for the Riverstone East Precinct.

4.1 Transport Planning Objectives

In developing the preferred transport network, in conjunction with the precinct mater planners, the following key transport objectives have been considered. These aim to provide for a coherent, legible transport network that supports movement both to, and within, the Riverstone East precinct.

- Provide a road network that allows for good access to all modes of transport, particularly public transport, walking and cycling;
- Design a physical site layout which encourages walking and cycling, particularly to key land uses and public transport nodes;
- Ensure the road network for Riverstone East provides suitable connections to adjacent development precincts in the North West Growth Centre;
- Integrate transport and land use planning so that high intensity land uses have strong accessibility to public transport;
- Provide high quality access to public transport stops to reduce the dependence on private vehicles. Future residents of Riverstone East should be located within at least a 400m radius of a bus stop;
- Develop an appropriate road hierarchy which provides adequate carrying capacity on higher order roads to meet reasonable community expectations;
- Protect residential areas from through traffic intrusion, particularly heavy vehicles.

4.2 Road Classification and Capacity

Guidelines for road network design can be allocated into three main categories:

- Road classification (road hierarchy) how will traffic move through the precincts and are roads designed to accommodate particular function in mind?
- Road capacity are adequate lanes provided on the streets to accommodate traffic without significant congestion?
- Intersection performance are delays at intersections acceptable?

The Growth Centres Development Code (Growth Centres Commission, 2006) classifies the hierarchy based on anticipated levels of daily traffic as summarised in Table 2. The classification of each road will dictate its physical form (i.e. number of lanes, road reserve width), function (what types of vehicles utilise the road) and the speed limit.

Road Type	AADT*	Functions and Connections	Speed Limit
Arterial/ Freeway	>35,000	Connects large urban areas	80km/hr +
Transit Boulevard	30,000 – 35,000	Located close to centres Pedestrian friendly environment Allows for long term upgrades and dedicated busways	60 - 80km/hr
Sub-Arterial	10,000 – 35,000	Arterial roads to town centres Carries major bus routes	Up to 70km/hr
Collector	3,000 - 10,000	Connects neighbourhoods Can accommodate public transport	Up to 60km/hr
Local	cal 1,000 – Priority to pedestrians and cyclists 3,000 Designed to slow residential traffic		Up to 50km/hr

 Table 2 Functional classification of roads

* Annual Average Daily Traffic

4.3 Road Cross Sections

Typical road corridor cross sectional design requirements for future urban development have been determined as a standard to be adopted throughout the North West Growth Centre. These are summarised in Table 3. Drawings showing each of the typical road cross sections are included as Appendix A to this report.

Road Type	Traffic Load (Vehicles/Day)	Road Corridor Width	Road Carriageway Lanes, Number and Width
Arterial Road	35,000 +	To be determined by the RMS	To be determined by the RMS
Transit Boulevard	30,000-35,000	41 metres	4 Lanes Divided plus 2x 5.5 metre service roads
Sub Arterial Road	10,000-35,000	35 metres	4 Lanes Divided plus 2x 1.8 metres cycle lanes
Collector Road	3,000-10,000	20 metres	12 metres or 13 metres if a bus route
Local Road	1,000-3,000	16 metres	10-11 metres

 Table 3 GCC standard road types and road corridor widths

In general all future road cross sections, as summarised in Table 3, should be reviewed at the precinct master planning Development Control Plan (DCP) stage to minimise any un-necessary "land take" requirement.

4.4 Heavy Vehicles

As identified in Section 3.5, heavy vehicles presently comprise a significant proportion of total traffic through the precinct. The ILP for Riverstone East should aim to minimise heavy vehicle intrusion to enhance the amenity of the area. Measures to achieve this objective include:

- Introducing load limits on certain roads within the precinct (buses excepted);
- Restricting certain left turn movements from Windsor Road into the precinct for vehicles over 6m; and
- Providing advisory signposting that requires that heavy vehicles use major arterial routes such as Windsor Road and Schofields Roads.

4.5 **Public Transport Provision**

For new precincts within the North West Growth Centre, where private vehicle has historically dominated the transport landscape, it is vital that strong public transport linkages are provided at an early stage when new residents establish their travel habits. As illustrated in Figure 5, private vehicle is the dominant mode of travel in the NWGC. Reducing this dependence on private vehicles must form one of the key objectives of this transport study.

Local bus routes should be planned so that they run through the core of the precinct, as opposed to along arterial roads with poor pedestrian connectivity. This provides a higher level of public transport accessibility to future residents of Riverstone East. Regional bus routes should be provided which allow more direct, time efficient services to key regional centres such as Rouse Hill and Blacktown. A number of local bus services would also service stations on the North West Rail Link to encourage multi-modal public transport trips.

Adequate facilities will need to be provided for public transport users to encourage a mode shift away from private vehicles. This includes infrastructure items such as bus shelters, waiting areas and other bus priority measures. The provision of a good quality, permeable footpath network will be critical in ensuring users are able to easily access public transport stops.

Land use planning will also play a vital role in facilitating increased use of public transport. High density developments should be located close to public transport nodes as these will be the areas where the highest mode shares to public transport will be achieved. Consideration should also be given to major pedestrian attractors such as schools and their proximity to the public transport network. Streets and roads containing public transport stops should be activated wherever possible to enhance the amenity and attractiveness for pedestrians waiting for services.

4.6 Walking and Cycling

Walking and cycling will play an important role in meeting the future transport needs of the Riverstone East precinct. Providing a mix of uses within the precinct, in close proximity to the village centre, will promote the use of sustainable travel modes.

It is important walking and cycling routes are integrated with those provided in adjacent growth centre precincts and regional cycle routes such as the Windsor Road shared path and Westlink M7 cycleway. Linkages to major land uses such as schools, retail and public transport nodes should also form a key consideration when planning pedestrian and cycle routes.

4.6.1 **Pedestrian Connectivity**

Footpaths should be provided on both sides of the road carriageway in accordance with the standard road cross sections described in Section 4.3. Appropriate pedestrian crossing facilities should be incorporated at intersections and along key desire lines to ensure safe and efficient pedestrian movements. Signalised intersections should provide crossing legs on all approaches, while pedestrian refuges should be integrated in the design of any roundabout within the precinct. Associated infrastructure such as pram ramps and bike lanterns at all traffic signals should be installed

Bus stops should be located to allow for good pedestrian accessibility. Where possible, stops should be positioned close to traffic signals or alternative safe pedestrian crossing facilities (e.g. pedestrian refuges, zebra crossings). Adequate shelter and seating should be provided, particularly to service less mobile users.

4.6.2 Cycling

The NSW Bicycle Guidelines (Roads and Traffic Authority, 2003) summarises the needs and requirements of bicycle users by the following five design principles:

- **Safety**: A good quality route enhances the safety of all users, including cyclists, pedestrians and motorists. Streets and intersections along key bicycle routes should be designed to a standard which incorporates cyclist movements.
- **Coherence**: The bicycle network should link popular destinations with local residential streets via a mix of both local and regional routes. The network should be continuous and easily identifiable to both novice and experienced cyclists.
- **Directness**: Bicycle routes should be as direct as possible, having consideration for major barriers such as road intersections and steep topography. The rider should ideally be able to maintain a safe and comfortable consistent riding speed throughout their journey.
- Attractiveness: The bicycle network must be designed so that it complements and enhances its environment in such a way that cycling is attractive. Clear and strategically placed wayfinding information should indicate distances and times to major destinations.

• **Comfort**: Bicycle routes must be comfortable and easy to use for all cyclists. Depending on the road environment and topography, some level of separation (e.g. clearly marked bicycle lanes, painted green) may be required.

The above requirements are also noted in The Cycling Aspects of Austroads Guides (2014), which contains information that relates to the planning, design and traffic management of cycling facilities. Austroads notes the following key components of a good quality bicycle network.

- a designated regional network of roads and paths that serves longer-distance commuter and recreational trips
- designated local networks and routes designed to provide low-stress routes, to feed the regional network and to provide for shorter local trips to shopping centres, recreational activities, public transport hubs
- *full construction of route sections between origins and destinations consistent with the route purpose*
- convenient access into and through residential, commercial and industrial subdivisions, and major developments
- access and facilities to travel with a bicycle on public transport
- secure long and short-term parking facilities at major destinations
- safer routes to schools
- well-defined bicycle facilities on arterial roads where significant cyclist demand exists including specifically for commuter trips
- appropriate maintenance practices which result in smooth surfaces
- calming in local streets
- paths which are interesting, that include rest areas at appropriate intervals on regional routes, and are designed to appropriate geometric standards
- *implementation of regulatory, warning and guidance signage on paths.*

When determining the most appropriate cycling treatment on a bicycle route, consideration must be given to the traffic speed and traffic volume. The NSW Bicycle Guidelines provide direction relating to the most suitable cycling treatment for different roads, as reproduced in Figure 18.

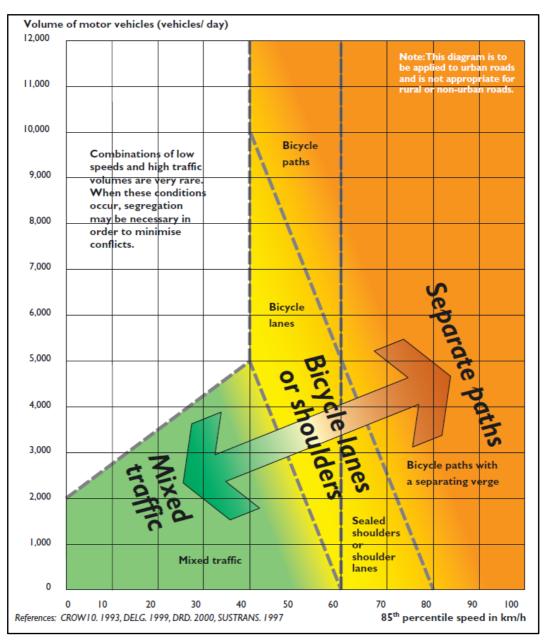


Figure 18 Separation of bicycles and motor vehicles

Source: NSW Bicycle Guidelines, Figure 3.2

The physical separation of cyclists from motor vehicles on to a bike path or shared path (minimum 3m wide) is recommended on local and collector roads that have traffic volumes of more than 5,000 vehicles per day on a 60 km/h road, as well as roads serving certain land uses including schools and open space areas. Shared paths act a safe, convenient bicycle facility where physical separation between cyclists and vehicles is necessary on roads with high vehicles speeds and volumes.

Directional signage should be installed to improve connectivity and wayfinding, with regulatory signs and lines to be installed as per NSW Bicycle guidelines. Street lighting along cycleways should be considered in reserve areas

Bicycle parking facilities should be provided at key destinations (e.g. train stations, village centres and major developments) to accommodate the needs of both short and long term cyclists. Parking for short stay cyclists should be provided in areas with passive surveillance - if the parking is visible it will be perceived as more secure and achieve greater utilisation. Parking for long stay cyclists should be in secure, lockable facilities which provides weather protection and conveys a sense of high priority for the treatment of riders.

4.7 Parking

On-site parking for land uses within the Riverstone East precinct should be provided in accordance with the rates outlined in Blacktown City Council Growth Centre Precincts DCP 2010, as summarised in Table 4 and

Table 5

Zone	Car Parking Requirement
R2 zone (shop top housing)	• 1-2 bedrooms: 1 covered space (min)
	• 3 bedrooms or more: 2 covered spaces (min)
R3 zone	• 1-2 bedrooms: 1 covered space (min)
	• 3 bedrooms or more: 2 covered spaces (min)
	• 1 visitor car parking space per 5 apartments
B2 and B4 zones	• 1-2 bedrooms: 1 covered space (max)
	• 3 bedrooms or more: 2 covered spaces (max)
	• 1 visitor car parking space per 8 apartments

Table 4 Parking Rates – Residential Uses

Table 5 Parking Rates – Non-Residential Uses	Table 5	Parking	Rates –	Non-F	Residential	Uses
--	---------	---------	---------	-------	-------------	------

Land Use	Car Parking Requirement
Commercial/office premises	1 space per 40m ² GFA
Retail shops/showrooms (less than 200m ² GFA)	1 space per 30m ² GFA
Retail shops/showrooms (greater than 200m ² GFA)	1 space per 22m ² GFA
Restaurants/cafes	1 space per 10m ² of dining area 1 space per 3 employees

5 Road Network Assessment

This section provides an assessment of the future road network that will support the development of the Riverstone East precinct and meet future traffic demands. It also evaluates the performance of key intersections within the precinct during the critical commuter peak hours. This assessment has informed the master planning team in the development of the draft ILP for the precinct.

5.1 Assessment Criteria

5.1.1 Road Capacity

Mid-block capacity requirements (for interrupted flow conditions) for roads supporting the Riverstone East precinct have been based on Austroads Guide to Traffic Management³. These are outlined in Table 6.

Lane Type (interrupte	One-way mid-block capacity (veh/hour)	
Median or inner lane	Divided road	1,000
	Undivided road	900
Middle lane	Divided road	1,000
	Undivided road	900
Kerbside lane	Adjacent to parking lane	900
	Occasional parked vehicles	600
	Clearway conditions	900

 Table 6 Mid-Block capacities of urban roads

Based on the mid-block capacities outlined above, in conjunction with future traffic volumes, roadway levels of service of service can be determined. This is summarised in Table 7 on the following page (as outlined in Austroads Guide to Traffic Management⁴).

³ Part 3: Traffic Studies and Analysis, Section 5.2.1, Table 5.1

⁴ Part 2: Roadway Capacity

Level of Service	Volume / Capacity Ratio	Description (interrupted flow conditions)	
А	0.00 to 0.34	Primarily free flow operations at average travel speeds	
В	0.35 to 0.50	Reasonably unimpeded operations at average travel speeds	
С	0.51 to 0.74	Stable operations; however, ability to manoeuvre and change lanes in mid-block locations may be more restricted	
D	0.75 to 0.89	Small increases in flow may cause substantial increases in delay and decreases in travel speed.	
Е	0.90 to 0.99	Significant delays and average travel speeds of 33% of the free flow speed or less	
F	>1.00	Characterised by urban street flow at extremely low speeds. Intersection congestion is likely at critical signalised locations	

Table 7 Roadway levels of service definition

5.1.2 Intersection Capacity

The performance of intersections in an urban environment is measured in terms of its Level of Service (LoS). Levels of service ranges from A (very good) to F (over capacity with significant delays). This is described in the RTA Guide to Traffic Generating Developments as summarised in Table 8. In peak hours at intersections controlled by traffic signals on key regional and arterial routes, a Level of Service D is generally acceptable.

Level of Service	Average Vehicle Delay (seconds)	Traffic Signals and Roundabouts	Priority Intersections ('Stop' and 'Give Way')
А	< 14	Good operation	Good operation
В	15 to 28	Good with acceptable delays and spare capacity	Acceptable delays and spare capacity
С	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Operating near capacity	Near capacity and accident study required
E	57 to 70	At capacity. At signals, incidents will cause excessive delays. Roundabouts require other control mode	At capacity; requires other control mode
F	>71	Unsatisfactory with excessive queuing	Unsatisfactory with excessive queuing; requires other control mode

Table 8 Intersection level of service

5.2 Strategic Network Modelling

A strategic transport network model has been developed for the NWGC, which utilises a NETANAL model set up for appraising the traffic generated by each of the precincts. It was also used to assess various ILP options in terms of identifying impacts associated with varying levels of generated traffic. This traffic model has been used as the basis to forecast future year traffic volumes in the vicinity of the Riverstone East Precinct.

The model was calibrated utilising traffic data collected in March 2014 (as outlined in Section 3.3 of this document). Travel time surveys were also conducted to assist in the calibration process.

The NETANAL model utilises defined travel demand between travel zone pairs, represented as assimilated traffic movements, throughout the Sydney Metropolitan Area. The model incrementally assigns vehicular traffic onto a computer-based road network developing link demand forecasts on each modelled section of road. To be consistent with transport modelling undertaken for a number of key planning studies and documents, the year 2036 was taken as the design year for assessing network performance, with 2014 used as the base year.

As part of the strategic transport model, travel zones (TZs) within Riverstone East were structured using areas of the precinct delineated by road classification. This process assists in assigning generated traffic onto relevant parts of the network for each traffic zone.

5.3 Traffic Generation and Trip Containment

Traffic generation estimates for the precinct have been sourced from the recently updated Roads and Maritimes (RMS) '*Guide to Traffic Generating Developments*'. These rates are summarised Table 9 below.

Land Use	Traffic Generation Rate (vehicles / hour)		
	AM Rate (8am – 9am)	PM Rate (5pm – 6pm)	
Large lot residential	0.81 / dwelling	0.83 / dwelling	
Low density residential	0.81 / dwelling	0.83 / dwelling	
Medium density residential	0.50 / dwelling	0.50 / dwelling	
High density residential	0.19 / dwelling	0.15 / dwelling	
Commercial (office)	1.6 / 100m ² GFA	1.2 / 100m ² GFA	
Retail	1.94 / 100m ² GFA	9.84 / 100m ² GFA	
Business Park / Light Industrial	0.52 / 100m ² GFA	0.56 / 100m ² GFA	

Table 9 Traffic generation rates

5.4 Future Traffic Volumes

The trip generation assumptions used for the traffic modelling was informed by the land uses incorporated in a preliminary master planning team and economic feasibility study undertaken by the wider project team for the Riverstone East precinct. Population and employment forecasts for the wider North West Growth Centre area were adopted from current estimates provided by the Bureau of Transport Statistics (BTS), following consultation with Planning and Environment.

Stage 1 & 2 development of the Riverstone East precinct will provide for approximately 1,800 dwellings and Stage 2 will allow for approximately 1,400 dwellings. For the ultimate development of the precinct, the following land uses have been assumed.

A total of 5,784 residential dwellings, comprising of:

- 606 high density residential dwellings;
- 949 medium density residential dwellings; and
- 4,154 low density residential dwellings
- 75 large lot / environmental living dwelling
- 10,880m² light industrial floor space;
- 2,400m² retail floor space (within a neighbourhood centre); and
- 2,580m² non-retail floor space

A further major generator of traffic within the Riverstone East precinct is the new 1,000 space commuter car park planned to service Cudgegong Road station. This car park is forecast to induce traffic not only from residents of Riverstone East but also surrounding precincts within the NWGC.

A summary of the forecast traffic volumes for the future year 2036 at key intersections within the Riverstone East precinct is provided in Table 10.

Intersection	Approach	Approach Traffic Volumes		
		AM Peak Hour	PM Peak Hour	
Windsor Road /	Windsor Road W	1,150	1,450	
Mt Carmel Road	Windsor Road E	1,355	903	
	Mt Carmel N	526	649	
	Mt Carmel S	808	599	
Garfield Road /	Garfield Road E	865	870	
Hambledon Road	Garfield Road W	459	366	
	Hambledon Road N	780	1,116	
	Hambledon Road S	1,093	794	
Windsor Road / Terry	Windsor Road W	868	1,228	
Street / Garfield Road	Windsor Road E	1,865	1,338	
	Terry Street N	647	876	
	Garfield Road W	694	441	
Guntawong Road /	Hambledon Road N	655	889	
Hambledon Road	Hambledon Road S	949	538	
	Guntawong Road E	484	688	
	Guntawong Road W	803	441	
Tallawong Road /	Tallawong Road N	343	451	
Guntawong Road	Tallawong Road S	453	179	
	Guntawong Road E	604	874	
	Guntawong Road W	665	329	
Cudgegong Road /	Cudgegong Road S	155	114	
Guntawong Road	Cudgegong Road E	630	481	
	Guntawong Road W	344	514	
Worcester Road /	Worcester Road S	138	165	
Guntawong Road	Guntawong Road E	517	612	
	Guntawong Road W	ng Road E 517	319	
Windsor Road /	Windsor Road N	1,931	1,682	
Guntawong Road	Windsor Road S	1,285	2,064	
	Guntawong Road E	203	93	
	Guntawong Road W	612	365	
Schofields Road /	Hambledon Road N	884	1,133	
Hambledon Road	Hambledon Road S	581	336	
	Schofields Road E	774	803	
	Schofields Road W	1,346	974	
Schofields Road /	Tallawong Road N	1,139	965	
Tallawong Road	Tallawong Road S	374	637	
	Schofields Road E	804	814	
	Schofields Road W	1,023	1,371	
Tallawong Road /	Hambledon Road NE	955	1,351	
Hambledon Road	Hambledon Road SW	747	325	
	Tallawong Road S	529	308	

Table 10 2036 Traffic Volumes Summary – Riverstone East

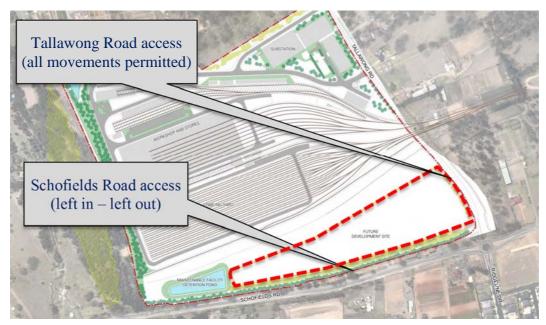
Transport for NSW Residual Lands

The North West Rail Link stabling yard is to be constructed within the Riverstone East precinct, west of Tallawong Road. A residual parcel of land south of the stabling yard has been identified for future urban development. The land is currently used and owned by Transport for NSW, but will become available for development upon completion of work associated with the stabling yard. The precinct master planners have advised that local light industrial land uses could be accommodated within this parcel of land, with approximately 11,000m² of GFA anticipated.

Vehicular access to the site is constrained, given the location of the stabling yards to the north. A signalised intersection on Schofields Roads to service the site is not recommended due to the close proximity of traffic signals at Tallawong Road and Hambledon Road.

Given the geometry of the land (long and narrowing towards the west), it is recommended the site be serviced via two access locations. This will provide for efficient vehicle circulation and movement. The final access arrangements by the ultimate developer of the residual land will need to involve detailed discussion with Roads and Maritime Services to develop and agree an optimal arrangement.

- 1. Tallawong Road, adjacent to the new east-west road serving the railway station. This would act as the primary access point to the site, with all traffic movements permitted.
- 2. A secondary access point on Schofields Road, however traffic movements would be restricted to left in left out only. Those approaching from Schofields Road (eastern approach) would turn right into Tallawong Road and then left into the site.



This arrangement is illustrated in Figure 19 below.

Figure 19 Transport for NSW residual lands

Note: Analysis has determined that the basin shown fronting Schofields Road is now longer required.

5.5 Traffic Distribution

The future year trip tables, produced by BTS, have been developed from a 4-step travel model based on forecast population, employment and the transport network. Generally, the traffic distribution utilised in the NETANAL modelling for the future year trip tables of the Sydney Metropolitan Region has been retained from the BTS trip matrices.

Route selection is determined on the basis of the shortest travel time or cost, considering the inherent route delays incurred along a possible link.

5.6 Road Network Hierarchy

The road network hierarchy supporting the Riverstone East precinct has been developed based on the future traffic demands arising from the development of the area, consistent with the transport planning objectives outlined in Section 4.1 of this study.

Guntawong Road

Following discussions with DP&E regarding the extension of the Rouse Hill Regional Park, an assessment has been undertaken to understand the traffic implications arising from the potential closure of Guntawong Road through the Rouse Hill Regional Park (i.e. between Worcester Road and Windsor Road). This is following a request by the Office of Environment and Heritage (OEH, now part of DP&E).

Guntawong Road has previously been identified as a collector road in transport planning undertaken for the North West Growth Centre. The Box Hill transport strategy recommended traffic signals be provided at the Windsor Road / Guntawong Road intersection as a result of increased traffic movements in future years, providing for full turning movements with Box Road into the Box Hill Industrial Precinct.

The primary function of Guntawong Road is to provide a road connection to Windsor Road to service the residential and employment population of both the Riverstone and Riverstone East precincts. Closure of Guntawong Road would result in the redistribution of traffic along Cudgegong Road and Tallawong Road onto Schofields Road – as illustrated in Figure 20.

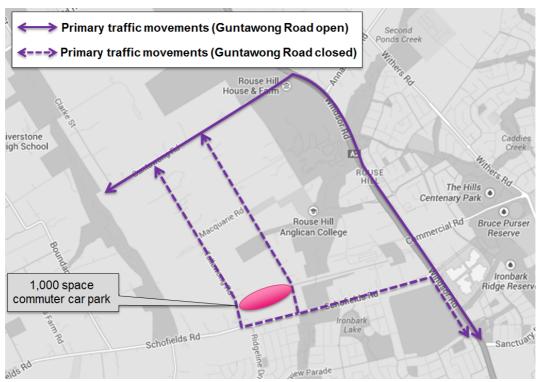


Figure 20 Traffic implications of Guntawong Road closure

In the Schofields Road upgrade and extension planning documentation, Guntawong Road was relied upon to direct traffic directly onto Windsor Road to reduce the pressure on the new signalised intersections on Schofields Road. These intersections, at Cudgegong Road and Tallawong Road, have been designed (and recently constructed as of July 2014) on this basis.

A sensitivity analysis has been undertaken which compares the performance of the road network under the following two scenarios:

- (i) Guntawong Road open to traffic
- (ii) Guntawong Road closed to traffic

The analysis has considered the future operation of intersections along Schofields Road, as well as the mid-block road capacity of Cudgegong Road and Tallawong Road. The results of the analysis are outlined in Table 11.

Location	Peak Hour	Level of Service*		
		Scenario 1: Guntawong Road Open	Scenario 2: Guntawong Road Closed	
Schofields Road /	AM Peak	D	Е	
Tallawong Road	PM Peak	C	Е	
Schofields Road / Cudgegong Road	AM Peak	C	D	
	PM Peak	D	Е	
(mid hla ala)	AM Peak	E	F	
	PM Peak	D	F	
Cudgegong Road	AM Peak	D	Е	
(mid-block)	PM Peak	E	F	

Table 11	Sensitivity	analysis -	Guntawong Road closure
----------	-------------	------------	------------------------

* Refer to Level of Service standards outlined in Table 6 and Table 8.

The traffic modelling results demonstrate a significant deterioration in road network performance following the closure of Guntawong Road. This is primarily the result of redistributed traffic along Tallawong Road and Cudgegong Road adding to congestion issues arising from vehicles accessing the 1,000 space commuter car park, as well as high regional traffic flows on Schofields Road.

Retaining Guntawong Road as part of the wider road network provides a 'release valve' for traffic travelling between Riverstone East and Windsor Road. It provides for a more permeable road network that best accommodates the anticipated traffic demands of the NWGC. It is therefore recommended that Guntawong Road form a component of the road network serving the Riverstone East precinct.

Guntawong Road should however be designed so as to minimise the level of severance and enhance north-south connectivity through Rouse Hill Regional Park. This may be achieved through narrowing the road width so that only two lanes of traffic are provided between Windsor Road and Worcester Road. Parking lanes in this section of road are not considered necessary.

Other alternatives which may be considered to improve north-south connectivity through the regional park across Guntawong Road include provision of a pedestrian overbridge, an at-grade zebra crossing or a land bridge associated with cut and cover of a 3m section of the road.

Preferred Road Network Hierarchy

The preferred road network structure to support the full development of the Riverstone East precinct as envisaged under the draft master plan is illustrated in Figure 21.

This network considers the functional road type as described in Section 4.2 to support the future land uses envisaged for the Riverstone East precinct. It is consistent with previous road network planning for adjacent growth centre precincts and recent road upgrade works such as Schofields Road.

These routes provide connectivity to key town and regional centres, as well as major road links including Richmond Road and the M7 motorway. The existing roads within the Riverstone East precinct will form the structure of the road network. The primary traffic routes (arterial and transit boulevards) serving the precinct include:

- Windsor Road;
- Garfield Road East; and
- Schofields Road.

A number of existing internal roads have been identified for improvement and form the framework for the higher-order (sub-arterial / collector) road network. These roads will accommodate internal traffic, bicycle and pedestrian movements, and include:

- Cudgegong Road;
- Tallawong Road; and
- Hambledon Road North.

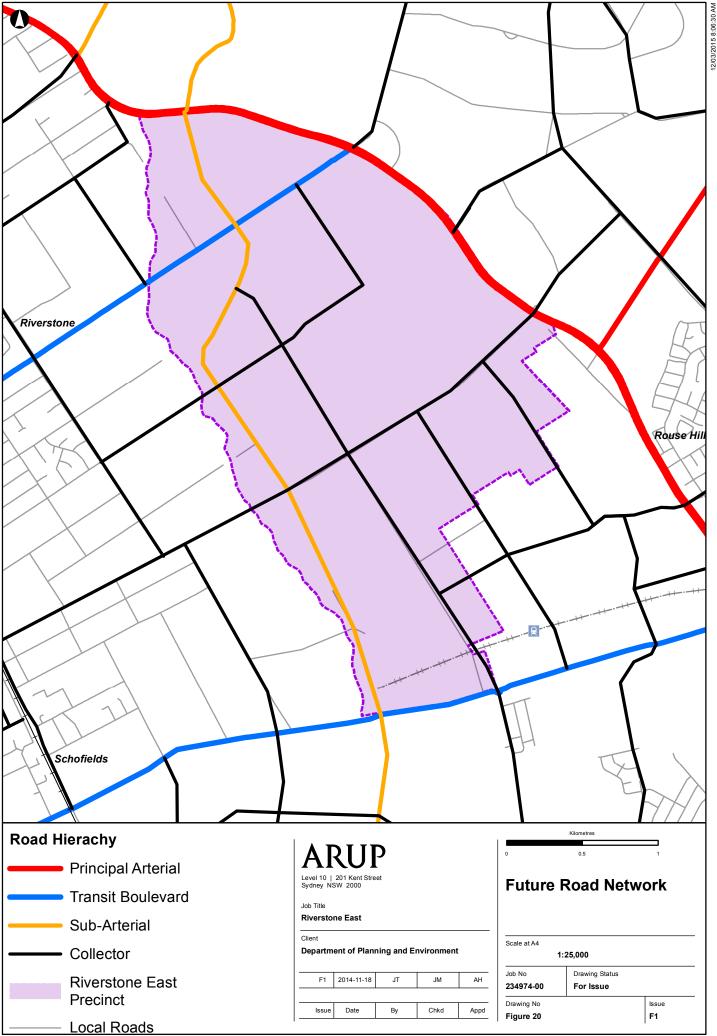
A number of new road links have been identified to enhance connectivity and permeability both within the Riverstone East precinct and to adjacent areas. These new road links include:

- Hambledon Road North: A sub-arterial road corridor which will provide a north-south connection between Hambledon Road South (in the Alex Avenue precinct) to Windsor Road. More broadly, this road link will serve as a connection for vehicles travelling between the M7 and Quakers Hill Parkway and Windsor.
- Tallawong Road: Extension of the existing Tallawong Road alignment between Riverstone Road and the Hambledon Road extension.
- Rouse Road: Extension of the existing Rouse Road alignment through the Area 20 precinct to connect with Cudgegong Road in the Riverstone East precinct.

Supporting this higher order road network will be a number of local roads which provide access into individual lots. These local roads would typically include traffic calming devices (e.g. raised thresholds, roundabouts) to reduce the instances of through traffic intrusion.

The RMS is currently undertaking a study which investigates the most appropriate road alignment for a grade separated crossing of the Richmond rail line through the Riverstone precinct. The study is being overseen by a Working Group consisting of NSW Roads and Maritime Services, Transport for NSW, Blacktown City Council and NSW Department of Planning & Environment. This will replace the existing Garfield Road level crossing, which is a source of traffic congestion during the commuter peak hours.

For the purposes of this transport study, it has been assumed a grade separated crossing of the Richmond railway line will be provided along the existing Garfield Road alignment. Should an alternative corridor be identified for the grade separated crossing, the traffic role of Garfield Road would be downgraded to serve more of a local function.



Stage 1 and Stage 2 Road Network

The following road upgrades will initially be provided to support the staged development of the precinct.

- Hambledon Road North (sub-arterial connection between Schofields Road and Garfield Road)
- Tallawong Road (between Schofields Road and Hambledon Road North)
- Macquarie Road (between Tallawong Road and Cudgegong Road)
- Guntawong Road (between Hambledon Road North and Tallawong Road_

Outside of these road upgrades, the existing road network is considered appropriate to support the level of development envisaged under the Stage 1/2 ILP for Riverstone East. Complementary intersection upgrades at the locations identified in Section 5.7 of this study will also be required to accommodate the additional traffic movements arising from the proposed development.

As the level of urban development in growth precincts adjacent to Riverstone East (e.g. Box Hill) increases, the necessity to upgrade existing roads which lie outside the Stage 1/2 boundary will need to be reviewed.

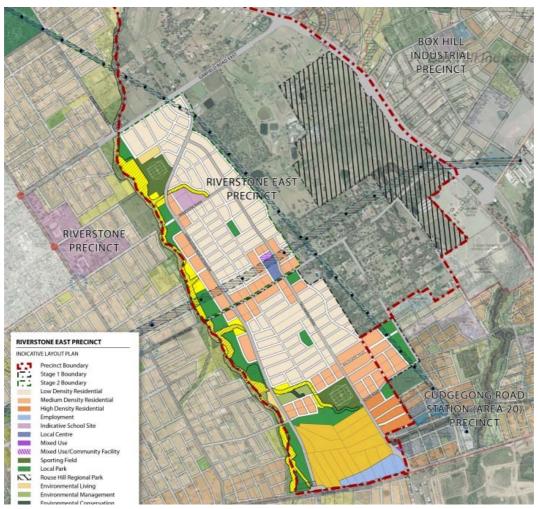


Figure 21 Stage 1 and Stage 2 Road Network

5.7 Intersection Capacity Analysis

Traffic modelling of key intersections in the Riverstone East precinct has been undertaken to determine the required layout and configuration to support the future population. The analysis has considered the ultimate development of the entire Riverstone East precinct as summarised in Section 5.4.

At key intersections (i.e. the junction of collector / sub-arterial roads), traffic signals are generally required to manage the high traffic movements expected following the development of the area. Signalised intersections provide benefit not only with respect to traffic capacity but also to support pedestrian and bicycle movements across major roads.

Intersection analysis has been undertaken for the following sites:

- Windsor Road / Mt Carmel Road
- Windsor Road / Garfield Road East
- Windsor Road / Nelson Road
- Windsor Road / Guntawong Road
- Schofields Road / Hambledon Road
- Schofields Road / Tallawong Road
- Schofields Road / Cudgegong Road
- Guntawong Road / Hambledon Road
- Guntawong Road / Tallawong Road
- Guntawong Road / Cudgegong Road
- Garfield Road East / Hambledon Road

The SIDRA Intersection Analysis V6 model package was used to assess the future peak-hour operating performance of the above intersections. The intersections within the precinct have typically been configured to operate at Level of Service D or better – in line with the criteria for evaluating the operational performance of intersections in the *RTA Guide to Traffic Generating Developments* (Table 8).

The future turning movement volumes for key intersections within the Riverstone East precinct used in the intersection assessment were extracted from the outputs of the NETANAL strategic modelling undertaken for the study. These were generated for both the 2036 AM peak and PM peak flows.

The recommended intersection controls are summarised in Table 12 below.

Intersection	Intersection Control	
	Current	Proposed
Windsor Road / Mt Carmel Road	n/a	Traffic Signals
Windsor Road / Garfield Road East	Traffic Signals	Traffic Signals
Windsor Road / Nelson Road	Traffic Signals	Traffic Signals
Windsor Road / Guntawong Road	Priority Control	Traffic Signals
Schofields Road / Hambledon Road	n/a	Traffic Signals
Schofields Road / Tallawong Road	Traffic Signals	Traffic Signals
Schofields Road / Cudgegong Road	Traffic Signals	Traffic Signals
Guntawong Road / Hambledon Road	Priority Control	Traffic Signals
Guntawong Road / Tallawong Road	Priority Control	Traffic Signals
Guntawong Road / Cudgegong Road	Priority Control	Roundabout
Garfield Road East / Hambledon Road	Priority Control	Traffic Signals
Tallawong Road / Railway Road South	n/a	Traffic Signals
Hambledon Road / Tallawong Road	n/a	Traffic Signals
Tallawong Road / Rouse Road	n/a	Roundabout
Tallawong Road / Riverstone Road	n/a	Roundabout

Table 12 Recommended Intersection Controls

n/a: Notes that intersection does not currently exist.

Traffic signals are recommended at eight new locations in the precinct based on the traffic modelling undertaken for this study. The RMS has specific requirements relating to vehicular and pedestrian volumes where it will consider the installation of traffic signals at an intersection. These are commonly referred to as signal warrants. Section 2 of the RMS Traffic Signal Design Manual outlines five different warrants for the installation of traffic signals at intersections. These are summarised in Table 13.

Table 13 Warrants for Traffic Signals at Intersections

Warrant	Requirements			
Traffic Demand	For each of the four one-hour periods of an average day:			
	(i) The major road exceeds 600 vehicles/hour in each direction; and			
	(ii) The minor road exceeds 200 vehicles/hour in one direction			
Continuous Traffic	For each of the four one-hour periods of an average day:			
	(i) The major road flow exceeds 900 vehicles/hour in each direction; and			
	(ii) The minor road exceeds 100 vehicles/hour in one direction; and			
	(iii) The speed of traffic on the major road or limited sight distance from the minor road causes undue delay/hazards to the minor road vehicles; and			
	(iv) There is no other nearby traffic signal site easily accessible to the minor road vehicles			

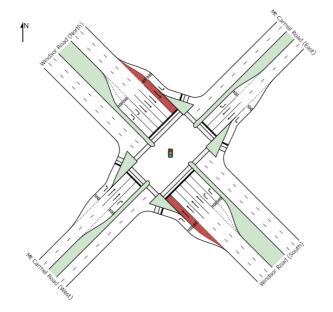
Warrant	Requirements		
Pedestrian Safety	For each of the four one-hour periods of an average day:		
	(i) The pedestrian flow crossing the major road exceeds 150 persons/hour; and		
	(ii) The major road exceeds 600 vehicles/hour in each direction or, where there is a central median at least 1.2m wide, 1000 vehicles/hour in each direction		
Pedestrian Safety –	For each of the four one-hour periods of an average day:		
high speed road	(i) The pedestrian flow crossing the major road exceeds 150 persons/hour; and		
	(ii) The major road exceeds 450 vehicles/hour in each direction or, where there is a central median at least 1.2m wide, 750 vehicles/hour in each direction; and		
	(iii) The 85 th percentile speed on the major road exceeds 75km/hr		
Crashes	(i) The intersection has been the site of an average three or more reported tow-away or casualty traffic accidents per year over a three year period, where traffic signals could have prevented the accidents; and		
	(ii) The traffic flows are at least 80% of the appropriate flow warrants		

Source: Roads and Maritime Services

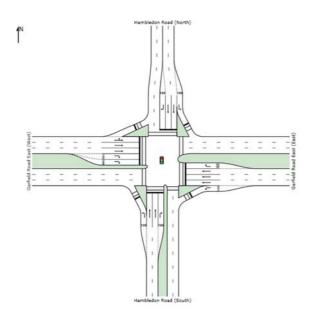
The installation of traffic signals at each of the locations identified in the Riverstone East precinct will be staged based on the development of the precinct and following demonstration that the above warrants may be met. This will be considered during detailed planning for the precinct as the land uses are more accurately defined. Access points and the means of traffic control at those access points will need to be discussed and agreed with Roads and Maritime Services and Blacktown City Council.

Indicative intersection configurations were developed based on the outputs of the traffic model and future turning movements. These proposed layouts are illustrated in Figure 23. The identified works for intersections along Windsor Road are largely consistent with those outlined in the transport strategies for the Box Hill and Area 20 precincts. During the further detailed planning for the precinct, these intersection configurations will be refined as the layout of the area becomes more accurately defined.

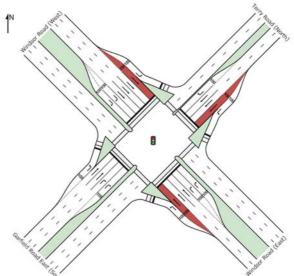
It should be noted that the existing Clarke Street – Garfield Road East intersection is to be closed and to become a cul-de-sac.



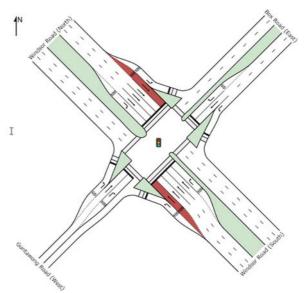




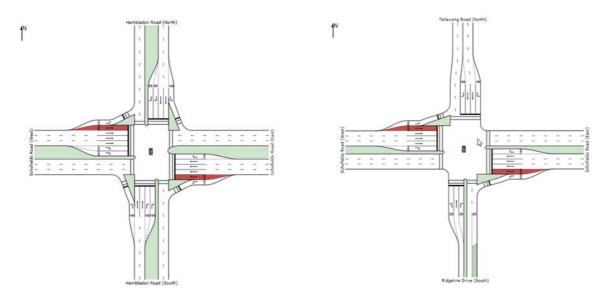
Garfield Road / Hambledon Road



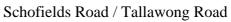
Windsor Road / Garfield Road

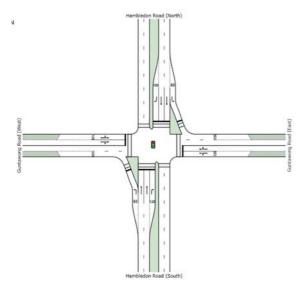


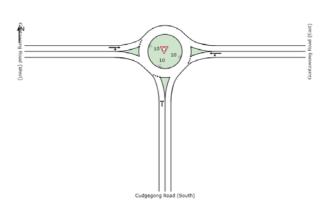
Windsor Road / Guntawong Road



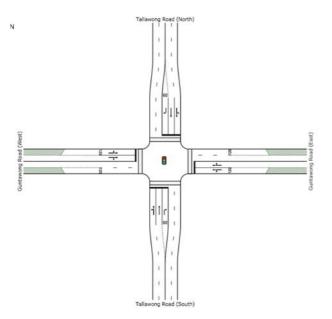
Schofields Road / Hambledon Road

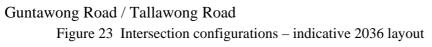






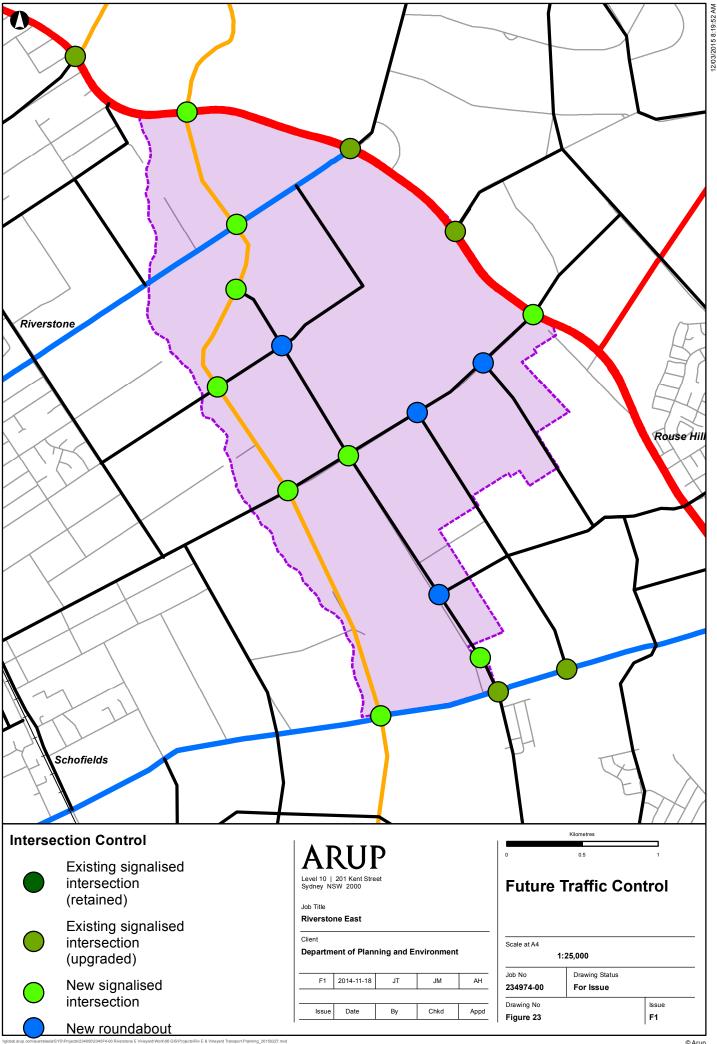
Guntawong Road / Hambledon Road





The recommended traffic controls (at intersections) to support the growth of the Riverstone East precinct is illustrated in Figure 24 on the following page

Guntawong Road / Cudgegong Road



The results of the traffic modelling, based on the criteria previously described in Section 5.1 of this report, are summarised in Table 14. These results demonstrate, based on the recommended configurations, the intersections will operate satisfactorily during both the AM and PM peak hours for the forecast year 2036. Detailed outputs of the traffic modelling undertaken for this study are provided in Appendix B.

Intersection	Peak Hour	LoS	Av Delay (s)	DoS
Garfield Road / Hambledon Road	AM	С	35	0.90
	PM	С	40	0.85
Windsor road / Garfield Road	AM	D	54	0.94
	PM	D	53	0.90
Guntawong Road / Tallawong Road	AM	В	19	0.98
	PM	В	16	0.72
Guntawong Road / Hambledon Road	AM	С	39	0.81
	PM	С	35	0.73
Guntawong Road / Cudgegong Road	AM	А	6	0.44
	PM	А	6	0.40
Schofields Road / Hambledon Road	AM	С	37	0.76
	PM	С	41	0.77
Windsor Road / Mt Carmel Road	AM	С	37	0.78
	PM	С	37	0.75
Tallawong Road / Hambledon Road	AM	В	17	0.65
	PM	В	16	0.61
Windsor Road / Guntawong Road	AM	D	44	0.82
	PM	D	53	0.88
Schofields Road / Tallawong Road	AM	С	39	0.85
	PM	Е	57	1.00

Table 14Traffic Modelling Results

DOS - Degree of Saturation, Av Delay - Average Vehicle Delay, LOS - Level of Service

5.8 Heavy Vehicle Movements

The future residential and commercial/retail uses within the Riverstone East Precinct are not excepted to generate significant heavy vehicle movements. Heavy vehicles associated with the nearby Riverstone West precinct will travel on the higher order road network (Windsor Road, Schofields Road and Garfield Road East) to access their final destination.

Should existing light industrial uses continue to operate within the core of the Riverstone East Precinct, measures which ameliorate the amenity issues arising from heavy vehicle movements will need to be considered. This may include (but is not limited to):

- Implementation of heavy vehicle load limits on certain roads;
- Positioning the frontage of residential lots away from roads expected to carry heavy vehicle traffic;
- Providing noise attenuation devices on local roads to reduce the noise impacts arising from heavy vehicles; and
- Introducing local area traffic management (LATM) devices, e.g. raised speed tables, to discourage heavy vehicle intrusion into the precinct.

6 Public Transport, Walking and Cycling

6.1 Bus Network

6.1.1 **Proposed Network of Routes**

The North West Sector Bus Servicing Plan, produced in 2012 by McCormick Rankin Cagney for NSW Transport and Infrastructure, provides for a future bus network to service the entire North West Growth Centre. The proposed network consists of five regional and twelve district routes, as described below:

- Regional routes are high frequency services intended to connect town and regional centres. These routes were planned to ensure 90% of residents of the NWGCC are within 800m of a service.
- District bus services are less frequent that typically run during the day only, providing further accessibility to village centres and extending bus service provision to the widest area practically possible.

The proposed bus network plan to service the Riverstone East precinct (illustrated in Figure 25) builds upon the principles of the North West Sector Bus Servicing Plan (illustrated in Figure 26). This has been modified based on the proposed road network layout, maximising the potential patronage and coverage – ensuring the significant majority of the Riverstone East precinct is located within 400m of a bus route.

The bus network proposes one regional and two district routes which run directly within the Riverstone East precinct, as described below:

• Route R1: Parramatta – Rouse Hill – Box Hill – Riverstone – Marsden Park

Within the Riverstone East precinct, regional route R1 runs east-west along Garfield Road and the extension of Tallawong Road – providing a connection between Windsor Road and Riverstone railway station. This service provides a link between Marsden Park to the west of the precinct, and Box Hill to the east. Additionally, the service provides wider linkages to Rouse Hill and Parramatta town centres.

• Route D3: Rouse Hill to Box Hill

This service provides a north-south connection through the Riverstone East precinct along Tallawong Road. This route provides the most direct link to the Box Hill town centre via Terry Street, and the Rouse Hill town centre via Cudgegong Road Station and Rouse Road.

• Route D7: Riverstone – Schofields

District route D7 runs along Hambledon Road on the western boundary of the Riverstone East precinct (via Victoria Street in the Riverstone precinct). The service provides a connection between the Riverstone East, Cudgegong Road and Schofields railway stations.

In addition to the above service, a further three bus routes run directly adjacent to the Riverstone East precinct:

• Route R2: Parramatta – Rouse Hill – Riverstone East – Vineyard

This route runs along the Schofields Road transit boulevard on the southern boundary of the Riverstone East precinct. It is envisaged a bus stop will be provided on Schofields Road directly adjacent to the new Cudgegong Road railway station to accommodate passengers interchanging between bus and rail.

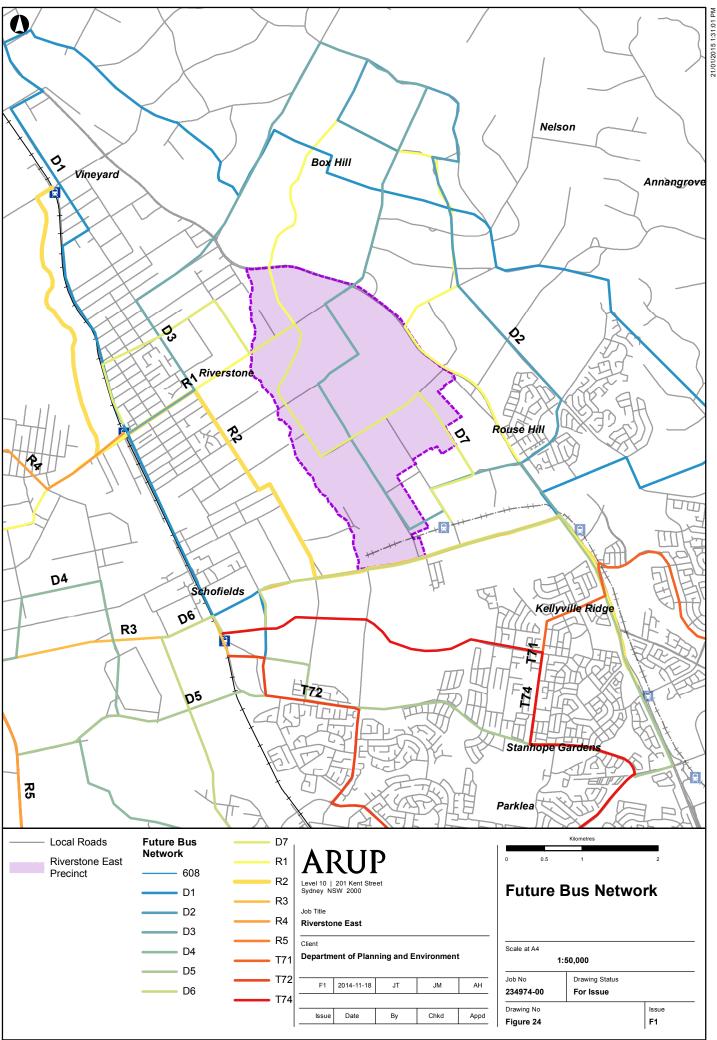
• Route R3: Mount Druitt – Marsden Park – Schofields – Rouse Hill

Similar to Route R2, this service runs along the Schofields Road between Rouse Hill and Schofields.

• Route 608: Windsor to Rouse Hill

This is an existing bus route running along Windsor Road through the Vineyard precinct. It provides a connection from Windsor and Mulgrave into Rouse Hill and east of the Riverstone East precinct. The North West Sector Bus Servicing Plan envisages that this service will be upgraded to a regional route depending on future demand.

An illustration of the future bus network serving the Riverstone East precinct is shown in Figure 25 on the following page.



Vglobal.arup.com/australasia/SYD/Projects/234000/234974-00 Riverstone E Vineyard/Work106 GIS/Projects/Riv E & Vineyard Transport Planning_20150901.mx

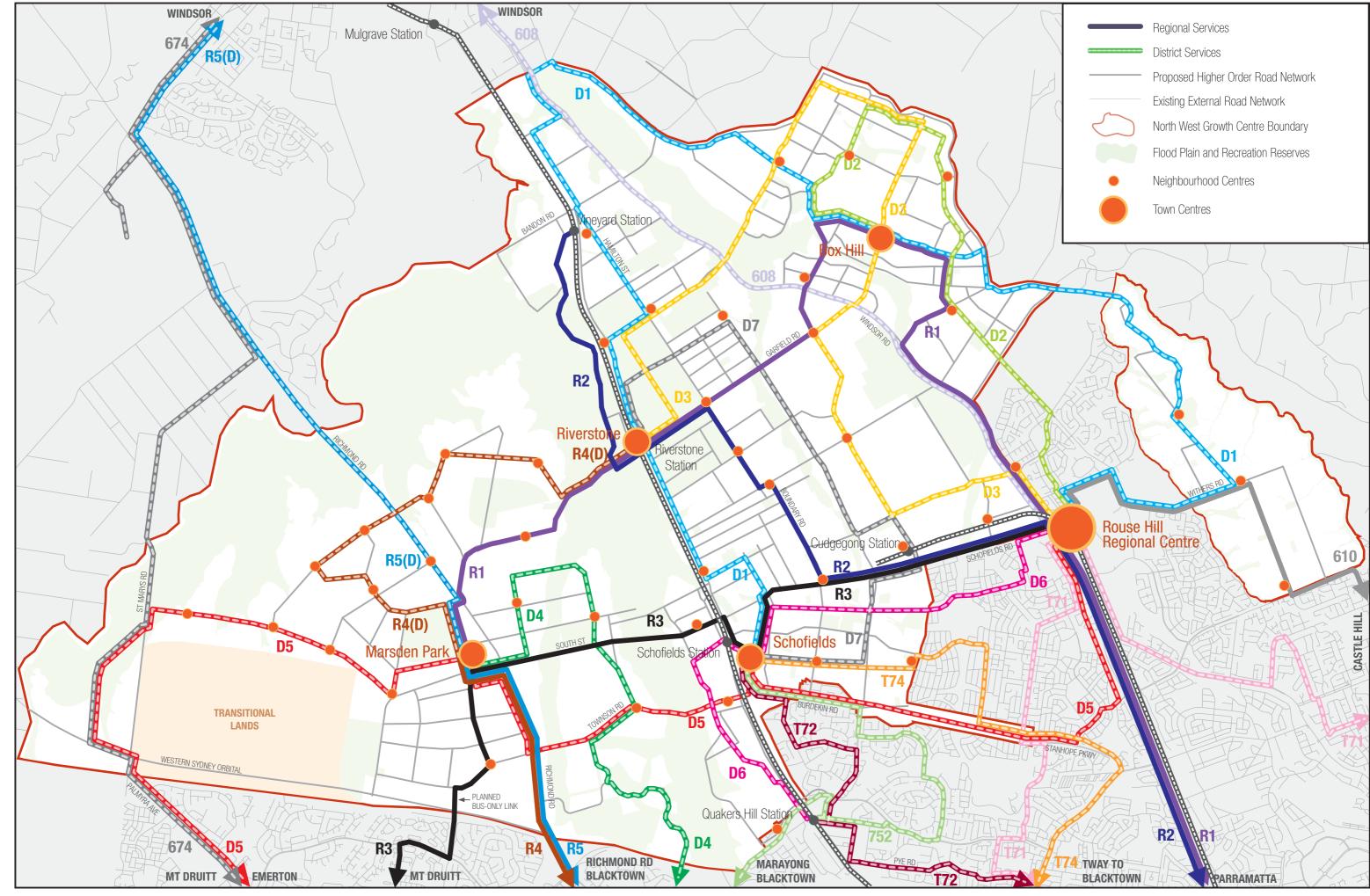


Figure 25: North West Growth Centre - Bus Servicing Strategy Source: MRCagney, 2012

6.1.2 Bus Servicing

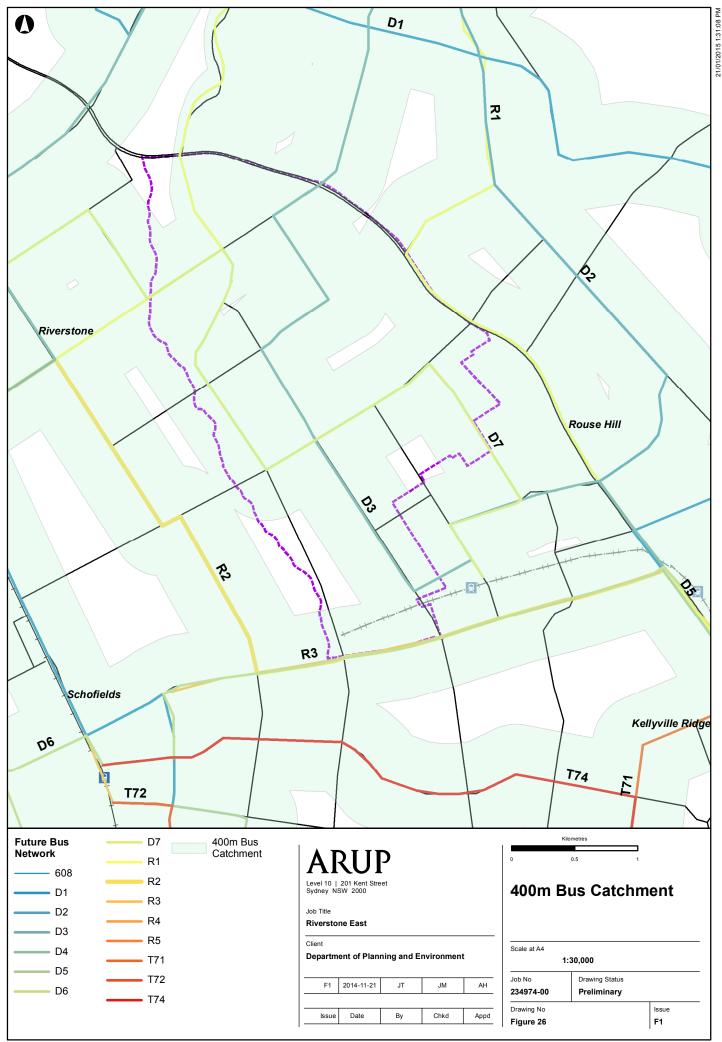
All routes identified as bus corridors would be required to provide a road reserve of sufficient width to accommodate bus stops, including shelters where appropriate. Lane widths for bus routes would need to be a minimum of 3.5m. Roads which accommodate bus routes are required to have two lanes in each direction so stopping buses will not adversely impact general traffic. Should it not be feasible to provide two traffic lanes, indented bus bays may be considered – although this option limits flexibility should bus stops need to be moved in the future.

Schofields Road and Garfield Road have been identified as transit boulevards and therefore critical links to support the regional bus network. Bus priority facilities (including bus lanes and bus jump stars) may be required at all signalised intersections to enable buses to maintain journey times and improve the attractiveness of public transport.

Bus stops should be provided with a minimum spacing of 400m to provide good accessibility to workers and residents of the Riverstone East precinct. This bus stop spacing will maintain route speeds and minimise the impact of stopping buses on general traffic flows. The exact locations of bus stops within Riverstone East will be identified during detailed planning for the precinct as the land uses are more accurately defined. As illustrated in Figure 27, the significant majority of the Riverstone East precinct is situated within 400m of bus route.

Bus stop locations will be identified and approved through the Local Traffic Committee at the earliest opportunity during the precinct planning phase. As a general principle, bus stops should be located to provide good access to key land uses including Cudgegong Road railway station, Area 20 town centre, Rouse Hill Regional Park and local schools.

Amenities for waiting passengers, including shading, seating and shelters, is recommended at bus stops located on key regional routes such as Schofields Road, Garfield Road and Tallawong Road. All bus stops should meet the latest requirements by the Disability Standards for Accessible Public Transport (DSAPT) and the Disability Discrimination Act (DDA).



A4

6.1.3 Future Bus Services

Sydney's Bus Future

In December 2013 Transport for NSW released the Sydney's Bus Future document, outlining a long term plan for bus services across Sydney. The bus plan indicates two future bus services within the North West Growth Centre (Figure 28). A rapid bus route is envisaged which will run between Rouse Hill and Blacktown along Schofields Road. A suburban bus route is planned between Penrith and Rouse Hill, again running along the Schofields Road transit boulevard.

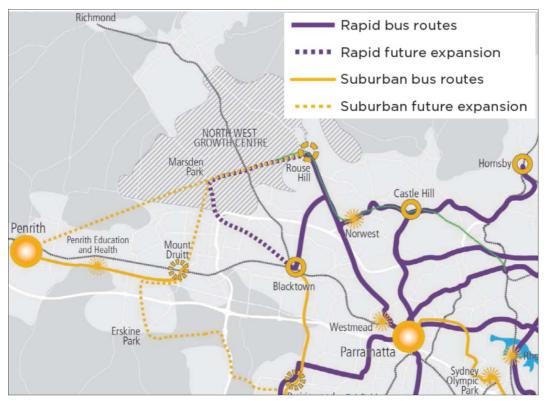


Figure 28 Sydney's Future Bus Network

Source: Transport for NSW, 2013

6.2 Rail Services

The Riverstone East precinct is well served by the Sydney Trains network via Riverstone Station on the Richmond Line and the Cudgegong Road Station on the future North West Rail Link. Providing strong access to these rail stations through a permeable road and pedestrian network will be a key component in facilitating a mode shift away from private vehicle to public transport.

In December 2012 the NSW Government announced a public transport corridor had been preserved in the NWGC for future generations. Known as the Marsden Park Transport Corridor, the route runs from the end of North West Rail Link at Cudgegong Road Station, west to Schofields Station and then further on to Marsden Park, a distance of approximately 6.8km. The indicative alignment of the corridor is shown in Figure 29.



Figure 29 Marsden Park transport corridor Source: Transport for NSW, 2013

Transport for NSW is currently undertaking detailed planning to define the alignment of the corridor – including the interface with existing and planned roads. A suitable public transport mode has not yet been determined, however the route of the corridor will be designed to accommodate a range of feasible modes including heavy rail, bus or light rail. The decision on the mode of selected transport mode will be based on detailed analysis to ensure the most appropriate mode is selected that best serves the corridor and the region in the longer term.

The NSW Government has postponed the second stage of the duplication of the Richmond Railway Line. This project, which would upgrade the existing tracks between Schofields and Vineyard stations, was initially announced in 2003 to increase capacity on the existing network. The project would benefit residents of the precinct through the increased provision of rail services from Riverstone station and an upgrade of the existing station– increasing the attractiveness of rail as a form of transport.

6.3 Pedestrian and Cycling Network

The future design objectives of the local area pedestrian and cyclist networks for the Riverstone East precinct should be to encourage residents, wherever possible to walk or cycle in preference to using motor vehicles for locally based travel and for access to public transport.

A number of intersection upgrades have been recommended in this strategy which will provide controlled pedestrian crossings across major roads – improving accessibility. Within the future neighbourhood centre, anticipated to be along Guntawong Road, additional pedestrian facilities such as zebra crossings and/or widened footpaths may be required to support the higher levels of pedestrian activity anticipated in this area.

Dedicated bicycle facilities (both on road and off road) are proposed to provide linkages to key land uses within Riverstone East and surrounding precincts. These land uses including Cudgegong Road railway station, Area 20 town centre, Rouse Hill Regional Park, schools and open space areas.

The bicycle network serving the Riverstone East precinct should be designed in accordance with the design principles and objectives as outlined in Austroads and NSW Bicycle Guidelines (refer Section 4.6.2) of this document. In particular, the precinct should provide a network of safe and connected bicycle paths which, where possible and appropriate, provide physical separation from vehicles and pedestrians.

A shared path already exists along the western side of Windsor Road. Garfield Road and Schofields Road have been identified as transit boulevards within the preferred road network hierarchy, and therefore include shared pedestrian / cycle paths on both sides of the street. A link along the southern side of Garfield Road will be important to facilitate bicycle movements between Windsor Road and the Riverstone town centre. On the northern side of Garfield Road, the open space area will provide for movements to the west of the Riverstone East precinct.

In addition, the following roads in Riverstone East have been identified as suitable corridors for the introduction of shared paths to improve the cycling network:

- Tallawong Road;
- Guntawong Road; and
- Cudgegong Road

There is the opportunity to provide a shared pedestrian/cycle path which links the existing Gordon Road within Riverstone, across First Ponds Creek into the Riverstone East precinct. This path would provide a connection to Cudgegong Road railway station and the Area 20 town centre.

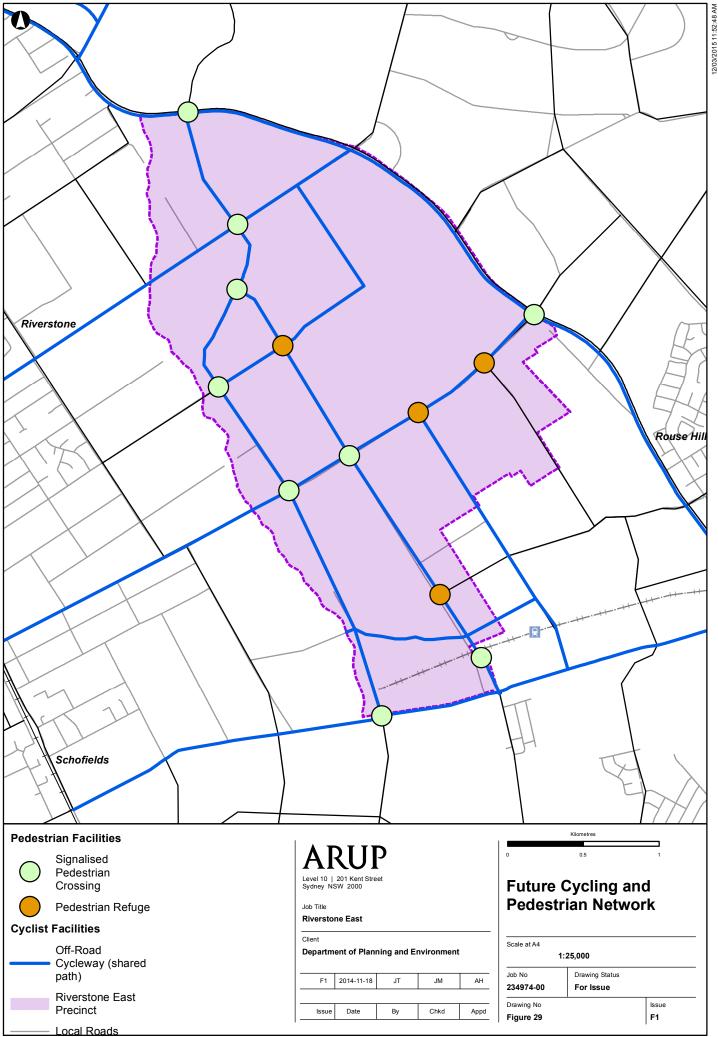
As previously noted, separated bicycle paths (either shared or dedicated path) are recommended on local and collector roads that have traffic volumes of more than 5,000 vehicles per day on a 60 km/h road, as well as roads serving certain land uses including schools and open space areas. Local roads which carry less than 5,000 vehicles per day will not necessitate the provision of a separated bicycle path, however should be designed to provide for the safe movement of cyclists. This may be achieved through lower speeds limits or advisory treatments such as pavement markings, warning signs and/or guide signs. Advisory treatments are used to indicate or advise road users of the potential presence of cyclists and of the location where cyclists may be expected to ride on a road.

It is likely a network of bicycle paths will be provided within Rouse Hill Regional Park. These paths should provide good connectivity with the wider bicycle network, specifically linkages to Windsor Road, Garfield Road and Guntawong Road. The location and scale of this shared path network within Rouse Hill Regional Park will be determined by the Office of Environment and Heritage.

Within any medium and high density residential development, or commercial development within the precinct, secure bicycle parking is recommended to be provided. Bicycle parking should be provided at rates consistent with those outlined in the *NSW Planning Guidelines for Walking and Cycling*. 45 bicycle parking spaces are planned at the Cudgegong Road railway station.

Visitor bicycle parking should be provided in the public domain with the village centre to support local bicycle trips to retail shops.

Figure 30 illustrates the recommended pedestrian and cyclist measures.



Viglobal.arup.com/australasia/S YD/Projects/234000/234974-00 Rivers.tone E Vineyard/Work/06 GIS/Projects/Riv E & Vineyard Transport Planning_20150227.mxx

7 Summary and Conclusions

This report has assessed the future traffic and transport requirements associated with the development of the Riverstone East precinct - located in the eastern, central portion of the North West Growth Centre, wholly within the Blacktown Local Government Area. The precinct ultimately has the capacity to accommodate approximately 5,600 residential dwellings, 10,880m² light industrial floor space and additional retail and commercial uses.

This report has provided an assessment of the future transport infrastructure components necessary to support the development of the precinct, based on a series of key objectives. These are summarised below:

- Provide a road network that allows for good access to all modes of transport;
- Design a physical site layout which encourages walking and cycling;
- Ensure the road network provides suitable connections to adjacent development precincts;
- Integrate transport and land use planning;
- Provide high quality access to public transport stops to reduce the dependence on private vehicles;
- Develop an appropriate road hierarchy which provides adequate carrying capacity; and
- Protect residential areas from through traffic intrusion, particularly heavy vehicles.

Road Network

A strategic transport network model was developed to forecast future year (2036) traffic volumes in the vicinity of the Riverstone East precinct. Based on the traffic modelling outputs, a road network hierarchy was developed to support the future land uses envisaged for Riverstone East. Windsor Road is currently classified as arterial roads and will support the majority of regional traffic movements. Schofields Road and Garfield Road have been identified as transit boulevards and therefore critical links to support regional traffic and bus movements.

A number of existing internal roads have been identified for improvement and form the framework for the higher-order (sub-arterial / collector) road network. These roads will accommodate internal traffic, bicycle and pedestrian movements, and include:

- Hambledon Road;
- Tallawong Road;
- Cudgegong Road; and
- Guntawong Road.

A number of intersections within Riverstone East were analysed using the SIDRA 6 modelling package to assess their future peak-hour operating performance. Traffic signals are recommended to be installed at eight intersections within the precinct, with a further three intersections currently containing traffic signals identified to be upgraded. The results of the traffic modelling demonstrate, based on the recommended configurations, the intersections will operate satisfactorily during both the AM and PM peak hours for the forecast year 2036.

Public Transport, Walking and Cycling

For new precincts within the North West Growth Centre, where private vehicle has historically dominated the transport landscape, it is vital that strong public transport, walking and cycling linkages are provided at an early stage when new residents establish their travel habits.

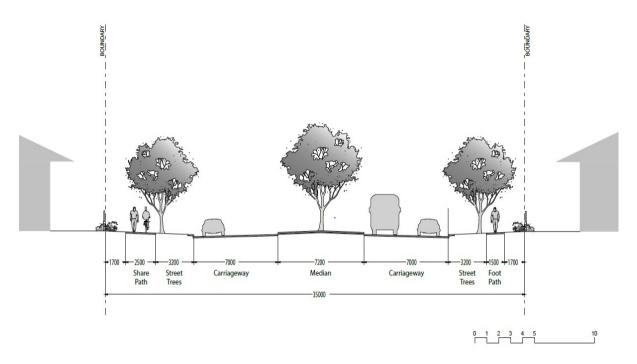
The Riverstone East precinct will be served by a number of new bus routes proposed as part of the North West Sector Bus Servicing Plan. Bus stops are recommended to be provided with a minimum spacing of 400m to provide good accessibility to workers and residents. Amenities for waiting passengers, including shading, seating and shelters, is recommended at bus stops located on key regional routes such as Tallawong Road, Schofields Road, Garfield Road East and Cudgegong Road.

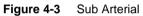
Future residents and employees of the Riverstone East precinct will benefit from improved accessibility across major roads through the intersection upgrades (and complementary controlled pedestrian crossing) which have been recommended in this strategy. Footpaths should be provided on both sides of the road carriageway in accordance with the standard road cross sections for the North West Growth Centre.

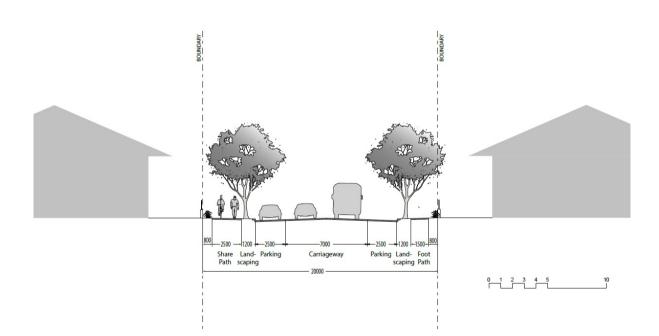
A number of dedicated bicycle routes have been identified in association with the road upgrade works recommended in this strategy. Shared pedestrian/cycle paths (minimum 3m wide) are recommended for major roads including Tallawong Road, Cudgegong Road and Garfield Road East. This will provide connections to the regional bicycle network, in particular the existing shared path along Windsor Road.

There is the opportunity to provide a shared pedestrian and cycle path which links the existing Gordon Road within Riverstone, across First Ponds Creek into the Riverstone East precinct. This path would provide a connection to Cudgegong Road railway station and the Area 20 town centre. Appendix A

Typical Road Cross Sections

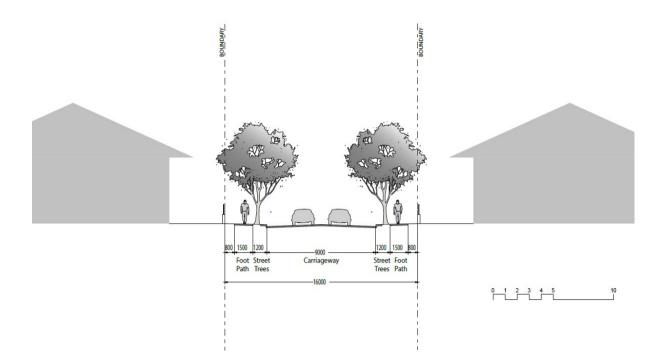






Collector (Bus Route) - 20m

Figure 4-4 Collector Road (Bus Route)





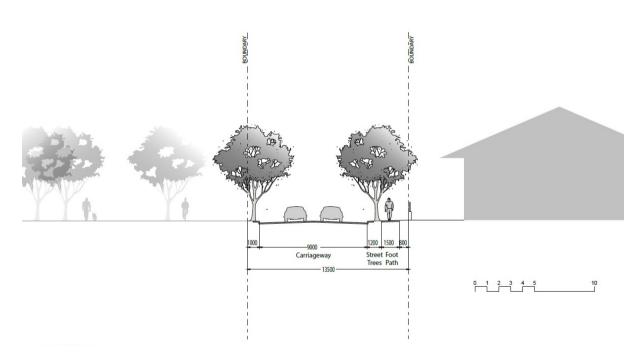
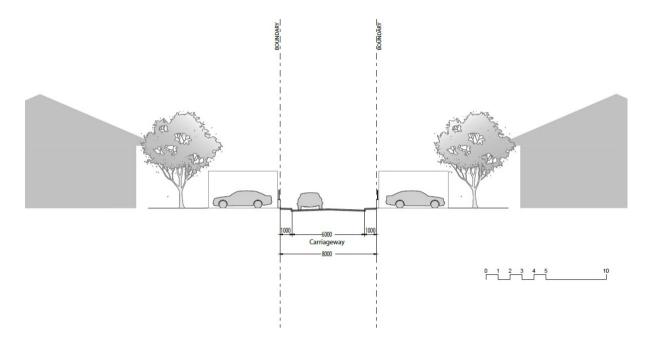
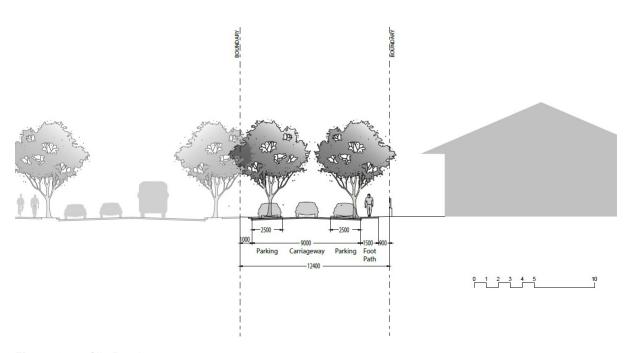
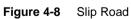


Figure 4-6 Local Street along Riparian Corridors/Parks







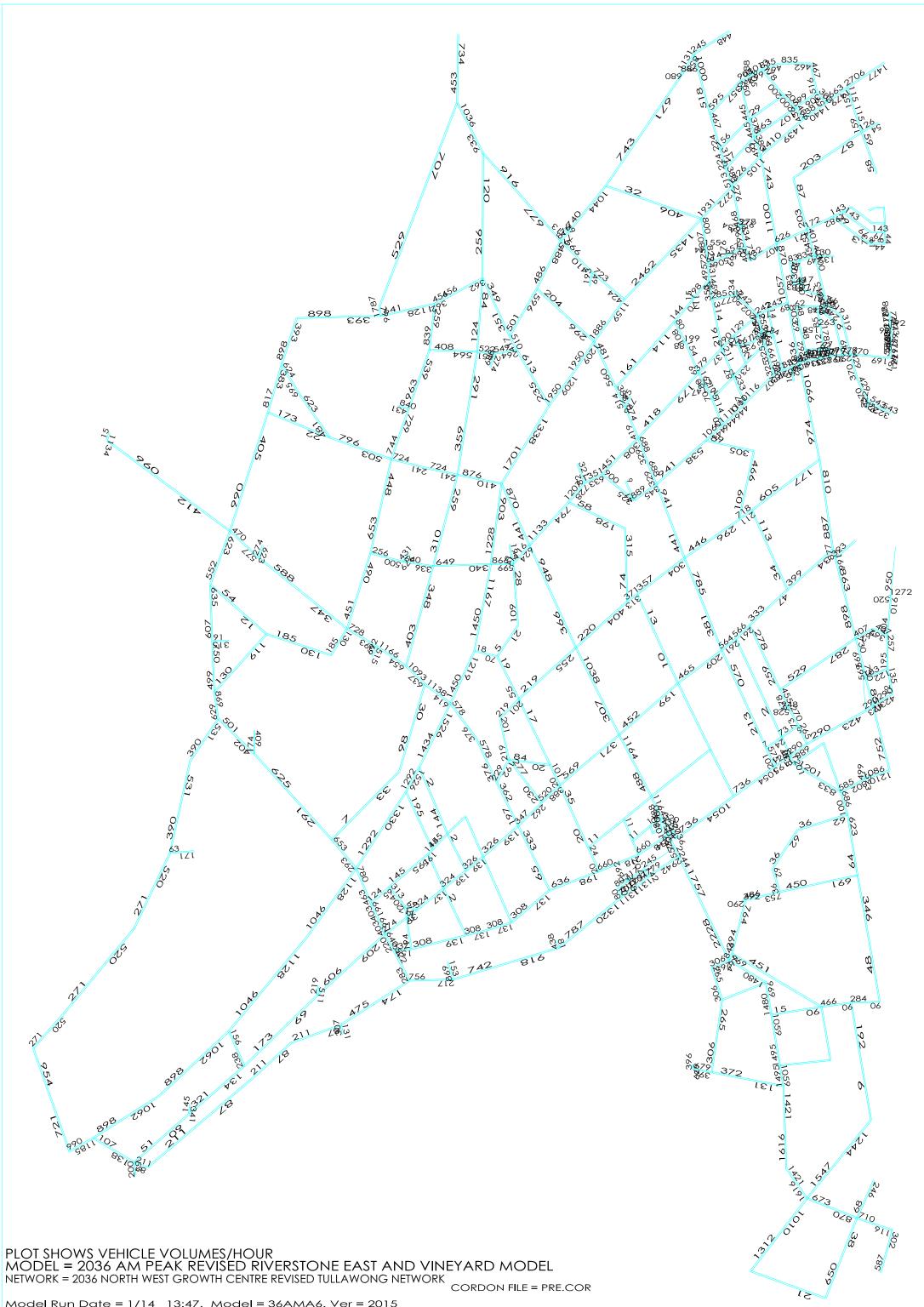


Appendix B

Traffic Model Outputs



Model Run Date = 1/14 13:50. Model = 36PMA6. Ver = 2015 X Scale Factor = 1. Y Scale Factor = 1. Plot File = 36PMA6.XCC. Network = 2036A6. O/D = 36PM14.



Model Run Date = 1/14 13:47. Model = 36AMA6. Ver = 2015 X Scale Factor = 1. Y Scale Factor = 1. Plot File = 36AMA6.XCC. Network = 2036A6. O/D = 36AM14.

Site: Schofields / Tallawong AM

New Site

Signals - Fixed Time Cycle Time = 100 seconds (Practical Cycle Time)

Mov OD ID Demand Flows Total Deg. HV Average Satn Level of Delay 95% Back of Queue Vehicles Distance Distance 30 L2 88 0.0 0.104 21.8 LOS B 2.4 16.9 31 T1 499 0.0 0.846 38.8 LOS C 24.5 171.7 32 R2 24 0.0 0.093 46.7 LOS D 1.0 7.3 Approach 612 0.0 0.846 36.6 LOS C 24.5 171.7 East: Schofields Road (East) 21 L2 57 0.0 0.073 24.0 LOS B 1.6 11.4 22 T1 684 1.0 0.526 37.6 LOS C 9.9 69.1 23 R2 74 0.0 0.661 60.0 LOS E 3.9 27.0	Prop. Effective Average
veh/h % v/c sec veh m South: Ridgeline Drive (South) 30 L2 88 0.0 0.104 21.8 LOS B 2.4 16.9 31 T1 499 0.0 0.846 38.8 LOS C 24.5 171.7 32 R2 24 0.0 0.093 46.7 LOS D 1.0 7.3 Approach 612 0.0 0.846 36.6 LOS C 24.5 171.7 East: Schofields Road (East) 21 L2 57 0.0 0.073 24.0 LOS B 1.6 11.4 22 T1 684 1.0 0.526 37.6 LOS C 9.9 69.1	
South: Ridgeline Drive (South) 30 L2 88 0.0 0.104 21.8 LOS B 2.4 16.9 31 T1 499 0.0 0.846 38.8 LOS C 24.5 171.7 32 R2 24 0.0 0.093 46.7 LOS D 1.0 7.3 Approach 612 0.0 0.846 36.6 LOS C 24.5 171.7 East: Schofields Road (East) 21.0 0.846 36.6 LOS C 24.5 171.7 21 L2 57 0.0 0.073 24.0 LOS B 1.6 11.4 22 T1 684 1.0 0.526 37.6 LOS C 9.9 69.1	
30 L2 88 0.0 0.104 21.8 LOS B 2.4 16.9 31 T1 499 0.0 0.846 38.8 LOS C 24.5 171.7 32 R2 24 0.0 0.093 46.7 LOS D 1.0 7.3 Approach 612 0.0 0.846 36.6 LOS C 24.5 171.7 East: Schofields Road (East) 21 L2 57 0.0 0.073 24.0 LOS B 1.6 11.4 22 T1 684 1.0 0.526 37.6 LOS C 9.9 69.1	n per veh km/h
31 T1 499 0.0 0.846 38.8 LOS C 24.5 171.7 32 R2 24 0.0 0.093 46.7 LOS D 1.0 7.3 Approach 612 0.0 0.846 36.6 LOS C 24.5 171.7 East: Schofields Road (East) 21 L2 57 0.0 0.073 24.0 LOS B 1.6 11.4 22 T1 684 1.0 0.526 37.6 LOS C 9.9 69.1	
32 R2 24 0.0 0.093 46.7 LOS D 1.0 7.3 Approach 612 0.0 0.846 36.6 LOS C 24.5 171.7 East: Schofields Road (East) 21 L2 57 0.0 0.073 24.0 LOS B 1.6 11.4 22 T1 684 1.0 0.526 37.6 LOS C 9.9 69.1	
Approach 612 0.0 0.846 36.6 LOS C 24.5 171.7 East: Schofields Road (East) 21 L2 57 0.0 0.073 24.0 LOS B 1.6 11.4 22 T1 684 1.0 0.526 37.6 LOS C 9.9 69.1	0.95 0.95 36.9
East: Schofields Road (East) 21 L2 57 0.0 0.073 24.0 LOS B 1.6 11.4 22 T1 684 1.0 0.526 37.6 LOS C 9.9 69.1	3 0.90 0.70 34.0
21 L2 57 0.0 0.073 24.0 LOS B 1.6 11.4 22 T1 684 1.0 0.526 37.6 LOS C 9.9 69.1	0.90 0.91 37.6
22 T1 684 1.0 0.526 37.6 LOS C 9.9 69.1	
	4 0.63 0.69 42.7
23 R2 74 0.0 0.661 60.0 LOSE 3.9 27.0	0.93 0.77 37.2
) 1.00 0.81 30.1
Approach 815 0.8 0.661 38.7 LOS C 9.9 69.1	0.92 0.77 36.8
North: Tallawong Road (North)	
24 L2 139 0.0 0.163 22.3 LOS B 3.9 27.4	4 0.62 0.73 43.0
25 T1 574 0.0 0.660 29.1 LOS C 18.2 127.5	5 0.87 0.75 40.8
26 R2 218 0.0 0.838 58.3 LOS E 11.7 81.7	7 1.00 0.96 30.8
Approach 931 0.0 0.838 34.9 LOS C 18.2 127.5	5 0.86 0.80 38.2
West: Schofields Road (West)	
27 L2 262 0.0 0.336 26.6 LOS B 8.6 60.1	0.72 0.77 41.5
28 T1 1063 1.0 0.818 45.2 LOS D 17.9 125.2	2 1.00 0.95 34.6
29 R2 92 0.0 0.822 63.5 LOS E 5.0 35.1	1.00 0.91 29.2
Approach 1417 0.8 0.822 42.9 LOS D 17.9 125.2	2 0.95 0.92 35.2
All Vehicles 3774 0.5 0.846 39.0 LOS C 24.5 171.7	7 0.91 0.85 36.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mov		Demand	Average	Level of	Average Back of	of Queue	Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P8	South Full Crossing	53	43.3	LOS E	0.1	0.1	0.93	0.93
P5	East Full Crossing	53	39.7	LOS D	0.1	0.1	0.89	0.89
P5S	East Slip/Bypass Lane Crossing	53	23.2	LOS C	0.1	0.1	0.68	0.68
P6	North Full Crossing	53	44.3	LOS E	0.1	0.1	0.94	0.94
P7	West Full Crossing	53	39.7	LOS D	0.1	0.1	0.89	0.89
P7S	West Slip/Bypass Lane Crossing	53	23.2	LOS C	0.1	0.1	0.68	0.68
All Pedestrians		316	35.6	LOS D			0.84	0.84



Site: Schofields / Tallawong PM

New Site

Signals - Fixed Time Cycle Time = 120 seconds (Optimum Cycle Time - Minimum Delay)

Mover	nent Perfo	rmance - Vehic	cles								
Mov	OD	Demano	Flows	Deg.	Average	Level of	95% Back o	f Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0		veh/h	%	v/c	sec		veh	m		per veh	km/h
	Ridgeline Dr	· ,									
30	L2	36	0.0	0.043	25.0	LOS B	1.1	8.0	0.59	0.68	41.7
31	T1	297	0.0	0.496	34.6	LOS C	13.8	96.5	0.85	0.72	38.5
32	R2	21	0.0	0.052	45.5	LOS D	1.0	6.8	0.82	0.69	34.4
Approa	ch	354	0.0	0.496	34.2	LOS C	13.8	96.5	0.82	0.72	38.5
East: S	chofields Ro	oad (East)									
21	L2	42	0.0	0.050	25.1	LOS B	1.4	9.5	0.59	0.68	42.2
22	T1	667	1.0	0.616	49.3	LOS D	12.1	84.4	0.97	0.81	33.3
23	R2	135	0.0	0.967	92.4	LOS F	10.1	70.6	1.00	1.10	23.8
Approa	ch	844	0.8	0.967	54.9	LOS D	12.1	84.4	0.96	0.85	31.6
North:	Tallawong R	oad (North)									
24	L2	73	0.0	0.087	25.5	LOS B	2.4	16.7	0.60	0.71	41.5
25	T1	719	0.0	0.971	65.4	LOS E	40.5	283.2	0.94	1.07	29.2
26	R2	400	0.0	0.994	100.7	LOS F	33.6	234.9	1.00	1.16	22.7
Approa	ch	1192	0.0	0.994	74.8	LOS F	40.5	283.2	0.94	1.08	27.1
West: S	Schofields R	oad (West)									
27	L2	246	0.0	0.295	27.8	LOS B	9.0	63.2	0.68	0.75	40.9
28	T1	718	1.0	0.663	49.8	LOS D	13.1	91.9	0.98	0.82	33.1
29	R2	61	0.0	0.438	65.7	LOS E	3.6	25.2	1.00	0.76	28.7
Approa	ch	1025	0.7	0.663	45.5	LOS D	13.1	91.9	0.91	0.80	34.4
All Vehi	icles	3415	0.4	0.994	56.9	LOS E	40.5	283.2	0.92	0.90	31.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

	nent Performance - Pedestrians							
Mov ID	Description	Demand	Average	Level of	Average Back of		Prop.	Effective
ID	Description	Flow ped/h	Delay sec	Service	Pedestrian ped	Distance m	Queued	Stop Rate per ped
P8	South Full Crossing	53	53.3	LOS E	0.2	0.2	0.94	0.94
P5	East Full Crossing	53	45.2	LOS E	0.2	0.2	0.87	0.87
P5S	East Slip/Bypass Lane Crossing	53	28.8	LOS C	0.1	0.1	0.69	0.69
P6	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P7	West Full Crossing	53	45.2	LOS E	0.2	0.2	0.87	0.87
P7S	West Slip/Bypass Lane Crossing	53	28.8	LOS C	0.1	0.1	0.69	0.69
All Pedestrians		316	42.6	LOS E			0.84	0.84



Site: Schofields / Hambledon AM

Existing Site

Signals - Fixed Time Cycle Time = 110 seconds (Practical Cycle Time)

30 L2		nd Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles	f Queue Distance	Prop.	Effective	Average
South: Hamble 30 L2	veh/h don Road (South) 122	%			Service	Vehicles	Dictoroo			
30 L2	don Road (South) 122		v/c	sec			Distance	Queued	Stop Rate	Speed
30 L2	122	0.0				veh	m		per veh	km/h
		0.0								
	405	0.0	0.148	24.8	LOS B	3.8	26.8	0.63	0.72	42.3
31 T1	405	0.0	0.357	33.2	LOS C	8.6	60.3	0.84	0.70	39.1
32 R2	74	0.0	0.273	60.3	LOS E	2.0	13.8	0.98	0.73	30.2
Approach	601	0.0	0.357	34.8	LOS C	8.6	60.3	0.81	0.70	38.3
East: Schofield	s Road (East)									
21 L2	163	0.0	0.261	34.1	LOS C	6.3	44.4	0.77	0.76	38.3
22 T1	615	1.0	0.327	30.9	LOS C	8.3	57.8	0.81	0.67	40.0
23 R2	41	0.0	0.221	56.4	LOS D	2.1	14.7	0.96	0.73	31.2
Approach	819	0.8	0.327	32.8	LOS C	8.3	57.8	0.81	0.69	39.1
North: Hambled	lon Road (North)									
24 L2	112	0.0	0.135	24.7	LOS B	3.5	24.4	0.62	0.71	42.4
25 T1	633	0.0	0.558	35.7	LOS C	14.4	100.9	0.90	0.77	38.1
26 R2	186	0.0	0.690	63.8	LOS E	5.3	37.0	1.00	0.83	29.5
Approach	931	0.0	0.690	40.0	LOS C	14.4	100.9	0.89	0.77	36.4
West: Schofield	s Road (West)									
27 L2	71	0.0	0.113	32.4	LOS C	2.6	18.1	0.72	0.72	38.9
28 T1	1063	1.0	0.565	33.8	LOS C	15.7	109.7	0.89	0.76	38.7
29 R2	283	0.0	0.762	62.5	LOS E	8.0	56.3	1.00	0.89	29.6
Approach	1417	0.8	0.762	39.5	LOS C	15.7	109.7	0.90	0.79	36.5
All Vehicles	3767	0.4	0.762	37.4	LOS C	15.7	109.7	0.86	0.75	37.3

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mover	Movement Performance - Pedestrians										
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped			
P8	South Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95			
P8S	South Slip/Bypass Lane Crossing	53	36.1	LOS D	0.1	0.1	0.81	0.81			
P5	East Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95			
P5S	East Slip/Bypass Lane Crossing	53	29.2	LOS C	0.1	0.1	0.73	0.73			
P6	North Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95			
P6S	North Slip/Bypass Lane Crossing	53	36.1	LOS D	0.1	0.1	0.81	0.81			
P7	West Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95			
P7S	West Slip/Bypass Lane Crossing	53	29.2	LOS C	0.1	0.1	0.73	0.73			
All Ped	estrians	421	40.9	LOS E			0.86	0.86			

Site: Schofields / Hambledon PM

Existing Site

Signals - Fixed Time Cycle Time = 120 seconds (Practical Cycle Time)

Movement Performance - Vehicles											
Mov	OD	Demano	d Flows	Deg.	Average	Level of	95% Back o	f Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Coutbu		veh/h	%	v/c	sec		veh	m		per veh	km/h
		Road (South)									
30	L2	71	0.0	0.081	24.3	LOS B	2.2	15.7	0.58	0.69	42.6
31	T1	240	1.0	0.212	34.3	LOS C	5.3	37.2	0.80	0.64	38.7
32	R2	42	0.0	0.124	61.1	LOS E	1.2	8.2	0.95	0.70	30.1
Approa	ch	353	0.7	0.212	35.5	LOS C	5.3	37.2	0.77	0.66	38.1
East: S	chofields Ro	ad (East)									
21	L2	42	0.0	0.068	34.5	LOS C	1.7	11.6	0.71	0.70	38.1
22	T1	634	1.0	0.368	36.5	LOS C	9.7	67.8	0.84	0.70	37.7
23	R2	169	0.0	0.730	63.6	LOS E	10.1	70.9	1.00	0.86	29.4
Approa	ch	845	0.7	0.730	41.8	LOS C	10.1	70.9	0.87	0.73	35.7
North: I	Hambledon F	Road (North)									
24	L2	143	0.0	0.165	25.2	LOS B	4.8	33.3	0.61	0.72	42.2
25	T1	811	0.0	0.765	42.7	LOS D	23.8	166.7	0.96	0.86	35.6
26	R2	239	0.0	0.702	66.5	LOS E	7.2	50.7	1.00	0.84	28.9
Approa	ch	1193	0.0	0.765	45.4	LOS D	23.8	166.7	0.92	0.84	34.6
West: S	Schofields Ro	oad (West)									
27	L2	205	0.0	0.332	37.7	LOS C	9.0	62.7	0.80	0.78	36.9
28	T1	769	1.0	0.446	37.5	LOS C	12.1	84.6	0.87	0.73	37.3
29	R2	52	0.0	0.111	56.6	LOS E	1.4	9.5	0.92	0.71	31.1
Approa	ch	1026	0.7	0.446	38.5	LOS C	12.1	84.6	0.85	0.74	36.8
All Vehi	cles	3417	0.5	0.765	41.4	LOS C	23.8	166.7	0.87	0.77	35.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mover	Movement Performance - Pedestrians										
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped			
P8	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95			
P8S	South Slip/Bypass Lane Crossing	53	40.9	LOS E	0.1	0.1	0.83	0.83			
P5	East Full Crossing	53	51.5	LOS E	0.2	0.2	0.93	0.93			
P5S	East Slip/Bypass Lane Crossing	53	31.6	LOS D	0.1	0.1	0.73	0.73			
P6	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95			
P6S	North Slip/Bypass Lane Crossing	53	40.9	LOS E	0.1	0.1	0.83	0.83			
P7	West Full Crossing	53	51.5	LOS E	0.2	0.2	0.93	0.93			
P7S	West Slip/Bypass Lane Crossing	53	31.6	LOS D	0.1	0.1	0.73	0.73			
All Ped	estrians	421	44.6	LOS E			0.86	0.86			

Site: Windsor / Garfield AM

Existing Site

Signals - Fixed Time Cycle Time = 150 seconds (Optimum Cycle Time - Minimum Delay)

Movement Performance - Vehicles											
Mov	OD	Demand	Flows	Deg.	Average	Level of	95% Back o	f Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
OsuthE	a a tu M <i>lin</i> al a au	veh/h	%	v/c	sec		veh	m		per veh	km/h
		r Road (East)									
21	L2	434	0.0	0.556	29.1	LOS C	18.4	128.6	0.81	0.82	40.4
22	T1	1346	1.0	0.940	69.5	LOS E	58.1	406.7	1.00	1.07	28.2
23	R2	184	0.0	0.930	100.1	LOS F	7.9	55.1	1.00	0.99	22.8
Approa	ch	1964	0.7	0.940	63.4	LOS E	58.1	406.7	0.96	1.01	29.5
NorthEa	ast: Terry Ro	ad (North)									
24	L2	238	0.0	0.363	37.2	LOS C	10.6	74.5	0.78	0.83	37.1
25	T1	388	1.0	0.329	43.5	LOS D	10.8	75.6	0.82	0.69	35.3
26	R2	55	0.0	0.737	89.8	LOS F	4.3	30.3	1.00	0.82	24.5
Approa	ch	681	0.6	0.737	45.0	LOS D	10.8	75.6	0.82	0.75	34.6
NorthW	est: Windso	r Road (West)									
27	L2	64	0.0	0.082	28.2	LOS B	2.6	18.2	0.63	0.68	40.7
28	T1	685	1.0	0.305	35.0	LOS C	11.4	79.9	0.75	0.63	38.4
29	R2	164	0.0	0.829	90.4	LOS F	6.6	46.0	1.00	0.90	24.2
Approa	ch	914	0.8	0.829	44.5	LOS D	11.4	79.9	0.79	0.68	34.8
SouthW	/est: Garfield	d Road East (Soເ	uth)								
30	L2	263	0.0	0.354	38.9	LOS C	13.2	92.7	0.75	0.78	36.4
31	T1	365	0.0	0.312	43.1	LOS D	10.2	71.4	0.82	0.68	35.4
32	R2	102	0.0	0.687	89.0	LOS F	4.0	28.1	1.00	0.80	24.5
Approa	ch	731	0.0	0.687	48.0	LOS D	13.2	92.7	0.82	0.73	33.7
All Vehi	cles	4289	0.6	0.940	53.8	LOS D	58.1	406.7	0.88	0.85	32.0

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mover	nent Performance - Pedestrians							
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P5	SouthEast Full Crossing	53	66.4	LOS F	0.2	0.2	0.94	0.94
P5S	SouthEast Slip/Bypass Lane Crossing	53	48.1	LOS E	0.2	0.2	0.80	0.80
P6	NorthEast Full Crossing	53	39.7	LOS D	0.2	0.2	0.73	0.73
P6S	NorthEast Slip/Bypass Lane Crossing	53	30.1	LOS D	0.1	0.1	0.63	0.63
P7	NorthWest Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P7S	NorthWest Slip/Bypass Lane Crossing	53	48.1	LOS E	0.2	0.2	0.80	0.80
P8	SouthWest Full Crossing	53	42.7	LOS E	0.2	0.2	0.75	0.75
P8S	SouthWest Slip/Bypass Lane Crossing	53	30.1	LOS D	0.1	0.1	0.63	0.63
All Ped	All Pedestrians		46.8	LOS E			0.78	0.78

Site: Windsor / Garfield PM

Existing Site

Signals - Fixed Time Cycle Time = 145 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Perfo	rmance - Vehic	les								
Mov	OD	Demand	I Flows	Deg.	Average	Level of	95% Back o	f Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0 11 5	4 147 1	veh/h	%	v/c	sec		veh	m		per veh	km/h
		r Road (East)									
21	L2	87	0.0	0.136	26.2	LOS B	3.0	20.7	0.72	0.72	41.7
22	T1	1002	1.0	0.891	62.5	LOS E	39.3	275.3	0.99	1.00	29.8
23	R2	319	0.0	0.889	88.0	LOS F	12.6	88.5	1.00	0.97	24.6
Approa	ch	1408	0.7	0.891	66.0	LOS E	39.3	275.3	0.98	0.98	28.9
NorthEa	ast: Terry Ro	ad (North)									
24	L2	368	0.0	0.472	28.7	LOS C	14.7	102.7	0.77	0.81	40.5
25	T1	461	1.0	0.361	40.0	LOS C	12.2	85.5	0.81	0.69	36.5
26	R2	93	0.0	0.904	93.4	LOS F	7.5	52.4	1.00	0.97	23.9
Approa	ch	922	0.5	0.904	40.8	LOS C	14.7	102.7	0.81	0.76	36.0
NorthW	est: Windso	r Road (West)									
27	L2	91	0.0	0.136	32.8	LOS C	4.0	27.7	0.70	0.71	38.8
28	T1	969	1.0	0.541	45.3	LOS D	18.7	131.0	0.89	0.76	34.6
29	R2	233	0.0	0.649	76.3	LOS F	8.3	57.8	1.00	0.81	26.7
Approa	ch	1293	0.8	0.649	50.0	LOS D	18.7	131.0	0.90	0.77	33.1
SouthW	/est: Garfield	d Road East (Soເ	uth)								
30	L2	139	0.0	0.164	30.1	LOS C	5.7	39.6	0.62	0.72	39.9
31	T1	233	0.0	0.192	38.9	LOS C	6.0	41.7	0.77	0.63	36.9
32	R2	93	0.0	0.603	85.1	LOS F	3.5	24.4	1.00	0.77	25.2
Approa	ch	464	0.0	0.603	45.5	LOS D	6.0	41.7	0.77	0.68	34.5
All Vehi	cles	4087	0.6	0.904	52.9	LOS D	39.3	275.3	0.89	0.83	32.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians											
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped			
P5	SouthEast Full Crossing	53	63.9	LOS F	0.2	0.2	0.94	0.94			
P5S	SouthEast Slip/Bypass Lane Crossing	53	45.7	LOS E	0.2	0.2	0.79	0.79			
P6	NorthEast Full Crossing	53	47.3	LOS E	0.2	0.2	0.81	0.81			
P6S	NorthEast Slip/Bypass Lane Crossing	53	36.7	LOS D	0.2	0.2	0.71	0.71			
P7	NorthWest Full Crossing	53	66.8	LOS F	0.2	0.2	0.96	0.96			
P7S	NorthWest Slip/Bypass Lane Crossing	53	45.7	LOS E	0.2	0.2	0.79	0.79			
P8	SouthWest Full Crossing	53	50.6	LOS E	0.2	0.2	0.84	0.84			
P8S	SouthWest Slip/Bypass Lane Crossing	53	36.7	LOS D	0.2	0.2	0.71	0.71			
All Ped	All Pedestrians		49.2	LOS E			0.82	0.82			

Site: Windsor / Guntawong AM

New Site

Signals - Fixed Time Cycle Time = 135 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Perfo	rmance - Vehic	les								
Mov	OD	Demand	Flows	Deg.	Average	Level of	95% Back o	f Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
O a uth E		veh/h	%	v/c	sec		veh	m		per veh	km/h
		r Road (South)									
21	L2	326	0.0	0.349	26.9	LOS B	12.7	88.9	0.64	0.76	41.3
22	T1	1629	1.0	0.811	43.8	LOS D	34.4	240.9	0.96	0.88	35.0
23	R2	169	0.0	0.725	69.9	LOS E	11.3	78.8	1.00	0.85	27.8
Approa	ch	2125	0.8	0.811	43.3	LOS D	34.4	240.9	0.91	0.86	35.1
NorthEa	ast: Box Roa	ad (East)									
24	L2	28	0.0	0.038	30.9	LOS C	1.1	7.7	0.63	0.67	39.6
25	T1	21	0.0	0.046	42.1	LOS C	1.1	7.4	0.80	0.58	35.7
26	R2	21	0.0	0.109	65.7	LOS E	1.3	9.0	0.94	0.70	29.0
Approa	ch	71	0.0	0.109	44.6	LOS D	1.3	9.0	0.77	0.66	34.7
NorthW	/est: Windso	r Road (North)									
27	L2	145	0.0	0.155	24.6	LOS B	5.0	35.3	0.57	0.71	42.4
28	T1	1455	1.0	0.694	39.6	LOS C	26.9	188.3	0.92	0.81	36.4
29	R2	193	0.0	0.824	74.4	LOS F	13.5	94.3	1.00	0.91	26.9
Approa	ch	1793	0.8	0.824	42.1	LOS C	26.9	188.3	0.90	0.81	35.5
SouthW	/est: Guntaw	vong Road (West	t)								
30	L2	117	0.0	0.154	32.3	LOS C	4.8	33.5	0.67	0.73	39.0
31	T1	117	0.0	0.253	44.8	LOS D	6.2	43.6	0.85	0.69	34.8
32	R2	156	0.0	0.809	75.9	LOS F	10.9	76.3	1.00	0.91	26.8
Approa	ch	389	0.0	0.809	53.5	LOS D	10.9	76.3	0.86	0.79	32.0
All Vehi	icles	4378	0.7	0.824	43.8	LOS D	34.4	240.9	0.90	0.83	34.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mover	nent Performance - Pedestrians							
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P5	SouthEast Full Crossing	53	58.9	LOS E	0.2	0.2	0.94	0.94
P5S	SouthEast Slip/Bypass Lane Crossing	53	40.9	LOS E	0.2	0.2	0.78	0.78
P6	NorthEast Full Crossing	53	36.4	LOS D	0.1	0.1	0.73	0.73
P6S	NorthEast Slip/Bypass Lane Crossing	53	29.4	LOS C	0.1	0.1	0.66	0.66
P7	NorthWest Full Crossing	53	61.8	LOS F	0.2	0.2	0.96	0.96
P7S	NorthWest Slip/Bypass Lane Crossing	53	40.9	LOS E	0.2	0.2	0.78	0.78
P8	SouthWest Full Crossing	53	32.8	LOS D	0.1	0.1	0.70	0.70
P8S	SouthWest Slip/Bypass Lane Crossing	53	29.4	LOS C	0.1	0.1	0.66	0.66
All Ped	estrians	421	41.3	LOS E			0.78	0.78

Site: Windsor / Guntawong PM

New Site

Signals - Fixed Time Cycle Time = 150 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Perfo	rmance - Vehic	les								
Mov	OD	Demand	l Flows	Deg.	Average	Level of	95% Back o	f Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
OsuthE	a a fu NA <i>llin</i> al a au	veh/h	%	v/c	sec		veh	m		per veh	km/h
		r Road (South)					<i></i>				
21	L2	188	0.0	0.193	25.2	LOS B	7.1	49.6	0.56	0.72	42.1
22	T1	939	1.0	0.467	41.2	LOS C	17.6	123.1	0.84	0.72	35.8
23	R2	98	0.0	0.377	69.4	LOS E	6.6	46.1	0.95	0.78	27.9
Approa	ch	1225	0.8	0.467	41.0	LOS C	17.6	123.1	0.80	0.72	35.8
NorthEa	ast: Box Roa	ad (East)									
24	L2	86	0.0	0.118	36.1	LOS C	3.9	27.5	0.67	0.72	37.5
25	T1	64	0.0	0.154	51.3	LOS D	3.8	26.6	0.85	0.66	32.8
26	R2	64	0.0	0.236	67.0	LOS E	4.2	29.3	0.92	0.75	28.7
Approa	ch	215	0.0	0.236	49.9	LOS D	4.2	29.3	0.80	0.71	33.0
NorthW	est: Windso	r Road (North)									
27	L2	164	0.0	0.168	24.9	LOS B	6.1	42.6	0.55	0.71	42.3
28	T1	1663	1.0	0.878	57.5	LOS E	43.7	305.6	0.98	0.96	31.0
29	R2	219	0.0	0.842	81.1	LOS F	17.0	119.0	1.00	0.92	25.6
Approa	ch	2046	0.8	0.878	57.4	LOS E	43.7	305.6	0.95	0.94	30.9
SouthW	/est: Guntav	vong Road (West	t)								
30	L2	177	0.0	0.242	37.9	LOS C	8.5	59.7	0.71	0.75	36.8
31	T1	177	0.0	0.425	54.9	LOS D	11.2	78.3	0.91	0.75	31.8
32	R2	236	0.0	0.866	82.8	LOS F	18.7	130.7	1.00	0.94	25.5
Approa	ch	589	0.0	0.866	60.9	LOS E	18.7	130.7	0.89	0.83	30.1
All Vehi	cles	4076	0.6	0.878	52.6	LOS D	43.7	305.6	0.89	0.85	32.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mover	nent Performance - Pedestrians							
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P5	SouthEast Full Crossing	53	66.4	LOS F	0.2	0.2	0.94	0.94
P5S	SouthEast Slip/Bypass Lane Crossing	53	48.1	LOS E	0.2	0.2	0.80	0.80
P6	NorthEast Full Crossing	53	41.2	LOS E	0.2	0.2	0.74	0.74
P6S	NorthEast Slip/Bypass Lane Crossing	53	34.1	LOS D	0.2	0.2	0.67	0.67
P7	NorthWest Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P7S	NorthWest Slip/Bypass Lane Crossing	53	48.1	LOS E	0.2	0.2	0.80	0.80
P8	SouthWest Full Crossing	53	37.5	LOS D	0.2	0.2	0.71	0.71
P8S	SouthWest Slip/Bypass Lane Crossing	53	34.1	LOS D	0.2	0.2	0.67	0.67
All Ped	estrians	421	47.3	LOS E			0.79	0.79

Site: Windsor / Mt Carmel AM

New Site

Signals - Fixed Time Cycle Time = 100 seconds (Practical Cycle Time)

Moven	nent Perfo	rmance - Vehic	les								
Mov	OD	Demand	Flows	Deg.	Average	Level of	95% Back o	f Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0, 11, 5		veh/h	%	v/c	sec		veh	m		per veh	km/h
		r Road (South)									
21	L2	72	0.0	0.096	25.5	LOS B	2.2	15.1	0.65	0.70	42.0
22	T1	1213	0.0	0.777	40.1	LOS C	18.3	128.1	0.98	0.90	36.3
23	R2	143	0.0	0.385	53.1	LOS D	3.4	24.0	0.98	0.76	31.9
Approa	ch	1427	0.0	0.777	40.7	LOS C	18.3	128.1	0.96	0.87	36.0
NorthEa	ast: Mt Carm	el Road (East)									
24	L2	111	0.0	0.124	20.9	LOS B	2.9	20.6	0.58	0.70	44.3
25	T1	387	0.0	0.310	27.6	LOS B	7.1	49.9	0.80	0.66	41.5
26	R2	56	0.0	0.334	53.9	LOS D	2.7	18.8	0.98	0.75	32.0
Approa	ch	554	0.0	0.334	28.9	LOS C	7.1	49.9	0.77	0.68	40.8
NorthW	est: Windso	r Road (North)									
27	L2	121	0.0	0.163	26.2	LOS B	3.8	26.3	0.68	0.73	41.7
28	T1	812	0.0	0.520	34.9	LOS C	10.8	75.5	0.91	0.76	38.2
29	R2	279	0.0	0.751	57.5	LOS E	7.2	50.5	1.00	0.88	30.8
Approa	ch	1212	0.0	0.751	39.2	LOS C	10.8	75.5	0.91	0.78	36.5
SouthW	/est: Mt Carr	mel Road (West)									
30	L2	425	0.0	0.477	24.4	LOS B	13.9	97.3	0.72	0.79	42.5
31	T1	298	0.0	0.239	26.9	LOS B	5.3	37.4	0.78	0.63	41.9
32	R2	127	0.0	0.762	58.7	LOS E	6.7	46.6	1.00	0.88	30.8
Approa	ch	851	0.0	0.762	30.4	LOS C	13.9	97.3	0.78	0.75	40.0
All Vehi	cles	4043	0.0	0.777	36.5	LOS C	18.3	128.1	0.88	0.79	37.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mover	nent Performance - Pedestrians							
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P5	SouthEast Full Crossing	53	44.3	LOS E	0.1	0.1	0.94	0.94
P5S	SouthEast Slip/Bypass Lane Crossing	53	24.6	LOS C	0.1	0.1	0.70	0.70
P6	NorthEast Full Crossing	53	40.6	LOS E	0.1	0.1	0.90	0.90
P6S	NorthEast Slip/Bypass Lane Crossing	53	29.7	LOS C	0.1	0.1	0.77	0.77
P7	NorthWest Full Crossing	53	44.3	LOS E	0.1	0.1	0.94	0.94
P7S	NorthWest Slip/Bypass Lane Crossing	53	24.6	LOS C	0.1	0.1	0.70	0.70
P8	SouthWest Full Crossing	53	40.6	LOS E	0.1	0.1	0.90	0.90
P8S	SouthWest Slip/Bypass Lane Crossing	53	29.7	LOS C	0.1	0.1	0.77	0.77
All Ped	estrians	421	34.8	LOS D			0.83	0.83

Site: Windsor / Mt Carmel PM

New Site

Signals - Fixed Time Cycle Time = 100 seconds (Practical Cycle Time)

Moven	nent Perfo	rmance - Vehic	les								
Mov	OD	Demand	Flows	Deg.	Average	Level of	95% Back o	f Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
SouthE	aat: Windoo	veh/h	%	v/c	sec		veh	m		per veh	km/h
		r Road (South)		0.075	00.4	100.0	4.0	10.0	0.74	0.70	40.0
21	L2	47	0.0	0.075	29.4	LOS C	1.6	10.9	0.71	0.70	40.2
22	T1	760	0.0	0.580	38.7	LOS C	10.6	74.3	0.95	0.78	36.8
23	R2	142	0.0	0.239	46.0	LOS D	3.1	21.7	0.91	0.75	34.0
Approa	ch	949	0.0	0.580	39.3	LOS C	10.6	74.3	0.93	0.77	36.5
NorthEa	ast: Mt Carm	nel Road (East)									
24	L2	160	0.0	0.160	17.9	LOS B	3.9	27.1	0.53	0.70	46.0
25	T1	458	0.0	0.367	28.2	LOS B	8.6	60.3	0.82	0.68	41.3
26	R2	68	0.0	0.526	57.3	LOS E	3.5	24.2	1.00	0.76	31.1
Approa	ch	686	0.0	0.526	28.7	LOS C	8.6	60.3	0.77	0.70	40.9
NorthW	est: Windso	r Road (North)									
27	L2	229	0.0	0.363	32.4	LOS C	8.4	59.1	0.80	0.78	38.9
28	T1	855	0.0	0.652	39.4	LOS C	12.2	85.2	0.96	0.80	36.6
29	R2	443	0.0	0.746	52.2	LOS D	11.0	77.3	1.00	0.88	32.2
Approa	ch	1527	0.0	0.746	42.1	LOS C	12.2	85.2	0.95	0.82	35.5
SouthW	/est: Mt Cari	mel Road (West)									
30	L2	316	0.0	0.315	19.2	LOS B	8.4	59.1	0.59	0.74	45.3
31	T1	221	0.0	0.177	26.3	LOS B	3.9	27.1	0.76	0.61	42.2
32	R2	95	0.0	0.729	59.9	LOS E	5.0	34.9	1.00	0.85	30.5
Approa	ch	632	0.0	0.729	27.8	LOS B	8.4	59.1	0.71	0.71	41.2
All Vehi	cles	3795	0.0	0.746	36.6	LOS C	12.2	85.2	0.87	0.77	37.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mover	nent Performance - Pedestrians							
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P5	SouthEast Full Crossing	53	44.3	LOS E	0.1	0.1	0.94	0.94
P5S	SouthEast Slip/Bypass Lane Crossing	53	24.6	LOS C	0.1	0.1	0.70	0.70
P6	NorthEast Full Crossing	53	44.3	LOS E	0.1	0.1	0.94	0.94
P6S	NorthEast Slip/Bypass Lane Crossing	53	32.9	LOS D	0.1	0.1	0.81	0.81
P7	NorthWest Full Crossing	53	44.3	LOS E	0.1	0.1	0.94	0.94
P7S	NorthWest Slip/Bypass Lane Crossing	53	24.6	LOS C	0.1	0.1	0.70	0.70
P8	SouthWest Full Crossing	53	44.3	LOS E	0.1	0.1	0.94	0.94
P8S	SouthWest Slip/Bypass Lane Crossing	53	32.9	LOS D	0.1	0.1	0.81	0.81
All Ped	estrians	421	36.5	LOS D			0.85	0.85

Site: Garfield / Hambledon AM

New Site

Signals - Fixed Time Cycle Time = 95 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Perfor	rmance - Vehic	les								
Mov	OD	Demand		Deg.	Average	Level of	95% Back o	f Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0		veh/h	%	v/c	sec		veh	m		per veh	km/h
		Road (South)									
21	L2	425	0.0	0.531	27.0	LOS B	14.5	101.6	0.79	0.81	41.3
22	T1	572	0.0	0.480	29.1	LOS C	10.9	76.0	0.87	0.73	40.8
23	R2	149	0.0	0.510	46.3	LOS D	6.6	45.9	0.96	0.80	34.0
Approa	ch	1146	0.0	0.531	30.6	LOS C	14.5	101.6	0.85	0.77	39.9
East: G	arfield Road	East (East)									
24	L2	35	0.0	0.042	21.5	LOS B	0.9	6.3	0.59	0.67	44.0
25	T1	829	0.0	0.641	36.9	LOS C	11.9	83.0	0.96	0.81	37.5
26	R2	46	0.0	0.395	55.1	LOS D	2.2	15.5	1.00	0.74	31.4
Approa	ch	911	0.0	0.641	37.2	LOS C	11.9	83.0	0.95	0.80	37.3
North: H	Hambledon F	Road (North)									
27	L2	82	0.0	0.102	22.7	LOS B	2.2	15.7	0.62	0.70	43.4
28	T1	475	0.0	0.399	28.2	LOS B	8.7	61.2	0.84	0.70	41.2
29	R2	263	0.0	0.897	60.9	LOS E	14.4	100.9	1.00	1.03	30.0
Approa	ch	820	0.0	0.897	38.2	LOS C	14.4	100.9	0.87	0.81	36.9
West: G	Sarfield Road	d East (West)									
30	L2	48	0.0	0.314	37.8	LOS C	5.4	37.6	0.86	0.73	38.2
31	T1	362	0.0	0.314	33.4	LOS C	5.4	37.6	0.88	0.71	38.7
32	R2	72	0.0	0.305	54.6	LOS D	1.7	11.9	0.99	0.72	31.5
Approa	ch	482	0.0	0.314	37.0	LOS C	5.4	37.6	0.89	0.72	37.4
All Vehi	cles	3359	0.0	0.897	35.1	LOS C	14.5	101.6	0.89	0.78	38.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mover	nent Performance - Pedestrians							
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P5	South Full Crossing	53	41.8	LOS E	0.1	0.1	0.94	0.94
P5S	South Slip/Bypass Lane Crossing	53	30.5	LOS D	0.1	0.1	0.80	0.80
P6	East Full Crossing	53	41.8	LOS E	0.1	0.1	0.94	0.94
P6S	East Slip/Bypass Lane Crossing	53	24.4	LOS C	0.1	0.1	0.72	0.72
P7	North Full Crossing	53	39.9	LOS D	0.1	0.1	0.92	0.92
P7S	North Slip/Bypass Lane Crossing	53	30.5	LOS D	0.1	0.1	0.80	0.80
P8	West Full Crossing	53	41.8	LOS E	0.1	0.1	0.94	0.94
P8S	West Slip/Bypass Lane Crossing	53	24.4	LOS C	0.1	0.1	0.72	0.72
All Ped	estrians	421	34.4	LOS D			0.85	0.85

Site: Garfield / Hambledon PM

New Site

Signals - Fixed Time Cycle Time = 105 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Perfo	rmance - Vehic	les								
Mov	OD	Demand	Flows	Deg.	Average	Level of	95% Back o	f Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0, 11, 1	1	veh/h	%	v/c	sec		veh	m		per veh	km/h
		Road (South)									
21	L2	79	0.0	0.109	27.3	LOS B	2.5	17.8	0.67	0.71	41.2
22	T1	617	0.0	0.573	35.4	LOS C	13.7	95.8	0.91	0.78	38.1
23	R2	140	0.0	0.317	41.4	LOS C	6.0	41.7	0.87	0.78	35.6
Approad	ch	836	0.0	0.573	35.6	LOS C	13.7	95.8	0.88	0.77	37.9
East: G	arfield Road	East (East)									
24	L2	174	0.0	0.189	21.3	LOS B	4.9	34.2	0.59	0.72	44.1
25	T1	661	0.0	0.565	41.5	LOS C	10.4	72.5	0.95	0.79	35.8
26	R2	81	0.0	0.764	64.6	LOS E	4.6	31.9	1.00	0.86	29.0
Approa	ch	916	0.0	0.764	39.7	LOS C	10.4	72.5	0.89	0.78	36.4
North: H	lambledon F	Road (North)									
27	L2	118	0.0	0.163	27.8	LOS B	3.9	27.3	0.69	0.73	40.9
28	T1	681	0.0	0.632	36.1	LOS C	15.4	107.9	0.93	0.80	37.9
29	R2	376	0.0	0.850	54.6	LOS D	20.8	145.3	1.00	0.95	31.6
Approa	ch	1175	0.0	0.850	41.2	LOS C	20.8	145.3	0.93	0.84	35.9
West: G	arfield Road	d East (West)									
30	L2	39	0.0	0.274	42.7	LOS D	4.7	33.2	0.87	0.72	36.4
31	T1	285	0.0	0.274	38.3	LOS C	4.7	33.2	0.88	0.71	36.8
32	R2	61	0.0	0.288	60.2	LOS E	1.6	11.2	0.99	0.72	30.1
Approa	ch	385	0.0	0.288	42.2	LOS C	4.7	33.2	0.90	0.71	35.5
All Vehi	cles	3312	0.0	0.850	39.5	LOS C	20.8	145.3	0.90	0.79	36.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mover	nent Performance - Pedestrians							
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P5	South Full Crossing	53	46.8	LOS E	0.1	0.1	0.94	0.94
P5S	South Slip/Bypass Lane Crossing	53	35.3	LOS D	0.1	0.1	0.82	0.82
P6	East Full Crossing	53	46.8	LOS E	0.1	0.1	0.94	0.94
P6S	East Slip/Bypass Lane Crossing	53	29.0	LOS C	0.1	0.1	0.74	0.74
P7	North Full Crossing	53	44.9	LOS E	0.1	0.1	0.93	0.93
P7S	North Slip/Bypass Lane Crossing	53	35.3	LOS D	0.1	0.1	0.82	0.82
P8	West Full Crossing	53	46.8	LOS E	0.1	0.1	0.94	0.94
P8S	West Slip/Bypass Lane Crossing	53	29.0	LOS C	0.1	0.1	0.74	0.74
All Ped	estrians	421	39.2	LOS D			0.86	0.86

😵 Site: Guntawong / Cudgegong AM

New Site Roundabout

Move	ment Perfo	ormance - Ve	hicles								
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/ł
South	Cudgegong	Road (South)									
1	L2	34	0.0	0.197	7.9	LOS A	1.1	7.8	0.64	0.77	49.
3	R2	129	0.0	0.197	11.5	LOS A	1.1	7.8	0.64	0.77	50.
Appro	ach	163	0.0	0.197	10.8	LOS A	1.1	7.8	0.64	0.77	50.
East: Guntawong Ro		Road (East)									
4	L2	107	0.0	0.441	4.7	LOS A	3.6	24.9	0.21	0.46	53.
5	T1	556	0.0	0.441	5.0	LOS A	3.6	24.9	0.21	0.46	54.
Appro	ach	663	0.0	0.441	4.9	LOS A	3.6	24.9	0.21	0.46	54.
West:	Guntawong	Road (West)									
11	T1	326	0.0	0.298	5.5	LOS A	2.0	14.1	0.39	0.53	53.
12	R2	36	0.0	0.298	8.8	LOS A	2.0	14.1	0.39	0.53	53.
Appro	ach	362	0.0	0.298	5.8	LOS A	2.0	14.1	0.39	0.53	53.
All Vel	nicles	1188	0.0	0.441	6.0	LOS A	3.6	24.9	0.32	0.52	53.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Processed: Thursday, 20 November 2014 3:55:17 PM SIDRA INTERSECTION 6.0.24.4877 Project: J:\234000\234974-00 Riverstone E Vineyard\Work\01 Arup Project Data\3. Analysis\RiverstoneE 281014 \201114\Guntawong_Cudgegong_201114.sip6 8000047, 6019197, ARUP PTY LTD, PLUS / Floating

SIDRA INTERSECTION 6

𝒞 Site: Guntawong / Cudgegong PM

New Site Roundabout

Move	ment Perfo	ormance - Ve	hicles								l
Mov ID	OD Mov	Demand Total	ΗV	Deg. Satn	Average Delay	Level of Service	95% Back of Vehicles	Distance	Prop. Queued	Effective Stop Rate	Average Speed
South:	Cudgegong	veh/h Road (South)	%	v/c	Sec	_	veh	m	_	per veh	km/h
1	L2	45	0.0	0.131	6.9	LOS A	0.7	4.9	0.55	0.70	51.0
3	R2	75	0.0	0.131	10.4	LOS A	0.7	4.9	0.55	0.70	51.6
Approa	ach	120	0.0	0.131	9.1	LOS A	0.7	4.9	0.55	0.70	51.4
East: (Guntawong F	Road (East)									
4	L2	81	0.0	0.356	4.8	LOS A	2.5	17.3	0.24	0.47	53.4
5	T1	425	0.0	0.356	5.1	LOS A	2.5	17.3	0.24	0.47	54.4
Approa	ach	506	0.0	0.356	5.0	LOS A	2.5	17.3	0.24	0.47	54.3
West:	Guntawong	Road (West)									
11	T1	487	0.0	0.397	5.2	LOS A	3.1	21.4	0.32	0.49	53.8
12	R2	54	0.0	0.397	8.5	LOS A	3.1	21.4	0.32	0.49	53.5
Approa	ach	541	0.0	0.397	5.5	LOS A	3.1	21.4	0.32	0.49	53.8
All Vel	nicles	1167	0.0	0.397	5.7	LOS A	3.1	21.4	0.30	0.50	53.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Processed: Thursday, 20 November 2014 3:55:55 PM SIDRA INTERSECTION 6.0.24.4877 Project: J:\234000\234974-00 Riverstone E Vineyard\Work\01 Arup Project Data\3. Analysis\RiverstoneE 281014 \201114\Guntawong_Cudgegong_201114.sip6 8000047, 6019197, ARUP PTY LTD, PLUS / Floating

SIDRA INTERSECTION 6

Site: Guntawong / Tallawong AM

Existing Site

Signals - Fixed Time Cycle Time = 47 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Perfo	rmance - Vehio	cles								
Mov	OD	Demand		Deg.	Average	Level of	95% Back o	f Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
	Tallawong R	· /									
30	L2	127	0.0	0.320	18.3	LOS B	3.6	25.2	0.78	0.72	46.4
31	T1	291	0.0	0.320	12.7	LOS A	3.7	26.0	0.78	0.66	49.0
32	R2	59	0.0	0.149	19.2	LOS B	1.0	7.3	0.77	0.73	44.7
Approa	ch	477	0.0	0.320	15.0	LOS B	3.7	26.0	0.78	0.69	47.7
East: G	untawong R	load (East)									
21	L2	97	0.0	0.576	17.6	LOS B	8.2	57.6	0.83	0.74	48.2
22	T1	445	0.0	0.576	15.6	LOS B	8.2	57.6	0.86	0.80	46.9
23	R2	96	0.0	0.855	34.6	LOS C	5.3	37.0	1.00	1.03	38.9
Approa	ch	638	0.0	0.855	18.8	LOS B	8.2	57.6	0.88	0.82	45.6
North: 7	allawong Ro	oad (North)									
24	L2	47	0.0	0.231	17.8	LOS B	2.5	17.8	0.75	0.65	47.7
25	T1	257	0.0	0.231	12.3	LOS A	2.6	18.1	0.75	0.63	49.5
26	R2	61	0.0	0.175	20.3	LOS B	1.1	7.9	0.79	0.73	44.1
Approa	ch	365	0.0	0.231	14.3	LOS A	2.6	18.1	0.76	0.65	48.3
West: G	Guntawong F	Road (West)									
27	L2	111	0.0	0.661	18.5	LOS B	10.0	70.5	0.87	0.79	47.7
28	T1	460	1.0	0.661	17.5	LOS B	10.0	70.5	0.88	0.85	45.8
29	R2	129	0.0	0.981	54.4	LOS D	7.0	49.2	1.00	1.29	31.8
Approa	ch	700	0.7	0.981	24.5	LOS B	10.0	70.5	0.90	0.92	42.6
All Vehi	cles	2180	0.2	0.981	19.0	LOS B	10.0	70.5	0.85	0.80	45.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestrians							
Mov	Description	Demand	Average	Level of	Average Back c		Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P8	South Full Crossing	53	17.9	LOS B	0.1	0.1	0.87	0.87
P5	East Full Crossing	53	17.9	LOS B	0.1	0.1	0.87	0.87
P6	North Full Crossing	53	17.9	LOS B	0.1	0.1	0.87	0.87
P7	West Full Crossing	53	17.9	LOS B	0.1	0.1	0.87	0.87
All Pec	lestrians	211	17.9	LOS B			0.87	0.87



Site: Guntawong / Tallawong PM

Existing Site

Signals - Fixed Time Cycle Time = 55 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Perfo	rmance - Vehic	cles								
Mov	OD	Demano	Flows	Deg.	Average	Level of	95% Back o	f Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Ocutha	Telleure e D	veh/h	%	v/c	sec		veh	m		per veh	km/h
	Tallawong R	· /			.						
30	L2	33	0.0	0.146	21.4	LOS B	1.7	11.6	0.77	0.66	45.3
31	T1	132	0.0	0.146	15.9	LOS B	1.7	11.8	0.77	0.62	47.1
32	R2	22	0.0	0.076	24.8	LOS B	0.5	3.5	0.82	0.70	41.8
Approa	ch	186	0.0	0.146	17.9	LOS B	1.7	11.8	0.78	0.64	46.1
East: G	untawong R	oad (East)									
21	L2	111	0.0	0.483	15.7	LOS B	8.3	58.1	0.71	0.66	49.3
22	T1	635	0.0	0.718	13.0	LOS A	11.3	79.0	0.79	0.75	48.4
23	R2	174	0.0	0.718	21.9	LOS B	11.3	79.0	0.89	0.85	45.3
Approa	ch	919	0.0	0.718	15.0	LOS B	11.3	79.0	0.80	0.76	47.9
North:	Fallawong Ro	oad (North)									
24	L2	72	0.0	0.351	22.6	LOS B	4.3	29.9	0.83	0.72	44.7
25	T1	323	0.0	0.351	17.1	LOS B	4.3	30.4	0.83	0.70	46.4
26	R2	81	0.0	0.209	22.9	LOS B	1.8	12.3	0.81	0.75	42.7
Approa	ch	476	0.0	0.351	18.9	LOS B	4.3	30.4	0.83	0.71	45.5
West: 0	Suntawong F	Road (West)									
27	L2	51	0.0	0.128	13.8	LOS A	1.8	12.3	0.57	0.57	50.0
28	T1	258	0.0	0.335	10.9	LOS A	4.2	29.4	0.68	0.61	50.0
29	R2	38	0.0	0.335	17.5	LOS B	4.2	29.4	0.72	0.63	48.7
Approa	ch	346	0.0	0.335	12.1	LOS A	4.2	29.4	0.67	0.61	49.8
All Vehi	cles	1927	0.0	0.718	15.7	LOS B	11.3	79.0	0.78	0.71	47.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	nent Performance - Pedestrians							
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Pedestrian			Effective Stop Rate
P8	South Full Crossing	ped/h 53	sec 15.3	LOS B	ped 0.1	0.1	0.75	per ped 0.75
P5	East Full Crossing	53	21.9	LOS C	0.1	0.1	0.89	0.89
P6	North Full Crossing	53	15.3	LOS B	0.1	0.1	0.75	0.75
P7	West Full Crossing	53	21.9	LOS C	0.1	0.1	0.89	0.89
All Ped	lestrians	211	18.6	LOS B			0.82	0.82



Site: Guntawong / Hambledon AM

Existing Site

Signals - Fixed Time Cycle Time = 110 seconds (Practical Cycle Time)

Moven	nent Perfo	rmance - Vehic	les								
Mov	OD	Demand		Deg.	Average	Level of	95% Back o	f Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Ocutha	Levelleder	veh/h	%	v/c	sec		veh	m		per veh	km/h
		Road (South)									
30	L2	120	0.0	0.282	42.9	LOS D	5.3	37.2	0.86	0.77	35.0
31	T1	699	0.0	0.604	35.5	LOS C	16.3	113.9	0.91	0.78	37.9
32	R2	180	0.0	0.807	59.2	LOS E	10.6	73.9	1.00	0.94	30.0
Approa	ch	999	0.0	0.807	40.6	LOS C	16.3	113.9	0.92	0.81	35.9
East: G	untawong R	oad (East)									
21	L2	102	0.0	0.506	36.9	LOS C	14.0	98.1	0.85	0.76	38.3
22	T1	351	0.0	0.506	39.0	LOS C	14.0	98.1	0.91	0.80	36.0
23	R2	56	0.0	0.732	58.2	LOS E	10.0	69.8	1.00	0.88	31.5
Approa	ch	508	0.0	0.732	40.7	LOS C	14.0	98.1	0.91	0.80	35.9
North: I	Hambledon F	Road (North)									
24	L2	135	0.0	0.503	53.6	LOS D	6.9	48.0	0.97	0.79	31.8
25	T1	475	0.0	0.406	33.0	LOS C	10.2	71.1	0.85	0.71	38.9
26	R2	80	0.0	0.536	55.4	LOS D	4.2	29.6	0.97	0.79	31.0
Approa	ch	689	0.0	0.536	39.7	LOS C	10.2	71.1	0.88	0.74	36.2
West: 0	Guntawong F	Road (West)									
27	L2	84	0.0	0.547	29.4	LOS C	18.4	128.5	0.79	0.72	41.9
28	T1	659	0.0	0.791	32.8	LOS C	20.0	140.0	0.87	0.80	38.6
29	R2	101	0.0	0.791	51.0	LOS D	20.0	140.0	1.00	0.93	33.7
Approa	ch	844	0.0	0.791	34.6	LOS C	20.0	140.0	0.88	0.81	38.2
All Vehi	cles	3041	0.0	0.807	38.8	LOS C	20.0	140.0	0.90	0.79	36.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mover	nent Performance - Pedestrians							
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P8	South Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P8S	South Slip/Bypass Lane Crossing	53	37.7	LOS D	0.1	0.1	0.83	0.83
P5	East Full Crossing	53	34.5	LOS D	0.1	0.1	0.79	0.79
P6	North Full Crossing	53	49.3	LOS E	0.2	0.2	0.95	0.95
P6S	North Slip/Bypass Lane Crossing	53	37.7	LOS D	0.1	0.1	0.83	0.83
P7	West Full Crossing	53	34.5	LOS D	0.1	0.1	0.79	0.79
All Ped	estrians	316	40.5	LOS E			0.86	0.86

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA INTERSECTION 6

Site: Guntawong / Hambledon PM

Existing Site

Signals - Fixed Time Cycle Time = 100 seconds (Practical Cycle Time)

Movement Performance - Vehicles											
Mov	OD	Demand	Flows	Deg.	Average	Level of	95% Back o	f Queue	Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0 11		veh/h	%	v/c	sec		veh	m		per veh	km/h
	Clarke Stree	()									
30	L2	51	0.0	0.141	40.6	LOS C	2.0	14.2	0.85	0.73	35.8
31	T1	385	0.0	0.341	30.2	LOS C	7.4	52.0	0.83	0.69	40.1
32	R2	131	0.0	0.715	52.9	LOS D	6.7	46.6	1.00	0.88	31.6
Approa	ch	566	0.0	0.715	36.4	LOS C	7.4	52.0	0.87	0.74	37.4
East: G	untawong R	oad (East)									
21	L2	73	0.0	0.488	26.3	LOS B	14.5	101.5	0.75	0.69	43.4
22	T1	558	0.0	0.488	27.7	LOS B	14.5	101.5	0.84	0.75	40.7
23	R2	94	0.0	0.706	45.3	LOS D	13.9	97.6	0.98	0.86	35.5
Approa	ch	724	0.0	0.706	29.9	LOS C	14.5	101.5	0.85	0.76	40.2
North: (Clarke Street	t (North)									
24	L2	245	0.0	0.729	50.0	LOS D	12.0	83.7	1.00	0.87	32.8
25	T1	580	0.0	0.513	32.1	LOS C	11.9	83.2	0.89	0.75	39.3
26	R2	113	0.0	0.447	43.3	LOS D	4.9	34.6	0.91	0.79	34.5
Approa	ch	938	0.0	0.729	38.1	LOS C	12.0	83.7	0.92	0.79	36.8
West: G	Buntawong R	Road (West)									
27	L2	46	0.0	0.453	32.6	LOS C	11.6	81.0	0.82	0.72	40.5
28	T1	372	0.0	0.453	33.0	LOS C	11.6	81.0	0.88	0.75	38.6
29	R2	46	0.0	0.656	52.6	LOS D	7.7	53.8	1.00	0.83	33.1
Approa	ch	464	0.0	0.656	34.9	LOS C	11.6	81.0	0.88	0.76	38.1
All Vehi	cles	2693	0.0	0.729	35.0	LOS C	14.5	101.5	0.88	0.76	38.0

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mover	nent Performance - Pedestrians							
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back o Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P8	South Full Crossing	53	44.3	LOS E	0.1	0.1	0.94	0.94
P8S	South Slip/Bypass Lane Crossing	53	32.9	LOS D	0.1	0.1	0.81	0.81
P5	East Full Crossing	53	32.9	LOS D	0.1	0.1	0.81	0.81
P6	North Full Crossing	53	44.3	LOS E	0.1	0.1	0.94	0.94
P6S	North Slip/Bypass Lane Crossing	53	32.9	LOS D	0.1	0.1	0.81	0.81
P7	West Full Crossing	53	32.9	LOS D	0.1	0.1	0.81	0.81
All Ped	estrians	316	36.7	LOS D			0.86	0.86

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA INTERSECTION 6