

Pa ramatta North Urban Renewal
Proposed Rezoning
Traffic and Transport Review
transportation planning, design and delivery

# Pa rramatta North Urban Renewal 

Proposed Rezoning

## Traffic and Transport Review

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## Executive Summary

Pa ramatta North Urban Renewal area (PNUR) is located to the west and north-west of the Pa rramatta CBD, Sydney's second CBD. PNUR includes many loc ational and site specific attributes, including frontage to the Parramatta River and a rich history of Aboriginal, early colonial, nineteenth and twentieth century uses.

PNUR in its end state proposes to provide about 5,600 residential dwellings, $35,000 \mathrm{~m}^{2}$ of adaptive reuse of historic buildings and $4,000 \mathrm{~m}^{2}$ of retail use in the Cumberland Precinct. It is also proposed to include $46,000 \mathrm{~m}^{2}$ of mixed use developments in the Sports and Leisure Precinct (which would be predominantly commercial use). The staging of the works will be over a 15 to 20 year period.

GTA Consultants has been engaged by UrbanG rowth NSW to assess the traffic and transport impacts/issues relating to the proposed amendment to the planning framework applying to the study a rea. The investigations relate only to the Cumberland and Sports and Leisure Precincts within the PNUR. The Pa rramatta Gaol and SES land do not form part of this rezoning proposal.

An assessment of carparking requirement using the current Parramatta City Council's development control plan indicates that the proposal would need to provide 8,820 to 9,770 car parking spaces.

However, in order to minimise the cartravel, a number of measures will be incorporated into the proposal. The potential measures are:

- limited parking ratios
- busimprovements
- cycle parking /facilities
- carsharing/carclub cars.

On the basis of all such measures being fully incorporated into the development, it is a nticipated that the subject site would generate signific antly less tra ffic than other residential sites in the vicinity, which will have the positive effect of reducing the traffic impact of the proposal.

The comparison of post development flows and the theoretic al ca pacity indic atesthat Church Street, south of Pennant Hills Road which is the section along the Church Street with only one tra velling lane in each direction (i.e. excluding the bus lane) would exceed its theoretical capacity. O'Connell Street, south of Bamey Street section and south of Victoria Road section and Bamey Street, east of O'Connell Street would also reach the theoretic al capacities under the current configurations.

Hence, the following intersection upgrades would be required to accommodate the additional traffic generated by the proposed development and the future background growth on key extemal roads:

- Church Street/Board Street - Upgrade to a partial signal (west side of Church Street only).
- Church Street/Ba mey Street - Additional right tum bay (i.e. dual right tum lanes) from Church Street southbound.
- For the intersections on Church Street between Factory Street and Grose Street, an additional through lane would be required for southbound traffic in the AM peak. For the PM peak, an additional northbound through lane would be required for the intersections on Church Street between east of Bamey Street and Grose Street.
- O'Connell Street intersec tions at Bamey Street \& Factory Street - Upgrade to a traffic signal.
- O'Connell Street intersections at Dunlop Street \& Fennell Street - Upgrade to a one-la ne roundabout.
- O'Connell Street/Victoria Road signalised intersection - Revise lane configuration.

In addition to the above, the Windsor Road bridge overthe Cumberland Highway is proposed to be widened aspart of the proposed Westem Sydney Regional Ring Road and this improvement will be necessary to address existing/ future traffic problems.

The provision of a new cycleway along the waterfront which will run from north of the site to south of the Sports Precinct would enhance the pedestrian and cycleway network signific antly.

Provision of a good quality shuttle bus service between the subject site and the Pa ramatta interchange is also proposed. In addition, the potential future introduction of light rail into the precinct would have the ability to signific antly reduce the travel by car mode.

In summary, the traffic impacts of the proposed development could be mitigated by the list of measures desc ribed in this report.

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## 1. Introduction

### 1.1 Background

Pa ramatta North Urban Renewal area (PNUR) is located to the west and north-west of the Pa ramatta CBD, Sydney's second CBD. Parramatta is loc ated in the geographical heart of Sydney and plays a significant role in the Greater Metropolitan area as the most important centre in Westem Sydney. The PNUR is located to the east of the Westmead Health campus, separated by the Parramatta River. The PNUR is also within close proximity to the Rydalmere Education Precinct a nd transport links.

GTA Consultants has been engaged by UrbanG rowth NSW to assess the traffic and transport impacts/ issues relating to the proposed amendment to the planning framework applying to the study a rea. The investigations relate only to the Cumberland and Sports and Leisure Precincts within the PNUR. The assessment has been undertaken to inform a State Signific ant Site study (the Study) which is investigating potential a mend ments of the sta tutory planning controls applying to the Cumberland and Sports and Leisure Precincts of the PNUR.

PNUR includes many locational and site specific attributes, including frontage to the Pa ramatta River and a rich history of Aboriginal, early colonial, nineteenth and twentieth century uses. The potential exists to deliver housing and employment opportunities in a precinct that will embrace and interpret these herita ge attributes to make them a focus of the urban environment that will emerge through future development and facilitate their retention and re-use.

The a mendment to the statutory planning provisions is a nticipated to be undertaken via a State Environmental Pla nning Policy (SEPP) to a mend the provisions of Pa ramatta City Centre LEP 2007 and Parramatta LEP 2011. Site specific Development Control Plan (DCP) provisionsare also proposed to be prepared to guide future development. Amendment of the planning framework will facilitate the lodgement of future Development Applications with Parramatta City Council to be assessed and determined under the provisions of Part 4 of the Environmental Planning and Assessment Act 1979.

### 1.2 Purpose of this Report

This report sets out an assessment of the antic ipated tra nsport implic ations of the proposed development, including consideration of the following:
i existing traffic and parking conditions surrounding the site
ii suitability of the proposed parking in terms of supply (qua ntum)
iii the traffic generating characteristics of the proposed development
iv the transport impact of the development proposal on the surrounding road network
v potential mitigation measuresto address transport impact of the development proposal.

Introduction

### 1.3 References

In preparing this report, reference has been made to the following:

- an inspection of the site and its surrounds
- Parramatta City Council's Development Control Plan 2011 Part 3 (DC)
- traffic and carparking surveys underta ken by Austraffic as referenced in the context of this report
- plansfor the proposed development prepared by AJ +C, Drawing Number 13031, Revision 13d, dated 14/10/2014
- other documents and data as referenced in this report
- meetings with TfNSW and Pa ramatta Council traffic engineers.


## 2. Existing Context

### 2.1 NSW Long Term Masterplan

The NSW Long Term Transport Master Plan 2012 (LTTMP) presents a 20 year vision for tra nsport planning through to 2031. The LTMP provides integrated advice with regards to transport polic y; identifying solutions to develop and manage the NSW's transport system with short to long term strategies. Forming part of the LTTMP is Sydney's Rail Future, a long-tem plan to inc rease the capacity of Sydney's rail network and update existing infrastructure.

Pa ramatta North has been identified as part of the ШTMP support of developments in the Greater Sydney region. More specific ally, the following strategieshave been outlined for Pennant Hills Road between North Parramatta and Wahroonga:

- Develop Strategic Bus Comidors for rapid service
- Investigate congestion management measures
- Improve road connectionsto Parramatta Road.


### 2.2 Parramatta Local Environmental Plan 2011

The Parramatta Local Environmental Plan provides guidelines, objectives and control for development in Parramatta C ity. Parramatta North is characterised by a reas of low density resid ential, mixed use, general industrial and enterprise (along Church Street comidor).

### 2.3 Parramatta Development C ontrol Plan 2011

The Development Control Plan outlines development principles including mea sures for sustainable transport, parking and vehic ular access, and access and connectivity. The following summarise development controls relating the North Pa ramatta area:

- North Pa rramatta is an early subdivision in Parramatta and identified as heritage conservation area.
- The Collet Park Precinct in North Parramatta has been identified as a special precinct, with DCP objectives to provide high and medium density developments and to improve pedestrian links throughout the precinct.
- The a rea surrounding All Saints Cemetery and the a rea bounded by Bric Kfield, Belmore, Buller and Albert Streets ha ve been identified as special character a reas, with specific design controls implemented to mainta in their character.


### 2.4 Proposed Westem Sydney Light Rail Network

Pa rramatta City Council advised that they had recently completed a feasibility study into the development of a light rail network for Westem Sydney. The proposed light rail network would link key centres within the westem Sydney region and would integrate with the existing public transport network. The light rail network is proposing to link Parramatta CBD to other westem Sydney centres such as Macqua rie Park and Strathfield in the east, to Rouse Hill in the north, to Bankstown and Liverpool in the south a nd to Wetherill Park and Blacktown in the west.

Council's proposed Westem Sydney Light Rail Network inc ludes two stops in the vic inity of the Cumberland Precinct - one within the site and another one on Church Street near Albert Street.

Existing Context

Figure 2.1 shows the proposed routes in the vicinity of the subject site.
Figure 2.1: Parramatta City Council's Proposed Light Rail Routes


Recently, the NSW Govemment announced that the Westem Sydney Light Rail network would be allocated a further $\$ 400$ million for a feasibility study to identify the highest prionity comidors from Pa ramatta.

The project is to be completed in two stages. Stage one is comprised of the Macquarie Park and C astle Hill lines, which would cost a combined $\$ 1.5$ billion for 30 kilometres of light rail and 21 light rail vehicles.

### 2.5 Proposed Westem Sydney Regional Ring Road

Pa ramatta City Council is proposing a regional ring road to address traffic congestion resulting from the entangling of cross regional carand freight flows. The proposed regional ring road would also improve efficiency in the road network so that the population of Westem Sydney can access employment and training opportunities close to home. A series of intersection upgrades are proposed along the M4 Motorway, J ames Ruse Drive and Cumberland Highway to create a free flowing arterial road network and allow traffic to circumna vigate Pa ramatta quickly and effic iently. Parramatta Council is also developing a City Ring Road to complement the Regional Ring Road.

Figure 2.2 shows the proposed Regional and City Ring Roads.

Figure 2.2: Proposed Westem Sydney Regional Ring Road


In the vic inity of the subject site, the Windsor Road bridge (Project Number 4 on the plan above) over the Cumberland Highway is proposed to be widened at a cost of $\$ 20$ million.

The Windsor Road intersection with the Cumberland Highway wasidentified in the discussion with Council as being critic al.

### 2.6 WestC onnex

WestConnex is a three stage upgrade of the M4 Westem Motorway whic encompasses 33 km of road upgrades in the form of road widening and tunnelling. The project will use a combination of above and below ground motorways in orderto save up to 40 minutes of travel time between Pa rramatta and Sydney Aiport. Stage 1 (Parramatta to Haberfield) of the project will be completed in two sections:
i Widening of the M4 Westem Motorway in both directions for 7.5 kilometres between Church Street, Pa rramatta and Homebush Bay Drive,
ii Widening M4 (east) and new 5 kilometre tunnel (under the Pa ramatta Road comidor) connecting Homebush Bay Drive with Parramatta Road and City West Link, Haberfield.

Along with the proposed works for widening and tunnelling, the M4 new motorway access points are to be introduced, including:

- eastbound accessto Westmead and Parramatta from the M4 near Coleman Street
- westbound access from Parramatta at Church Street to the M4.

The staging of works for WestC onnex is shown in Figure 2.3.

Figure 2.3: WestConnex - Stage 1: Panamatta to Haberfield


Image Source: WestC onnex Fact Sheet - Stage 1: Parramatta to Haberfield

### 2.7 Sydney's Bus Future: Ra pid Bus Routes

Sydney's Bus Future, December 2013 document outlines the NSW G ovemment's long term plan for the bus network to meet customer needs.

The proposed upgrade forthe Sydney bus network will include the addition of new rapid bus routes while maintaining and improving elements of the existing bus network, such ascross-city services on Metro bus routes. The additional rapid bus services are intended to operate every 10 minutes (or more often) during the week, between 6am and 7 pm , and every 15 minutes in weekends.

Rapid bus routes will offerfaster and more reliable bustravel for commuters between major city centres asextra senvices are planned to be implemented and busstops to be further dispersed along routes, generally spaced 800 meters to one kilometre a part.

Existing suburban and local service routes will rema in to provide commuter accessto local, neighbourhood destinations. An additional 20 suburban routes are to be introduced.

Proposed network upgrades would fill the gaps in the heavy rail network, strengthening links from the Parramatta region to areas including Norwest, Castle Hill, Macqua rie Park, Ryde, Bankstown, and Liverpool.

The proposed rapid bus routes include:

- Castle Hill to Liverpool via Parramatta
- Parramatta to the CBD via Ryde
- Rouse Hill to Hurstville via Parramatta and Bankstown
- Mona Vale to the CBD
- Maroubra Junction to the CBD
- North Bondi to the CBD
- Castle Hill to the CBD.

An example of the existing bus route and proposed rapid route from Pa ramatta to Sydney CBD via Ryde is shown in Figure 2.4. The proposed rapid bus routes connecting Parramatta to westem suburbs is shown in Figure 2.5.

Figure 2.4: Curent and Future Rapid Bus Route: Panamatta to CBD via Ryde


Current bus route
Rapid bus route
Bus stop
Future Suburban services and Local services (not shown) will connect with Rapid services

Image Source: Sydney's Bus Future 2013

Figure 2.5: Panamatta Rapid and Suburban Bus Routes


Image Source: Sydney's Bus Future 2013

## Existing Conditions

## 3. Existing Conditions

### 3.1 Site Description

Pa rramatta North Urban Renewal area (PNUR) is located to the west and north-west of the Pa ramatta CBD, Sydney's second CBD. Pa rramatta is loc ated in the geographical heart of Sydney and plays a significant role in the Greater Metropolitan area as the most important centre in Westem Sydney. The PNUR is located to the east of the Westmead Health campus, separated by the Parramatta River. The PNUR is also within close proximity to the Rydalmere Education Precinct a nd transport links.

Figure 3.1 presents the location plan of the site.
Figure 3.1: Location Plan


The Study relates only to the Cumberland a nd Sports and Leisure precincts within the PNUR.
The Sports and Leisure Precinct (SLP) is located centrally within the PNUR. The SLP is delineated to the west and south by the meander of the Parramatta River, O'Connell Street to the east and Grose Street to the north.

The Cumberland Precinct (CP) is the northem most part of the PNUR and is broadly delineated by the meander of the Parramatta River to the west and north, O'Connell Street to the east and Grose Street to the south.

Combined, the two precincts comprise the areas of the PNUR to the east of the Parramatta River and west of O'C onnell Street. The lands to the west of the Parramatta River contain Parramatta Park, including Old Govemment House and Domain.

### 3.2 Existing Land Uses

Land uses and facilities currently located within the SLP include Pa ramatta Sta dium and associated facilities, Parramatta public pool, Parramatta Leagues Club, open space parkland and venue carparking. These built facilities and associated structures occupy predominantly the north eastem two thirds of the precinct. The balance of the precinct, nestled inside the meander of the Pa rramatta River, is predominantly landscaped open space with some incursion of at grade carparking.

Existing land uses within the Cumberland Precinct include the Cumberland Hospital, the NSW Linen Service, allied health related uses and Non Govemment Organisations (NGOs) and the former Parramatta Gaol. The precinct contains buildings of State and local heritage significance as well as potential Aboriginal archaeological sites. Buildings are dispersed through the precinct serviced by an iregular access network and broadly sumounding a central oval. These clusters of buildings are interspersed with vegetation and are framed by an almost continuous band of vegetation framing the eastem bank of the Pa ramatta River.

### 3.3 Surrounding Areas

The PNUR study area is located to the immediate west and north-west of the Parramatta CBD. The north-eastem area of the CBD isemerging as a mixed use residential precinct with residential towerforms.

To the east of the study area, uses range from educ ational uses, residential accommodation in forms ranging from single dwellings to three storey residential flat buildings, interspersed with nonresidentia I uses of former dwellings. Further east a spine of retail and commercial uses are located along Church Street and Victoria Road.

To the north east of the site, generally along O'Connell Street building forms are typic ally three storey residential flat buildings and commercial and retail land uses in the areas to the east of the former Parramatta Gaol.

To the north of the site on the opposite bank of the Parramatta River is the Northmead industrial area including large format industrial buildings.

To the north-west of the site is a small pocket of single storey cottages bound by further industrial development to the west and three storey residential flat buildings fronting Briens Road, Northmead

To the west of the site beyond Parramatta Park is the Westmead medic al precinct which is adjoined by a residential area bound generally by Hawkesbury Road, Ha insworth Street, Park Avenue and Railway Parade. Development in this area is predominantly three storey residential flat build ing forms interspersed with ta ller higher density residential flat buildings. This residential pocket of land is separated from the lands the subject of this Study by Pa ramatta Park.

## Existing Conditions

### 3.4 Road Network

The road network in the vicinity of the site includes Church Street, Vic toria Road, O'Connell Street and a number of local streets such as Fleet Street, New Street, Dunlop Street, Fa ctory Street, Albert Street, Fennel Street and Marsden Street. Figure 3.2 shows the road network in vic inity of the site.

Figure 3.2: Road Network


Church Street is a north-south arterial road with one general traffic lane plus one buslane in each direction. Kerbside parking is not permitted on either side of the road. North of Bamey Street, Church Street is widened to provide two traffic lanes and one bus lane in each direction. It has a sign posted speed limit of $60 \mathrm{~km} / \mathrm{hr}$. Generally intersections along Church Street are controlled by traffic lights. Where the intersections are not controlled by traffic lights, traffic movements are generally restricted to left-in and left-out from/to the side streets.

Victoria Road is an east-west arterial road with generally two or three travelling lanes in each direction. Kerbside parking is not permitted on either side of the road. It has a sign posted speed limit of $60 \mathrm{~km} / \mathrm{hr}$. Generally intersections a long Victoria Road are controlled by traffic lights. Where the intersections are not controlled by traffic lights, traffic movements are generally restric ted to left-in and left-out from/to the side streets.

O'Connell Street is a north-south sub-arterial/collector road with one traffic lane in each direction. Kerbside parking is permitted intermittently along O'Connell Street. It also has a posted speed limit of $60 \mathrm{~km} / \mathrm{hr}$. Intersections a long $\mathrm{O}^{\prime}$ Connell Street are generally controlled by priority signs. At Factory Street, a median is provided along $\mathrm{O}^{\prime}$ Connell Street to restrict traffic movements to left-in a nd left-out only.

Fleet Street, New Street and Marsden Street are local streets. These loc al streets provide vehicular access to propertiesfronting onto them. They run in the north-south direction. Time restricted kerbside parking is permitted on either side of the road. Street trees are planted on both sides of Fleet Street making Fleet Street na rrower than New Street. These streets have a sign posted speed limit of $50 \mathrm{~km} / \mathrm{hr}$.

Similarly, Dunlop Street, Factory Street, Albert Street and Fennell Street are all local streets. Due to a level change, these locations provide vehicular access to residential propertiesbutting them. It is noted that Albert Street forms a cul-de-sac at its west end; therefore it does not connect to Fleet Street to permit access into the subject site from Albert Street.

Greenup Drive and Eastem Circ uit togetherform a two-way loop road that connects to Fleet Street on the eastem side and to Bridge Road on the westem side of the site. The intemal loop road provides access to the various parking areas within the precinct. RiverRoad joins Greenup Drive at two locations to provide access to the southem part of the subject site.

Some of these intemal roads have only sufficient width to allow two vehic les to pass a nother (but it might not be possible for a car to pass a truck around bends). The intemal roads have only green verge except for a short section on Eastem Circuit and in some placesdo not have any kerb and gutter. The intemal roads have a posted speed limit of $20 \mathrm{~km} / \mathrm{hr}$.

### 3.5 Future Intersection Upgrades

Historic discussions with RMS and Parramatta City Council have suggested that a number of intersection upgrades are being considered in the vicinity of the site. These include:

- Removal of the median strip at the Factory Street intersection to a llow cross traffic across O'C onnell Street and replacement of the existing intersection arrangement with a roundabout.
- The intersection at O'C onnell Street with Fennell Street is being considered for an upgrade to either traffic signals (Council's preferred choice) or a roundabout (RMS' preferred choice) to address road safety concems.
- The Windsor Road bridge overthe Cumberland Highway is proposed to be widened as part of the proposed Westem Sydney Regional Ring Road.


### 3.6 Access

The subject area can be accessed from the north via Windsor Road then using Bamey Street, Factory Street, Grose Street or Vic toria Road. Accessfrom the north-east to the site can also be gained from Pennant HillsRoad then via Albert Street or Bamey Street, Dunlop Street, Factory Street, Grose Street or Victoria Road via Church Street.

From the south, it can be accessed via O'C onnell Street or Marden Street/Victoria Road/O'Connell Street.

Access from the east can be gained from Victoria Road via O'Connell Street or Church Street.

There is a vehic ular bridge which links the site to Bridge Road on the westem side of the Pa ramatta River. This bridge link is controlled by boom gates (although these are regularly left open during the day).

The main access to Cumberland Precinct is currently from Fleet Street at Greenup Drive (near the westem end of Albert Street). A separate access is also available off $O^{\prime}$ Connell Street near Broad Street. However, this access road is provided as an unsealed road linking to an isolated area to the north of the precinct. There is no connection provided to the intemal loop road.

The access to the Sports and Leisure site is currently provided along O'Connell Street and off the Grose Street extension.

### 3.7 Historic al Traffic Flows

Historical a verage daily traffic flows have been sourced from RMS for a number of selected locations in the vicinity of the site. These are presented in Table 3.1.

Table 3.1: Average Daily Traffic Rows

| Location | 2002 | 2005 | 2009 | 2012 |
| :--- | :---: | :---: | :---: | :---: |
| Church St, South of Albert St | 29,771 | 28,490 | 27,490 | - |
| Church St, South Of North Rocks Rd | - | - | 32,934 | 32,362 |
| Pennant Hills Rd, South of J ames Ruse Dr | - | - | 14,289 | 14,592 |
| O'Connell St at bridge over Pa ramatta River | 30,275 | 29,099 | 28,519 | - |
| O'Connell St, North of Factory St | 14,315 | 12,933 | - | - |
| Victoria Rd, East of Church St | 28,751 | 27,853 | 26,045 | - |

Source: Roads and Ma ritime Services
The historical traffic data indicates that traffic in the vicinity has generally decreased since 2002 by a rate of about one percent per annum. However, in the last few years the level of traffic appears to have stabilised.

### 3.8 Traffic and Parking Surveys

GTA Consultants commissioned the following surveys as part of this study:

- Intersection movement count and queue length surveys
- Origin-Destination surveys for two access points at the Cumberland hospital precinct
- Parking occupancy and duration surveys within the Cumberla nd hospital site
- Tra vel time surveys a long O'C onnell Street and Church Street.

Figure 3.3 presents the type and location of the surveys. The results of these surveysare presented below. Detailed surveyed results are also included in Appendix A.

Figure 3.3: Survey Plan


## Intersection Tuming Movement Count \& Queue Length Surveys

The traffic movement counts and queue length surveysat key roads in the vicinity of the site was undertaken on Saturday $9^{\text {th }}$ August and Thursday $14^{\text {th }}$ August 2014 during the following periods:

- 7:00am and 9:00am (for Thursday)
- $4: 00 \mathrm{pm}$ and $6: 00 \mathrm{pm}$ (for Thursday)
- 12:00pm and 2:00pm (for Saturday).

The following intersections were surveyed:

- Windsor Road/ Cumberland Highway
- Church Street/ The Junction Access
- Church Street/ North RocksRoad
- Church Street/ Board Street/ Seville Street
- Church Street/ Bamey Street
- Church Street/ Factory Street
- Church Street/ Albert Street/ Pennant HillsRoad
- Church Street/ Grose Street
- Church Street/ Victoria Road
- Church Street/ Market Street
- O'C onnell Street/ Board Street
- O'Connell Street/ Bamey Street
- O'Connell Street/ Dunlop Street
- O'C onnell Street/ Factory Street
- O'C onnell Street/ Fennell Street
- O'C onnell Street/ Albert Street
- O'C onnell Street/ Grose Street
- O'Connell Street/ Victoria Road
- O'C onnell Street/George Street
- Victoria Road/ Marsden Street
- Victoria Road/ Wilde Avenue
- Factory Street/ New Street
- Greenup Drive/ Fleet Street
- Marsden Street/ Market Street.

The survey results indicated the peak hours were genera lly between 7:45-8:45a m for the Thursday AM, 4:30-5:30pm for the Thursday PM and 12-1pm for the Saturday midday.

It is noted that C hurch Street, south of Market Street was closed during the surveyed days. This section of the road is to be closed until December 2014 due to construction works on the Lennox Bridge. Hence, all traffic using Church Street, south of Market Street has been detoured to Marsden Street or Wilde Avenue.

In order to predict the effect of this road closure on other nearby intersections, GTA has obtained SCATS (Sydney Coordinated Ada ptive Traffic System) counts from RMS at Victoria Road intersections at Marist Place, Church Street and Wilde Avenue on the same surveyed days (i.e. with road closure) and March 2014 (i.e. without road closure). The comparison of traffic flows on Victoria Road, Church Street, Marist Place and Wilde Avenue with and without the road closure south of Market Street indicated that there is no substantial difference in tuming movement flows at intersections along Victoria Road.

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Both Market Street intersections at Marsden Street and Church Street are non-signa lised intersections. Hence, historic ally data is unava ilable from RMS. Without historic al data (i.e. without road closure), the effect of road closure on these two local intersections cannot be predicted. Nevertheless, the operating conditions at Market Street intersections at Marsden Street and Church Street as shown in Table 3.7 indicate that both intersections currently operate at level of service A with ample spare capacities.

Table 3.2 summarises the mid-block two-way peak hour flows derived from the intersection tuming movement flows for the Thursday AM, Thursday PM and Saturday midday peak hours.

RMS guidelines indic ates that a rterial roads generally ha ve daily flows greater than 20,000 vehic les per day (vpd) and sub-arterial roads have daily flows between 5,000 vpd to $20,000 \mathrm{vpd}$. Other roads have daily flows of 10,000 vpd or less. Typically, peak hour flows are approximately 8 to 10 percent of the daily flows. The surveyed flows are generally within these limits.

## Existing Conditions

Table 3.2: 2014 Surveyed Mid-block Rows

| Location | Thursday AM |  |  | Thursday PM |  |  | Saturday Midday |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | NB/ <br> EB | $\begin{aligned} & \text { SB/ } \\ & \text { WB } \end{aligned}$ | Twoway | $\begin{gathered} \text { NB/ } \\ \text { EB } \end{gathered}$ | $\begin{aligned} & \text { SB/ } \\ & \text { WB } \end{aligned}$ | Twoway | $\mathrm{NB} /$ | $\begin{aligned} & \hline \text { SB/ } \\ & \text { WB } \end{aligned}$ | Twoway |
| Church St, south of Vic toria Rd | 228 | 319 | 547 | 304 | 251 | 555 | 236 | 347 | 583 |
| Church St, north of Vic toria Rd | 472 | 950 | 1422 | 824 | 654 | 1478 | 592 | 739 | 1331 |
| Church St, south of Penna nt Hills Rd | 583 | 1247 | 1830 | 1150 | 730 | 1880 | 852 | 915 | 1767 |
| Church St, south of Factory St | 430 | 862 | 1292 | 809 | 567 | 1376 | 649 | 691 | 1340 |
| Church St, south of Bamey St | 441 | 874 | 1315 | 849 | 563 | 1412 | 667 | 660 | 1327 |
| Church St, south of Board St | 694 | 1772 | 2466 | 1662 | 1008 | 2670 | 1042 | 1239 | 2281 |
| Church St, south of North Rocks Rd | 893 | 1867 | 2760 | 2079 | 1014 | 3093 | 1342 | 1255 | 2597 |
| Church St, south of J a mes Ruse Dr | 861 | 1858 | 2719 | 2128 | 1190 | 3318 | 1562 | 1552 | 3114 |
| Church St, north of J ames Ruse Dr | 848 | 2956 | 3804 | 2308 | 1632 | 3940 | 1729 | 1946 | 3675 |
| O 'Connell St, south of George St | 1966 | 1238 | 3204 | 1319 | 1315 | 2634 | 1101 | 1102 | 2203 |
| O 'Connell St, south of Vic toria Rd | 1323 | 1498 | 2821 | 1194 | 1415 | 2609 | 872 | 1161 | 2033 |
| $\mathrm{O}^{\prime}$ 'Connell St, south of Grose St | 921 | 1120 | 2041 | 971 | 844 | 1815 | 627 | 808 | 1435 |
| O 'Connell St, south of Albert St | 540 | 1039 | 1579 | 939 | 596 | 1535 | 520 | 694 | 1214 |
| O 'Connell St, south of Bamey St | 377 | 814 | 1191 | 731 | 401 | 1132 | 396 | 494 | 890 |
| O 'Connell St, south of Board St | 281 | 15 | 296 | 379 | 23 | 402 | 310 | 23 | 333 |
| Fleet St, south of Albert St | 160 | 54 | 214 | 74 | 171 | 245 | 54 | 47 | 101 |
| Fleet St, south of Factory St | 22 | 85 | 107 | 99 | 24 | 123 | 30 | 22 | 52 |
| Ma rist St, south of Market St | 505 | 560 | 1065 | 777 | 512 | 1289 | 648 | 601 | 1249 |
| Marist St, south of Vic toria Rd | 329 | 267 | 596 | 553 | 277 | 830 | 482 | 295 | 777 |
| Wilde Ave, south of Victoria Rd | 383 | 1132 | 1515 | 878 | 537 | 1415 | 364 | 379 | 743 |
| Market St, east of Marist St | 200 | 332 | 532 | 272 | 267 | 539 | 216 | 345 | 561 |
| Vic toria Rd, east of O'Connell St | 504 | 541 | 1045 | 447 | 635 | 1082 | 374 | 444 | 818 |
| Vic toria Rd, east of Marist St | 603 | 612 | 1215 | 687 | 734 | 1421 | 603 | 586 | 1189 |
| Vic toria Rd, east of Church St | 1245 | 874 | 2119 | 990 | 1187 | 2177 | 838 | 811 | 1649 |
| Grose St, east of O 'C onnell St | 275 | 193 | 468 | 181 | 222 | 403 | 127 | 152 | 279 |
| Grose St, west of Church St | 255 | 464 | 719 | 290 | 308 | 598 | 195 | 285 | 480 |
| Fennell St, west of O 'C onnell St | 63 | 235 | 298 | 181 | 68 | 249 | 93 | 73 | 166 |
| Fennell St, east of O 'Connell St | 74 | 20 | 94 | 68 | 40 | 108 | 34 | 21 | 55 |
| Albert St, west of Fleet St | 40 | 210 | 250 | 238 | 68 | 306 | 28 | 25 | 53 |
| Albert St, east of O 'Connell St | 219 | 288 | 507 | 298 | 248 | 546 | 177 | 223 | 400 |
| Pennant Hills Rd, east of Church St | 413 | 767 | 1180 | 646 | 492 | 1138 | 440 | 501 | 941 |
| Factory St, east of Fleet St | 12 | 18 | 30 | 32 | 7 | 39 | 6 | 8 | 14 |
| Fac tory St, east of O 'C onnell St | 8 | 38 | 46 | 13 | 26 | 39 | 10 | 39 | 49 |
| Dunlop St, west of O 'Connell St | 19 | 86 | 105 | 78 | 27 | 105 | 31 | 22 | 53 |
| Dunlop St, east of O'Connell St | 16 | 18 | 34 | 28 | 13 | 41 | 23 | 12 | 35 |
| Bamey St, east of O 'C onnell St | 115 | 824 | 939 | 387 | 403 | 790 | 139 | 515 | 654 |
| Board St, east of O'Connell St | 269 | 16 | 285 | 380 | 18 | 398 | 313 | 23 | 336 |
| North Rocks Rd, east of Church St | 663 | 830 | 1493 | 713 | 680 | 1393 | 665 | 700 | 1365 |

NOTE: NB - Northbound; EB - Eastbound; SB - Southbound; WB - Westbound
The intersection tuming movement flows at the surveyed locations are presented in Appendix A.1.

The results of the queue length surveys are also presented graphic ally in Figure 3.4 to Figure 3.6 for the different surveyed peak periods.

Figure 3.4: Maximum Queue Length Recorded on Thursday 7-9AM


Figure 3.5: Maximum Queue Length Recorded on Thursday 4-6PM


Figure 3.6: Maximum Queue Length Recorded on Saturday 12-2PM


## Origin-Destination Surveys

Origin-destination surveys for two access roads serving the Cumberland hospital precinct was undertaken at the same period as the intersection count surveys. The surveys were undertaken to establish the extent of rat-running traffic from the Westmead Hospital through the Cumberland Hospital site. The locations of the access roads are shown in Figure 3.3.

This survey involved recording number plates of all vehic lespassing Bridge Road, west of the Cumberland hospital precinct and Greenup Drive, west of Fleet Street. The result of the origindestination survey is presented in Table 3.3. A detailed surveyed data is included Appendix A.2.

Table 3.3: Origin-Destination Survey

| Surveyed <br> Period | No. of Vehic les Recorded (2 hr period) |  |  | No. of Vehic les Passing Two Access <br> Points (during 2 hr period) |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Greenup Dr | Bridge Rd | Total | Eastbound | Westbound | Two-way |
| Thursday 7-9AM | 382 | 252 | 634 | 30 | 106 | 136 (21\%) |
| Thursday 4-6PM | 117 | 164 | 281 | 125 | 98 | 223 (79\%) |
| Saturday 12-2PM | 65 | 46 | 111 | 27 | 40 | 67 (61\%) |

NOTE: Figures in parenthesis presents the percentage of vehicles passing the Cumberland hospital precinct over the 2 hour period.
The results indic ate that a signific ant proportion of vehic les using the two access roads serving the Cumberland hospital precinct do not have destination within the hospital site and are rat-running through the hospital precinct. This behaviour is more domina nt during the weekday aftemoon period with about 79 percent of vehic les using the two access points as a through link.

## Parking Occupancy \& Duration Surveys

Parking occupancy and duration surveys were undertaken on the Cumberland Hospital site on Saturday $9^{\text {th }}$ of August and Thursday $14^{\text {th }}$ August 2014 during the following periods:

- 7:00am and 7:00pm (for Thursday)
- 9:00am and 5:00pm (for Saturday).

A boundary of the parking surveys is shown in Figure 3.3. The survey was undertaken to establish whether there was an element of parking at the hospital by people who were not working or visiting the hospital (i.e. they were using the site as a free long term carpark but working elsewhere).

The survey of parking inventory indic ated that the Cumberland hospital precinct provides about 1,005 car parking spaces. About 105 spaces are on-street car parking spaces and about 900 spacesare provided within on-site carparks.

Table 3.4: Summary of Parking Occupancy and Duration Surveys

|  | Thursday (7am-7pm) | Saturday (9am-5pm) |
| :---: | :---: | :---: |
| Supply/Capacity | 1005 | 1005 |
| Average Occupancy (\%) | $49 \%$ | $17 \%$ |
| Maximum Occupancy (\%) | $70 \%$ | $19 \%$ |
| Average Duration of Stay (h:mm) | $4: 04$ | $4: 26$ |
| Maximum Duration of Stay (h:mm) | $12: 00$ | $8: 00$ |
| Total Users (no. of vehicles) | 1454 | 306 |

Figure 3.7 and Figure 3.8 show the parking duration and occupancy profiles, respectively.

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Figure 3.7: Duration of Parking Profiles for Thursday and Saturday



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Figure 3.8: Parking Occupancy Profiles forThursday and Saturday


The results of parking occupancy and duration surveys indic ate that:

- The average parking occupancy across the hospital precinct is $49 \%$ (for Thursday) and 17\% (for Saturday).
- The maximum parking occupancy recorded is $70 \%$ (for Thursday) and $19 \%$ (for Saturday).
- The average parking duration is about 4 to 4.5 hours.

There is no clearevidence from the parking surveys that the hospital site may be used as long tem carpark for people working elsewhere such as the Westmead hospital. The average parking occupancy for parking areas in the vicinity of the Bridge Road access remainssimilar to the average occupancy of the overall hospital precinct.

A detailed surveyed data is included Appendix A.3.

## Travel Time Surveys

Travel time surveys were camied out along O'Connell Street and Church Street on Saturday $9^{\text {th }}$ of August and Thursday 14 ${ }^{\text {th }}$ of August 2014 during the following peak periods:

- 7:00am and 9:00am (for Thursday)
- 4:00pm and 6:00pm (for Thursday)
- 12:00pm and 2:00pm (for Saturday).

The travel time survey routes along O'Connell Street and Church Street are shown in Figure 3.3.
The a verage travel time survey results are presented in Table 3.5.
Table 3.5: Surveyed Average Travel Time

| Survey Period |  |  <br> Cumberland Hwy Average Travel Time |  |  <br> Board St Average Travel Time |  |
| :--- | :---: | :---: | :---: | :---: | :---: |
|  |  | Southbound | Northbound | Southbound |  |
|  |  | $5: 44$ | $1: 53$ | $2: 52$ |  |
| Thursday 4-6PM | $6: 28$ | $4: 54$ | $2: 23$ | $2: 33$ |  |
| Saturday 12-2PM | $4: 28$ | $5: 17$ | $2: 00$ | $2: 24$ |  |

The results indic ate that the average travel time for northbound traffic is longest during the aftemoon peak period for both Church Street and O'C onnell Street. The average travel time for southbound traffic is longest during the moming peak period. The recorded travel time during the Saturday midday period is generally less than the Thursday moming and aftemoon peak periods.

The tra vel time surveys were undertaken to assist with calibrating the traffic model. A detailed surveyed data is included Appendix A.4.

### 3.9 Traffic Signal Operation

GTA has also obtained LX files of the study area and the Intersection Diagnostic Monitor (IDM) at the following signalised intersections from RMS:

- Windsor Road/Cumberland Highway
- Windsor Road/ The Junction Access Road
- Church Street/North Rocks Road
- Church Street/Bamey Street
- Church Street/Fa ctory Street
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- Church Street/Pennant Hills Road/Albert Street
- Church Street/G rose Street
- Church Street/Victoria Road
- Victoria Road/Marist Place
- Victoria Road/Wilde Avenue
- O'Connell Street/Albert Street
- O'Connell Street/Grose Street
- O'Connell Street/Victoria Road
- O'Connell Street/George Street.

The LX file is the data file that feeds into the region computer. It contains the data necessary for communic ations, signal timing, coordination and variation routines.

IDM data are used to validate the operation of the traffic signals in the model. IDM data forthe above intersections are pre-arranged with RMS to be on the same day as other traffic data. The data files contain a record of which phases and split plans were called, which link plans were used as well as phase times and cycle times throughout the monitored period. This data has been used to calibrate the model.

### 3.10 Intersection Operation

The operation of the key intersections within the study area has been assessed using LinSig/SIDRA INTERSECTION ${ }^{1}$, a computer based modelling package which calculates intersection performance.

In general, most of signalised intersections along Church Street, O'Connell Street and Vic toria Road were modelled using LinSig program and the rest, using SIDRA INTERSEC TION program. Table 3.7 presents the intersection performance and also indicates which modelling package has been used to model the particular intersections.

The commonly used measure of intersection performance, as defined by the RMS, is vehicle delay. SIDRA INTERSEC TION determines the average delay that vehic les enc ounter and provides a mea sure of the level of service. Table 3.6 shows the level of service criteria.

[^0]
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Table 3.6: Level of Senvice (LOS) Criteria

| Level of Service (LoS) | Average Delay per <br> vehicle (secs/veh) | Traffic Signals, <br> Roundabout | Give Way \& Stop Sign |
| :---: | :---: | :---: | :---: |
| A | Less than 14 | Good operation | Good operation |
| B | 15 to 28 | Good with acceptable <br> delays and spare capacity | Acceptable delays and <br> spare capacity |
| C | 29 to 42 | Satisfactory | Satisfactory, but accident <br> study required |
| D | 53 to 56 | Nearcapacity | Nearcapacity, ac cident <br> study required |
| E | Greaterthan 70 | At capacity, at signals <br> incidentswill cause <br> excessive delays | At capacity, requires other <br> control mode |
| F | Extra capacity required | Extreme delay, major <br> treatment required |  |

Table 3.7 presents a summary of the existing operation of the intersection for the Thursday AM, Thursday PM and Saturday midday peak hours, with full results presented in Appendix B of this report.

The results presented in Table 3.7 indicate that Windsor Road/Cumberland Highway intersection c urrently operates with level of service (LoS) F for all three peak periods. Similarly, O'Connell Street/Fennell Street intersec tion operates at capacity with LoS E/F.

A number of signa lised intersections a long Church Street (i.e. at North Rocks Road, Bamey Street and Victoria Road intersections) operate at nearcapacity during at least one of the modelled peak period.

Intersec tions along O'Connell Street generally operate at LoS B or better except for Fennell Street intersection mentioned above.
$\qquad$

Table 3.7: Existing Operating Conditions

|  | Intersections | Control Type | Thursday AM |  | Thursday PM |  | Saturday Midday |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Level of Service | Average Delay (sec) | Level of Service | Average Delay (sec) | Level of Service | Average Delay (sec) |
| (LinSig) | Windsor Rd/ Cumberland Hwy | Signal | F | 73 | F | 119 | F | 83 |
| (LinSig) | Church St/ The J unction Access | Signal | A | 10 | B | 19 | B | 19 |
| (LinSig) | Church St/ North RocksRd | Signal | D | 50 | B | 26 | C | 33 |
| (SIDRA) | Church St/ Board St/ Seville St | Priority | C | 40 | C | 33 | B | 26 |
| (LinSig) | Church St/ Bamey St | Signal | C | 40 | D | 47 | C | 33 |
| (LinSig) | Church St/ Factory St | Signal | B | 16 | A | 13 | A | 14 |
| (LinSig) | Church St/ Albert St/ Pennant Hills Rd | Signal | C | 40 | B | 29 | B | 26 |
| (LinSig) | Church St/ Grose St | Signal | C | 35 | C | 30 | B | 24 |
| (LinSig) | Church St/ Victoria Rd | Signal | C | 32 | D | 49 | E | 62 |
| (SIDRA) | Church St/ Market St | Priority | A | 9 | A | 9 | A | 9 |
| (SIDRA) | O'Connell St/ Board St | Pronity | A | 9 | A | 9 | A | 9 |
| (SIDRA) | O'Connell St/ Bamey St | Priority | A | 13 | A | 10 | A | 10 |
| (SIDRA) | O 'Connell St/ Dunlop St | Priority | A | 14 | A | 14 | A | 12 |
| (SIDRA) | O 'Connell St/ Factory St | Priority | B | 16 | B | 15 | A | 13 |
| (SIDRA) | O'Connell St/ Fennell St | Priority | F | 82 | F | 117 | E | 59 |
| (LinSig) | O'Connell St/ Albert St | Signal | B | 19 | A | 14 | B | 17 |
| (LinSig) | O'Connell St/ Grose St | Signal | B | 21 | B | 21 | B | 17 |
| (LinSig) | O 'Connell St/ Vic toria Rd | Signal | C | 31 | C | 30 | C | 35 |
| (SIDRA) | O 'Connell St/George St | Signal | B | 18 | A | 12 | A | 13 |
| (LinSig) | Victoria Rd/ Marsden St | Signal | C | 37 | D | 51 | C | 39 |
| (LinSig) | Vic toria Rd/ Wilde Ave | Signal | C | 40 | C | 34 | B | 25 |
| (SIDRA) | Factory St/ New St | Priority | A | 9 | A | 9 | A | 9 |
| (SIDRA) | Greenup Drive/ Fleet St | Priority | A | 9 | A | 9 | A | 9 |
| (SIDRA) | Marsden St/ Market St | Priority | A | 10 | A | 11 | A | 10 |



## Existing Conditions

### 3.11 Existing Mode Share

The 2011 CensusJ oumey to Work (JTW) data provides the most robust pic ture of existing travel pattemsto and from the Cumberland hospital and Sports/Leisure precincts. The smallest geographical area for which JTW data is available is known as a travel zone (TZ).

The subject site is located in travel zones 1018 and 1025, asshown in Figure 3.9. According to the 2011 J TW data, these two travel zones provide employment for about 2,600 people and about 20 people reside within this area.

Figure 3.9: Travel Zones 1018 \& 1025


Background Image Source: Bureau of Transport Statistics website (http://visual.bts.nsw.gov.au/jtwbasic/\#1018,1025)
Table 3.8 presents the travel mode share of employees travelling to TZs 2018 and 1025.
Table 3.8: 2011 J oumey to Work Data

| Travel Mode | Percentage |
| :---: | :---: |
| Car(driver) | $79 \%$ |
| Car(passenger) | $6 \%$ |
| Train | $\mathbf{7 \%}$ |
| Bus | $3 \%$ |
| Walked only | $3 \%$ |
| Mode not stated | $\mathbf{2 \%}$ |
| Total | $\mathbf{1 0 0 \%}$ |

Data Source: Bureau of Transport Statistics website (http://visual.bts.nsw.gov.au/jtwbasic/\#1018,1025)

Table 3.8 indic ates that percentage of people travelling by carmode (including drivers and passengers) is about 85 percent. The percentage of people using public transport mode is about 10 percent.

The results ind ic a te that there is currently a high reliance of private vehicle usage of people travelling to work within the subject site.

### 3.12 Existing Public Transport Services

The site is located in close proximity to Pa rramatta CBD and Westfield Shopping Centre.
It is approximately 2.3 km in terms of walking distance to Parramatta Railway Station. As such, whilst not considered as being Transit Orientated Development (TOD), the site is within a walkable catc hment of the CBD in the same way that Sumy Hills or Redfem are connected to Sydney CBD.

The site is presently served by bus servic es operated by Sydney Buses as well as Hills Bus. These servicescan either be accessed from O'Connell Street and/or C hurch Street. It is noted that Church Street has designated buslanes in both directions as indic ated previously. Much of the subject site is within 400 m of bus stops a long $\mathrm{O}^{\prime} \mathrm{C}$ onnell Street, which is generally accepted as the distance that public transport usersare prepared to walk. Figure 3.10 presents a map of the existing bus routes in the area that are operated by Sydney Buses, while servicesprovided by Hills Bus are shown in Figure 3.11.

Figure 3.10: Existing Bus Routes Operated by Sydney Buses


Figure 3.11: Existing Bus Routes Operated by Hills Bus


These services connect the Cumberland Precinct site with the Parramatta CBD (and railway station) as well as Carlingford, Epping and Macquarie Park to the north east of the site and to Castle Hill, Rouse Hill and Homsby to the north.

Table 3.9 presents the number of (inbound and outbound) services for the three hour moming (6:30am to 9:30am) and evening (4:00pm to 7:00pm) peak periods.

Table 3.9: Existing Bus Service

| Bus Route | Bus Operator | Moming Peak Period | Evening Peak Period |
| :---: | :---: | :---: | :---: |
| 549 | Sydney Buses | $7(7)$ | $6(7)$ |
| M54 | Sydney Buses | $18(18)$ | $18(18)$ |
| 600 | Hills Bus | $3(1)$ | $-(-)$ |
| 601 | Hills Bus | $10(8)$ | $7(10)$ |
| 603 | Hills Bus | $6(2)$ | $3(6)$ |
| 604 | Hills Bus | $5(2)$ | $4(5)$ |
| 606 | Hills Bus | $6(4)$ | $4(5)$ |
| 609 | Hills Bus | $6(1)$ | $-(6)$ |
| 625 | Hills Bus | $6(5)$ | $6(7)$ |
| $M 60$ | Hills Bus | $15(15)$ | $17(19)$ |

$\infty$ This service operates along Church Street/Pennant Hills Road. The nearest bus stop on this service is some 800 m walking distance from the site.
7 (8) - Inbound (Outbound)
The bus services in the vicinity of the site have a combined frequency of approximately one minute orless.

### 3.13 Parramatta Shuttle Bus

The Parramatta Shuttle Bus is a free service that connects commuters to the commercial, retail and recreational landmarks of the city. The shuttle service operates in a loop with major attractions along its course including Parramatta Wharf, Transport Interchange, Parramatta Library, Westfield and Parramatta Park.

The Shuttle Bus operates within the vicinity of the Parramatta Square, primarily along Macquarie Street, Darcy Street, Argyle Street and Marsden Street.

The bus route and bus stops of the Parramatta Shuttle Bus service are shown in Figure 3.12.
Figure 3.12: Parramatta Free Shuttle Bus Service Route


Image Source: TFNSW

### 3.14 Cycle and Pedestrian Networks

Information available from Parramatta City Council website indic ates that there are currently onroad bicycle routes nearthe site. These include routesalong O'Connell Street and Fleet Street as well as along Factory Street. Council'sinformation also ind ic ates that the route along Fleet Street branches out into the site via Greenup Drive. This then continues along Eastem Circ uit to connect to Bridge Road to link across the Pa rramatta River into Westmead Hospital.

Existing a vailable bicycle network is shown in Figure 3.13.
Figure 3.13: Existing Bicycle Network Map


However, from our site observations, bic ycle symbols (indicating an on road bicycle route) were observed on both sides of O'Connell Street, but not on the other streets mentioned above.

Council's Bike Plan, as show in Figure 3.14, containsa map depicting both the existing and proposed bike routes. In the vicinity of the site, it showsonly a proposed off road route along Church Street.

Figure 3.14: Council's Bike Plan


Source: Parramatta City Council Website
Pedestrian facilities are available in the area by way of constructed footpaths on one or both sides of the road.

However, the only controlled pedestrian crossing in the vicinity of the site is that at the signalised intersection of O'C onnell Street with Albert Street. However, there are a number of controlled crossings are loc ated along Church Street including those at Albert Street and Factory Street.

There are presently no formally identified bicycle routes within the Cumberland Precinct site. Simila rly, pedestrian facilities are also very limited.

## 4. Development Proposal

### 4.1 Background

Pa rramatta North Urban Renewal a rea (PNUR) is located to the west and north-west of the Pa ramatta CBD, Sydney's second CBD. Pa rramatta is loc ated in the geographical heart of Sydney and plays a significant role as the most important centre in Westem Sydney. The PNUR is located to the immediate east of the Westmead Health campus, separated by the Parramatta River.

The PNUR is a 146 Ha area and has been divided into four distinct Precincts as shown in Figure 4.1, comprising of:

- The Cumberland Precinct ( 40 Ha )
- Sport and Leisure Precinct ( 21 Ha )
- Old Kings School Precinct (4 Ha), a nd
- Parramatta Park Precinct (81 Ha).

This Study has been prepared in orderto identify how best to plan for the urban renewal of the Cumberland Precinct and the Sports and Leisure Precinct only noting that the Parramatta Gaol and Lot 1 are not part of the current rezoning. The recommended planning controls have been prepared recognising the locational advantages of the PNUR to the Parramatta CBD, the Westmead Health Precinct, the Rydalmere Education Precinct, and transport options.

The renewal of the area provides exceptional opportunities for the delivery of housing, cultural and community uses, and the capacity to protect, enhance and re-use signific ant heritage buildings and structures.

Figure 4.1: Precinct Map


### 4.2 Proposed Development

The Parramatta North Urban Renewal (PNUR) a rea providesopportunities to protect and enhance heritage signific ant sites, and deliver housing, cultural uses and employment on the edge of the Parramatta CBD. The a rea is also exceptionally well loc ated in close proximity to the Westmead Health and Rydalmere Education specialised precincts, as well asexisting and planned transport.

The location at the edge of the Parramatta CBD also places the area at the westem extent of the Global Economic Comidor and Pa ramatta Road Comidor. These locational advantages, in concert with the proximity to the Westem Sydney Employment Area, underline the strategic ments of the urban renewal of the area. PNUR includes many locational and site specific attributes, including frontage to the Parramatta River and a rich history of Aboriginal, early colonial, nineteenth and twentieth century uses. The potential exists to deliver housing and employment opportunities in a precinct that will embrace and interpret these heritage attributes to make them a focus of the urban environment that will emerge through future development.

The Study has been undertaken to prepare an appropriate suite of planning controls to guide the urban renewal of the area and future development. This hasled to an Indicative Layout Plan (ILP) guiding future open space, transport links a nd building footprints, as well as zoning and height of building controls, which are to be implemented in conjunction with site specific Development Control Plan provisions to guide the fine grain development of the area.

This suite of controls has had regard to the site's heritage, environmental values and physic al constraints.

The ILP envisages the creation of a mixed use area within the Cumberland Precinct that accommodates new development for housing, employment, cultural and community uses in new buildings and through the adaptation of existing heritage buildings. For the Sports and Leisure Precinct, the ILP envisages the strengthening of the current role of the area asa major sports venue and the introduction of allied retail and commercial uses to support the role of Pa rramatta Stadium as a major sport and entertainment venue for Pa ramatta and greater westem Sydney. The Sports and Leisure Precinct may also accommodate ancillary retail to support the resident and employee population to be accommodated in the PNUR.

The study proposes a mendments to the planning framework, including revisions to the development controls that will facilitate a mixed use residential redevelopment of the study area. The proposed amended planning framework will facilitate the lodgement of future development applications for the land in the study area which are anticipated to achieve the following development yields:

- Cumberland Precinct
- Approximately 4,100 dwellings
- Approximately $28,000 \mathrm{~m}^{2}$ GFA of adaptive reuse of reta ined heritage buildings
- Up to $4,000 m^{2}$ GFA of retail space
- Sports and Leisure Precinct
- Approximately $34,000 \mathrm{~m}^{2}$ GFA of mixed-use (likely to be predominantly commercial).

Figure 4.2 shows the boundary of the proposed rezoning.

Figure 4.2: Rezoning Boundary


For the purposes of this assessment of potential traffic generation impacts, a higher development yield has been assumed to reflect the outcome of a future redevelopment of la nd not included as part of the overall study into the a mendment of the planning framework. For the purpose of traffic impact modelling the development has assumed a development yield potential of:

- Cumberland Precinct
- Approximately 5,600 dwellings.
- Approximately $35,000 \mathrm{~m}^{2}$ GFA of adaptive reuse of retained herita ge buildings. Of which about $11,000 \mathrm{~m}^{2}$ (i.e. about 140 dwellings) would be used as residential area and the rema ining (i.e. $24,000 \mathrm{~m}^{2} \mathrm{GFA}$ ) would be used for commercial area.
- Up to $4,000 m^{2}$ GFA of retail space.
- Sports and Leisure Precinct
- Approximately $46,000 \mathrm{~m}^{2}$ GFA of mixed-use. Of which $75 \%$ of the area has been estimated to be commercial area and $25 \%$ of the area would be for residential area (i.e. about 130 dwellings).

The above potential development yields have been a dopted for the purpose of the traffic assessment and modelling.

### 4.3 Proposed Layout

Figure 4.3 presents the indic ative layout plan for the proposed development. As discussed above, for the purpose of the traffic assessment, a higher development yield has been assumed. This inc ludes the potential for nea rby future developments, which are not part of the current rezoning. Figure 4.4 shows the location of potential developments and the indic ative development yields for the traffic assessment puposes.

Figure 4.3: Indicative Layout Plan


Figure 4.4: Development Location \& Indic ative Development Yields (for Traffic Assessment Purposes Only)


NOTE: The retail areas (i.e. $4,000 \mathrm{~m}^{2}$ in total) are included as the commercial component.

### 4.3.1 Road Layout

A number of new roads and new connections have been proposed to accommodate the proposed development. Forthe traffic assessment purposes, it is a ssumed that the intemal roads will be connected forthe overall development (i.e. including development on Pa ramatta Gaol and SES lot, which are not part of the current rezoning).

In summary, the following new links and changes to the existing intersections are proposed:

- Bamey Street, west of O'Connell Street, forming a new westem approach at the O'Connell Street-Bamey Street intersection (i.e. proposed 4-way intersection)
- Dunlop Street, west of New Street, forming a new westem approach at the New StreetDunlop Street intersection (i.e. proposed 3-way intersection)
- Factory Street, west of Fleet Street, forming a new westem approach at the Fleet StreetFactory Street intersection (i.e. proposed 4-way intersection)
- A through link that joins Albert Street, west of O'Connell Street and Greenup Drive, east of Fleet Street. This forms a new eastem approach at the Fleet Street-Greenup Drive intersection (i.e. proposed 4-way intersection).


### 4.3.2 Pedestrian \& Cycleway Connec tivity

It is intended to construct a cycleway along the waterfront which will run from north of the site to south of the Sports Precinct. This will tie into existing cycle ways and those being planned by others. The proposed cycleway includes 1.75 km of cycle path to connect areas either side of the development, as shown on the indic ative layout plan.

Figure 4.5 shows the proposed pedestrian and cycleway for the proposed development as well as the existing and proposed cycleway in accordance with Pa ramatta City Councils, Pa ramatta Bike Plan 2009.

### 4.3.3 Public Transport Connectivity

It is proposed to provide a good quality shuttle bus service between the subject site and the Pa ramatta interchange. Figure 4.6 shows the possible shuttle bus route. Similar to the existing Pa rramatta Free Shuttle Bus, it would be a one-way loop service starting from Parramatta Interchange travelling northbound along O'Connell Street then using Bamey Street/Castle Street/Factory Street to retum to Church Street travelling southbound to join the current Free Shuttle Bus route.

The shuttle bus route and the location of bus stops will be refined during the later stage in consultation with the public.

It is the intention that this service is to be provided at least every 10 minutes in the weekday peak periods and every 20 to 30 minutes during othertimes of the day and at the weekend.

Figure 4.5: Proposed Pedestrian \& Cycleway Network


Reproduced from http://www.parracity.nsw.gov.au/_data/assets/pdf file/0004/34843/Pa ramatta BikePlan.pdf

Figure 4.6: Proposed Shuttle Bus Service Route between Panamatta Interchange and the Site


Reproduced from http://www.transportnsw.info/resources/documents/maps/pa ramatta-shuttle-map.pdf

## 5. Parking Provision

### 5.1 CarParking

Carparking requirement for the proposed development has been a ssessed against Pa rramatta City Council's development control plan, na mely Development C ontrol Plan 2011 Part 3 (Section 3.6.2 Parking and VehicularAccess).

## Residential Development

In relation to high density residential developments, the DCP stipulated separate parking rates for develop ments located within and not within 400m walking distance of a railway station or transitway busstop with an average service frequency of 10 minutes or less. These rates are presented in Table 5.1.

Based on our experience elsewhere, we have assumed the 5,870 residential apartments would have the following mix:

- one-bedroom units- 2,640 (45 per cent)
- two-bedroom units-2,350 (40 percent)
- three-bedroom units- 880 ( 15 per cent).

The required parking provisions are presented in Table 5.1.
Table 5.1: DCP Parking Provision

| Unit Types | Number of Units | Within $\mathbf{4 0 0 m}$ |  | Not Within 400 m |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Parking <br> Provision | Parking Rate | Parking <br> Provision |  |
| 1-Bedroom Units | 2,640 | 1.0 per unit | 2,640 | 1.0 per unit | 2,640 |
| 2-Bedroom Units | 2,350 | 1.0 perunit | 2,350 | 1.25 perunit | 2,938 |
| 3-Bedroom Units | 880 | 1.2 perunit | 1,056 | 1.5 perunit | 1,320 |
| Visitors | - | 0.25 perunit | 1,468 | 0.25 perunit | 1,468 |
| Total | 5,870 |  | 7,514 |  | 8,366 |

It is expected that parking provision for a 5,870 high density resid ential apartment development at the proposed site would be in the range from 7,510 to 8,370 parking spaces using the Parramatta DCP parking provision rates.

## Commercial/Retail Development

Pa rramatta DCP 2011 also states the required carparking rates for "business premises and office premises" and "retail premises". These rates are presented in Table 5.2.

Table 5.2: Required Commercial/ Retail Parking Provision

| Land Use | Gross Foor Area (GFA) | Parking Rate | Parking Provision |
| :---: | :---: | :---: | :---: |
| Commercial | 58,500 | 1 per $50 \mathrm{~m}^{2}$ | 1,170 |
| Retail | 4,000 | 1 per $30 \mathrm{~m}^{2}$ | 133 |
| Total | 62,500 |  | 1,303 |

The required parking provision forcommercial and retail component would be in the order of 1,300 car parking spacesusing the Pa ramatta DCP parking provision rates.

## Total Development

If the parking provision of the proposed development is to be provided in accordance with the current Parramatta City Council's DCP, it is expected that total parking provision would be in the range of 8,820 to 9,770 carparking spaces.

Finally, to encourage a greatermodal shift to non-carmodes, a lower parking provision rate should be sought from the Council. It is believed that the provision of car parking for the proposed site should be approached innovatively and that site specific carparking provision rates should be agreed with the Council. This is disc ussed in Section 6.

In orderto achieve this, a number of possible measures which can be implemented to reduce the car dependency and encourage use of sustainable transport modes are proposed in Section 6 of this report.

### 5.2 Bicycle Parking

Bicycle parking requirement for the proposed development has been assessed against Pa ramatta City Council's DCP Part 3 (Section 3.6.2 Parking and VehicularAccess). These rates are presented in Table 5.3.

Table 5.3: Bic ycle Parking Provision

| Land Use | Development Size <br> (No. of dwellings/ area) | Bicycle Parking Rate | Bicycle Parking <br> Provision |
| :---: | :---: | :---: | :---: |
| Residential | 5,870 dwellings | 1 per2 dwellings | 2,935 |
| Commercial | $58,500 \mathrm{~m}^{2}$ | 1 per 200m |  |
| Retail | $4,000 \mathrm{~m}^{2}$ | 1 per $200 \mathrm{~m}^{2}$ | 293 |
| Total |  |  | 20 |

The required bicycle parking provision for the overall development would be about 3,250 spaces.
The DCP also stipulates that "Bicycle parking is to be provided in the form of Class 2 compounds, as specified in AS 2890.3 - Bicycle Parking Facilities. These facilities may be located in storage areas if good access is provided".

The Class 2 compounds will have medium level sec unity and are locked compounds with communal a ccess using duplic ate keys.

For commercial and retail developments, trip end facilities including showers and lockers would need to be provided to adequately service the bicycle users.

## 6. Travel Demand Management

Transport is a necessary part of life which has effects that can be managed. The transport sector is one of the fastest growing emissions sectors in Australia and travel demand management provides an opportunity for reducing greenhouse gases. As well as delivering better environmental outc omes, providing a range of travel choices with a focus on walking, cycling and public transport will have major public health benefits and will ensure a strong and prosperous community at the site and in the surrounding suburbs.

The planning of the new precinct will need to accommodate innovative ideas to manage the transport demand of the project. Whilst it will be necessary to manage the traffic impacts of the development, it will be necessary to introduce new measures to ensure that the movement trips generated by the proposed development are not all carbased (particularly single occupancy trips).

### 6.1 Potential Measures

Some of the measures that will be incorporated to minimise single vehicle cartravel are:

- limited parking ratios
- busimprovements
- cycle parking /facilities
- carsharing/carclub cars
- green travel plan.


## Car Parking Ratios

One of the most effective ways to reduce traffic congestion and pollution, and encourage a shift to susta inable transportation modes, is through parking reform.

Excessive off-street parking requirements can ha m the environment by encouraging traffic and its associated pollution, high parking requirements can make housing prohibitively expensive to build, particula ly for affordable housing especially where the cost of land is relatively high. Every parking space increases the a mount of land that needs to be developed and each parking space can cost up to $\$ 40,000$ per space.

Any reduction in such parking rates does however require the provision of altemative good quality non-c ar ba sed transport.

## Bus Improvements

As described earlier in the text, it is the intention to provide a good quality shuttle bus between the subject site and the Parramatta interchange. The recent introduction of Opal cards means that transfers/ changing modes at such interchanges is much easier and without the historic cost implications.

It would be the intention forsuch a service to be at least every 10 minutes in the weekday peak periods and every 20 to 30 minutes during othertimes of the day and at the weekend.

Clearly the potential future introduction of light rail into the precinct would have the ability to reduce the need to travel by carsignific antly.

## Cycle Parking

Cycling is becoming increasingly recognised for the contribution it can make asbeing a sustainable and healthy form of transport for trips within and a round our towns and cities.

There are two main elements to providing a quality cycle outcome:

- Provision of comidor infrastructure
- Provision of good quality parking facilities.

As described in Section 4.3 .2 of this report, it is intended to construct a cycleway along the waterfront which will run from north of the site to south of the SportsPrecinct. This will tie into existing cycle ways and those being planned by others.

Cycle parking needsto be allowed forearly in the development layout, asspace needed to accommodate cyclescan be significant. The importance of well thought out design cannot be overstated, as all too often space set aside for cycle parking is left half empty because it is either not possible to manoeuvre cyclesinto designated spaces, or the location is inconvenient. This in tum leads to cycles being left attached to railings or street fumiture nearer entrances. Consequently, cycle parking both for residents and visitors will be incorporated into the design.

## Evidence of Less Car Ownership

Whilst over the last 30 years there has been a long term trend towards higher rates of car ownership in the population, there is evidence that people aged under 35 are becoming less likely to hold a driver's licence.

Papers such as "Why are young people driving less? Trends in licence-holding and travel behaviour" presented at the Australasian Transport Research Forum in Canberra in 2010, exa mined licence-holding trends for young people in NSW and Sydney, explore possible reasons for these trends, and their polic $y$ and planning implic ations.

The report concluded that "transport modelling and transport planning needs to begin to adjust to this new paradigm of lower levels of lic ence-holding by young people. The increasing importance of public transport accessto jobs, services, and loc al shopping opportunities are clear, and are already reflected in the NSW State Plan prionity of improving public transport accessto key majorcentres in the metropolitan region. There is also an opportunity for cycling and walking to play a much larger role in the transport task for this age group".

The changesobserved in this paper should be viewed as a positive trend for road safety, for the environment, and for more liveable cities. These finding also acknowledge that the transport planner's toolkit is much larger than transport infrastructure and service provision. Educ ation policies, lic ensing polic ies and communic ations developments are all possible contributors to this signific a nt new trend,

## CarShare

Carshare is a concept by which members join a carownership club, choose a rate plan and pay an annual fee. The feescover fuel, insurance, maintenance, and cleaning. The vehicles are mostly sedans, but also include SUVs and station wagons. Each vehicle has a home location, referred to as a "pod", either in a parking lot or on a street, typically in a highly-populated urban neighbourhood. Members reserve a carby web ortelephone and use a key card to access the vehicle.

Similarly located councils (i.e. City of Canada Bay Council) have reported that "each share car replacesbetween 8 and 23 private carparking spaces, depending on the location of the

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development". Consequently, provision of carshare in the site should be able to reduce both the parking demand for the site and the traffic generated by it.

There are numerous examples in Sydney, and elsewhere in Australia, where one of the main operators, GoGet, has provided carshare cars to reduce the environmental impact of the development (http://www.goget.com.au/developer-partners/) some of which are listed below:

- Central Park Sydney, Chippendale NSW-2100 apartments, 2000 parking spaces, 44 GoGet on-site pods
- Trio Apartments, Camperdown NSW-397 a partments, 355 parking spaces, 10 GoGet on-site pods
- Belvedere Apartments, North Sydney NSW, 195 apartments, 140 parking spaces, 3 GoGet on-site pods.

Clearly, the subject site would be an ideal location for the introduction of similar car share spaces and it is not unrealistic to suggest that up to 100 cars could be provided.

## Green Travel Plan (GTP)

A GTP is a package of measuresaimed at promoting and encouraging sustainable travel and reduc ing reliance on the private car. It is not designed to be 'anti-car', but will make apparent, encourage and support people's aspirations for camying out their daily business in a more sustainable way. GTPscan provide both:

- measureswhich encourage reduced caruse (disincentives or ‘sticks')
- measureswhich enc ourage or support sustainable travel (also known as Active Transport), reduce the need to travel or make travelling more effic ient (incentives or 'carrots').

Active transport relates to physic al activity undertaken as a means of transport. It includestra vel by foot, bicycle and othernon-motorised vehicles. Use of public transport is also included in the definition asit often involves some walking or cycling to pick-up and from drop-off points.

Such travel plans have been implemented by GTA at sites such as Harold Park in Sydney. At that site, the following measures are provided:

- Compliance with the stringent parking controls applicable to the site.
- Creation of street networks a nd associated cycle ways, footpaths and links to encourage cycling and walking.
- Provision of a Transport Access Guide which would be given to every new occupant of dwellings.
- Provision of public transport noticeboardsto make residents a nd visitors more a ware of the altemative transport options a vailable to them. The format would be based upon the Transport Access Guide.
- Provision of yearly membership to a GoOccasional carshare which would have dedicated cars and dedicated parking spaces reasonably close to the proposed development.
- Provision of free weekly light rail and travel ten bus tic kets for the initial occupation of the dwellings so that residents will be encouraged to make public transport their modal choice from the day they occupy the property. The provision of Opal cards with prepa id credits is likely to be the preferred method of tic ket for future precincts when the Opal system is fully rolled out.
- All properties will be provided with high quality telecommunication points which will provide residents with the opportunity to work at home and to reduce the need to travel.
- Provision of bic ycle parking spaces both for residents and for visitors to the site.
- Provision of a half yearly newsletter to residents to promote local travel initiatives.

The sites are not yet fully occupied but the early signs of higher than average sustainable travel use is encouraging.

### 6.2 Summary

On the basis of all such measures being fully incorporated into the development, it is a nticipated that the subject site would generate signific antly less traffic than other residential sites in the vic inity. This will have the positive effect of reducing traffic impact.

## 7. Traffic Impact Assessment

### 7.1 Traffic Generation

## Existing Cumberland Hospital Precinct

The surveys at two access roads serving the Cumberland hospital precinct were undertaken as part of the intersection count surveys on Saturday $9^{\text {th }}$ of August and Thursday 14 ${ }^{\text {th }}$ August 2014.

Table 7.1 summarisesthe network peak hour traffic generation of the existing hospital precinct. The number of vehicles recorded to be using the hospital access roadsasa through-link to Westmead precinct (i.e. rat running through the Cumberland precinct) is not included in the traffic generation figures.

Table 7.1: Curent Traffic Generation of the Cumberland Hospital Precinct

|  | Inbound | Outbound | Total (2-way) |
| :--- | :---: | :---: | :---: |
| Thursday AM Peak Hour |  |  |  |
| -At Bridge Rd, west of site | 136 | 27 | 163 |
| -At Greenup Dr, west of Fleet St | 169 | 20 | 189 |
| - Thursday AM Traffic Generation | 305 | 47 | 352 |
| Thursday PM Peak Hour | 35 | 126 | 161 |
| -At Bridge Rd, west of site | 15 | 176 | 191 |
| -At Greenup Dr, west of Fleet St | 50 | 302 | 352 |
| - Thursday PM Traffic Generation |  |  |  |
| Saturday Midday Peak Hour | 9 | 8 | 17 |
| -At Bridge Rd, west of site | 8 | 15 | 23 |
| -At Greenup Dr, west of Fleet St | 17 | 23 | 40 |
| - Saturday Traffic Generation |  |  |  |

The table above shows that the Cumberland hospital precinct currently generates about 352 vehic les per hour during the weekday moming and aftemoon peak periods and about 40 vehic les per hour during the Saturday midday periods.

As previously discussed in Section 4 of this report, the curent Cumberland hospital precinct will be redeveloped to provide residential and commercial developments. Hence, the traffic generated by the existing hospital use will no longer be present in the future.

## Residential Development

RMS has recently released a Tec hnic al Direction (TDT2013/04) providing a summary of trip generation rates for various land uses to replace the suggested trip rates in their Guide to Traffic Generating Developments. The sites surveyed for high density residential are summarised in Table 7.2 below.
$\qquad$

Table 7.2: Revised RMS Traffic Generation Rates for High Density Residential Apartments

|  | No. of Units | Moming Peak Hour <br> (Thips per Unit per Hour) | Evening Peak Hour (Trips <br> per Unit per Hour) |
| :--- | :---: | :---: | :---: |
| Site 1 - St Leonards | 70 | 0.14 | 0.07 |
| Site 2 - Chatswood | 129 | 0.14 | 0.12 |
| Site 3 - Cronulla | 28 | 0.07 | 0.11 |
| Site 4 - Rockdale | 234 | 0.32 | 0.18 |
| Site 5 - Pa ramatta | 83 | 0.27 | 0.12 |
| Site 6 - Lberty Grove | 64 | 0.28 | 0.41 |
| Site 7 - Strathfield | 31 | 0.1 | 0.06 |
| Site 10 - Pymmont | 131 | 0.18 | 0.1 |
| Average | 96 | 0.19 | 0.15 |

The a verage updated traffic generation rates in the Technical Direction for high density resid ential developments are 0.19 and 0.15 trips per peak hour per unit during the moming and evening peak periods respectively (asopposed to their previousguidance which suggested 0.29 trips per peak hour per unit). These rates are generally on the basis of excellent public transport/active transport facilities. However rather than using the average rate, it is generally more accepted to delve into the detail to provide a more reasonable/accurate rate and hence, the first point of reference would be the surveys that were undertaken in Pa rramatta.

It is also noted that the Parramatta survey in Table 7.2 reported a higher than average moming peak hour traffic generation of 0.27 tripsperhour per unit. The Parramatta site surveyed wasat Ha ssall Street which is only 300 m from the railway station. Consequently, the J oumey to Work data conta ined in Bureau of Transport Statistic s (BTS) have been reviewed to understand the transport characteristic s of the Hassall Street precinct and the residential precinct close to the Cumberland precinct to establish the difference in the travel pattems between two locations. Table 7.3 indic ates the following travel modes were recorded for residents at Parramatta and North Parramatta.

Table 7.3: Joumey to Work, Travel Mode (for residents)

| Travel Mode | Panramatta (Hassall St, <br> near Railway Station) | North Panamatta <br> (near the subject site) |
| :---: | :---: | :---: |
| Train | $45 \%$ | $21 \%$ |
| Car (driver) | $29 \%$ | $50 \%$ |
| Walked | $10 \%$ | $10 \%$ |
| Bus | $10 \%$ | $10 \%$ |
| Car(passenger) | $5 \%$ | $5 \%$ |
| Others | $1 \%$ | $4 \%$ |
| Total | $100 \%$ | $100 \%$ |

The table above indicates that the Parramatta site nearthe railway station hasthe percentage of people who travel to work by car (as driver) is $29 \%$. North Parramatta, however, indic ates that the percentage of people who travel to work by car (as driver) is about $50 \%$, which is signific antly higher than North Parramatta sites, a lbeit not nec essarily all in the peak hour.

As described in Section 6, it is the intention to introduce a "sea change" in the provision of facilities for non-car based modes of transport.

It is considered that in the longerterm assignific ant public transport options are improved such as possible light rail and/oroperation of shuttle busto and from the Parramatta station, the trip

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generation of high density residential dwellings in the subject site would reduce below that of a traditional Pa rramatta unit block.

For assessment purposes, the trip generation rate of 0.23 trips perhour per unit has been adopted, on the basis that the site will undergo a signific ant improvement to public transport active travel measure in the future. Section 6 discusses the possible measures which can be implemented to reduce the cardependency and encourage use of sustainable transport modes.

Using the trip generation rate of 0.23 trips per hour per unit for the expected residential development scenario of some 5,870 apartments (including the Cumberland and Sport/Leisure precincts), about 1,350 peak hour vehiculartrips would be generated forthe residential component. For assessment purposes, the same trip generation rate is adopted for all three peak periods (i.e. Thursday AM, Thursday PM and Saturday midday peak hour).

## Commercial/Retail Development

RMS' Technic al Direction (TDT2013/04) also provides updated traffic generation rates for the office blocks. The sites surveyed for office blocks are summarised in Table 7.4 below.

Table 7.4: Revised RMS Traffic Generation Rates for Office Blocks

|  | Gross Foor Area (m²) | Moming Peak Hour <br> (Thips per 100m² GFA) | Evening Peak Hour <br> (Thips per 100m² GFA) |
| :--- | :---: | :---: | :---: |
| Site 1 - North Sydney | 31,400 | 0.17 | 0.14 |
| Site 2 - Chatswood | 10,214 | 1.03 | 0.84 |
| Site 3 - Sydney Olympic Park | 34,131 | 1.48 | 1.41 |
| Site 4 - Hurstville | 3,254 | 2.86 | 1.84 |
| Site 5 - Macquarie Park | 5,748 | 2.07 | 1.84 |
| Site 6 - Pa ramatta | 27,000 | 0.69 | 0.61 |
| Site 7 - Livemool | 2,817 | 2.49 | 1.70 |
| Site 8 - Norwest | 1,200 | 2.75 | 1.17 |
| Site 9 - Newcastle | 12,182 | 1.03 | 1.14 |
| Site 10 - Wollongong | 12,921 | 0.95 | 0.77 |
| Average |  | 1.55 | 1.15 |

The following revised average rates are provided for the AM and PM peak hours:

- AM peak hour vehic le trips $=1.6$ per $100 \mathrm{~m}^{2}$ gross floor area
- PM peak hour vehicle trips $=1.2$ per $100 \mathrm{~m}^{2}$ gross floor a rea.

Of the ten sitesthat were surveyed by RMS, eight of the sites were loc ated where a good level of public transport is provided. The remaining two sites were located at Sydney Olympic Park and Norwest.

In anticipation that the site will provide a good level of public transport as well as active travel opportunities, a trip generation rate of 1.25 per $100 \mathrm{~m}^{2}$ gross floor a rea has been a ssumed for both AM and PM peak hour vehicle trips.

Whilst the proposed development consists of a bout $4,000 \mathrm{~m}^{2}$ of retail area in total, it is antic ipated that small areas of retail space would be provided throughout the precinct. The nature of the proposed retail use on site would generally consist of a local minimart for supply of general provision to service the surrounding residential area. It is not envisaged that the retail development would attract traffic into the area asit would serve only local residents or employee. The majority of customers would be from walk in pedestrians from the surrounding
developments. Hence, it is expected that little orno additional vehicular traffic will be generated by the retail component. However, conservatively the proposed retail area has been included in the commercial area (i.e. $62,500 \mathrm{~m}^{2}$ of commercial a rea has been assumed).

Using the trip generation rate of 1.25 per $100 \mathrm{~m}^{2}$ gross floor area, the proposed commercial/retail area of some $62,500 \mathrm{~m}^{2}$ will generate about 780 vehiculartraffic per weekday AM and PM peak hour for the commercial development component. It is expected that the proposed commercial/retail development will not generate any vehicular trips during the Saturday midday periods.

## Total Development Traffic Generation

Table 7.5 presents the total traffic generation of the proposed development then subtrac ts the current hospital generated traffic and the number of vehicles passing through the site without having a destination within the site.

Table 7.5: Resultant Traffic Generation by the Proposal

|  | Peak Hour Traffic (vehic les per hour) |  |  |
| :---: | :---: | :---: | :---: |
| Development Generated Traffic |  | Thursday AM | Thursday PM |
| Residential | +1350 |  |  |
| Commercial/Retail | +780 | +1350 | +1350 |
| Total (additional) | +2130 | +2130 | +1350 |
| Existing Cumberland Precinct Traffic |  |  |  |
| Hospital generated traffic ${ }^{\#}$ | -60 | -115 | -30 |
| No. of vehic les rat-running through the site | -190 | -190 | -20 |
| Total (loss of existing traffic) | $-\mathbf{2 5 0}$ | -305 | -50 |
| Resultant Increase | $\mathbf{+ 1 8 8 0}$ | $\mathbf{+ 1 8 2 5}$ | $\mathbf{+ 1 3 0 0}$ |

NOTE: Total hospital generated traffic is 352 vph (AM); 352 vph (PM); 40 (Sat). However the figures presented in the table above only represents vehic les using the main hospital accessi.e. does not include traffic using the Bridge Road access asthese vehicles do not use the surveyed intersections along O'Connell Street/Church Street. Hence the removal of vehicularaccess at Bridge Road will not have an impact on road network east of the Cumberland precinct.

The resultant increase in traffic due to the proposal would be in the order of 1,800 to 1,900 vehic les per hour during the weekday AM, PM and Saturday midday peak periods. These figures have been adopted for the post development traffic modelling purposes.

### 7.2 Trip Distribution

The directional distribution for residential traffic was assumed to be 20 percent inbound and 80 percent outbound during the moming peak period. Simila rly for traffic arising from the commercial use, 70 percent of the development traffic wasassumed to be inbound while the remaining 30 percent would be outbound. These inbound/outbound percentagesare reversed in the aftemoon peak period.

The development traffic was distributed on the local road network based on 2011 joumey to work data as follows:

- residential trips-joumey to work data based on the North Pa ramatta residential area
- commercial trips - joumey to work data on the Parramatta/North Parramatta employment area including the current Cumberland hospital precinct.

The distribution factors are presented in Table 7.6.

Table 7.6: Development Traffic Distribution Percentages

| To/ From Directions | Residential | Commercial |
| :--- | :---: | :---: |
| Windsor Rd-North | $10 \%$ | $18 \%$ |
| Cumberland Highway-West | $10 \%$ | $17 \%$ |
| Pennant Hills Rd-East | $28 \%$ | $10 \%$ |
| Victoria Rd-East | $25 \%$ | $20 \%$ |
| O'Connell St-South | $27 \%$ | $35 \%$ |
| Total | $100 \%$ | $100 \%$ |

Using the above traffic distribution percentages and the resultant increase in traffic generated by the proposal presented in Table 7.5, the development generated traffic using the key extemal road network is calculated and presented in Table 7.7.

Table 7.7: Additional Development Traffic on the Existing Road Network

| To/ From Directions | Thursday AM | Thursday PM | Saturday Midday |
| :--- | :---: | :---: | :---: |
| Windsor Rd-North | 230 | 220 | 125 |
| Cumberla nd Highway-West | 230 | 220 | 125 |
| Pennant Hills Rd-East | 420 | 420 | 370 |
| Victoria Rd-East | 440 | 430 | 330 |
| O'Connell St-South | 560 | 535 | 350 |
| Total | 1880 | 1825 | 1300 |

### 7.3 Background Growth

There are a large number of development sites proposed in Parramatta, many in the CBD. This includes:

- Parramatta Square - It will include public space, corporate facilities, residential apartments, retail and dining, new Council headquarters, and connections to the transport interchange. It is expected to house up to 13,000 jobs upon completion.
- Lennox Bridge CarPark site - A range of uses will be accommodated including cafés/ bars/ restaurants and Councils new Discovery Centre.
- Macquarie Street CarPark site - The site will be redeveloped into a new residential towerand a new commercial towerabove a multi deck public car.
- Eclipse Tower, 60 Station Street - This will be a $26,000 \mathrm{~m}^{2}, 20$ storey commerc ial building close to the transport interchange.
- 100 George Street - This is a ground floor public domain and retail space and 9,600 m2 of commercial office space in the heart of the Parramatta CBD.
- 89 George Street - A proposed 14 storey boutique commercial building, DA-approved and will be approximately $10,000 \mathrm{~m}^{2}$ when built.
- 105 Phillip Street - A proposed 13 storey commerc ial building will deliver 20,500 m².
- 111 George Street - This is a 17 Storey mixed use development.
- Westfield Tower - A proposed 20 storey $35,000 \mathrm{~m}^{2}$ commercial tower designed to sit atop the current Parramatta Westfield Shopping Centre.
- Cumberland Newspaper/ NewsLtd Site - Stage 1 will be a five storey commercial building aspart of a broader precinct development plan.
- UWS - Westmead Precinct - The vision for this future mixed use development is to establish a diverse range of activities to enhance and support Westmead' s role as a specialist medical research hub. It is expected to create new links to the adjoining

Westmead Hospital, schools and key public transport nodes including Westmead Station and the new Transit Way.

- Westmead Millennium Institute - This 7 storey purpose-built facility.
- Major Residential Developments - The Parramatta CBD hasseen a surge of inner-city residential development in recent years. Below is a sample of the key sites:
- Altitude - Meriton Development - This proposed mixed use development on the former Da vid J ones site, , includes an "East" tower at 30-storeys ( 242 serviced a partments), a "West" tower at 53-storeys (354 residential a partments) and a 3storey podium including eight commercial tenancies.
- V By Crown, 45 Macquarie Street - This is a 20+storey development with signific ant ground floor retail and 5,800 m2 commercial space and 336 luxury apartments loc ated above street level.
- B1 Tower, 118 Church Street - This will be a 28 level mixed-use building featuring 80 a partments and 5 floors of commercial \& retail.
- Focus, 6-10 Charles Street - This is a 12 storey, 100 residential unit twin tower design situated between Parramatta \& Hamis Park railway stations.

Traffic will be generated by these developments and there is likely to be an increase in background traffic growth. However, many of these developments are proposed to be located in sustainable locations which mean that car use will be minimal.

Along the Church Road comidor, the main development proposal is the Cumberland/Sports precinct proposal and it will be the subject site that will create most of the localised traffic growth. It is also noted than many of the roadsclose to the subject site are close to capacity.

Consequently, ratherthan resulting in high peak hour traffic growth increases, there is likely to be peak spreading when the length of the peak time period extends. As a result, the background traffic growth, created by other developments is likely to be low, and a figure of 5\% over the development of the site hasbeen assumed. Such traffic growth has been applied to Church Street, Pennant Hills Road and Vic toria Road. These key extemal roads are shown in Figure 7.1.

Notwithstanding the above, RMS has recently released a tenderforthe strategic modelling study of Parramatta area. The results of this study would provide additional guidance on the future growth in Parramatta and its vicinity, which could be incorporated in the laterstage once the modelling results are made a vailable.

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Figure 7.1: Background Traffic Growth


The resultant increase in development generated traffic has been assigned to surrounding road network using the trip distribution percentages presented above and superimposed on to the existing intersection tuming movements (refer to Appendix A.1). The post development intersection tuming movements including the background traffic growth is presented in Appendix C.

### 7.4 Mid-Block Capacity

The forecast future peak hour mid-block traffic flows are shown in Table 7.8.
The figurespresented in Table 7.8 represent the post development flows with the background growth added to the key extemal roads as disc ussed above.

Traffic Impact Assessment

Table 7.8: Future Mid-Block Traffic Fows

| Location | Thursday AM |  |  | Thursday PM |  |  | Saturday Midday |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{gathered} \text { NB/ } \\ \text { EB } \end{gathered}$ | $\begin{aligned} & \text { SB/ } \\ & \text { WB } \end{aligned}$ | Twoway | $\begin{gathered} \text { NB/ } \\ \text { EB } \end{gathered}$ | $\begin{aligned} & \text { SB/ } \\ & \text { WB } \end{aligned}$ | Twoway | $\begin{gathered} \text { NB/ } \\ \text { EB } \end{gathered}$ | $\begin{aligned} & \text { SB/ } \\ & \text { WB } \end{aligned}$ | Twoway |
| Church St, south of Victoria Rd | 228 | 319 | 547 | 304 | 251 | 555 | 236 | 347 | 583 |
| Church St, north of Vic toria Rd | 603 | 1240 | 1843 | 1113 | 784 | 1897 | 704 | 850 | 1554 |
| Church St, south of Penna nt Hills Rd | 729 | 1596 | 2325 | 1491 | 887 | 2378 | 992 | 1057 | 2049 |
| Church St, south of Factory St | 547 | 1320 | 1867 | 1105 | 733 | 1838 | 774 | 915 | 1689 |
| Church St, south of Bamey St | 656 | 1048 | 1704 | 1136 | 716 | 1852 | 840 | 742 | 1582 |
| Church St, south of Board St | 917 | 2046 | 2963 | 1909 | 1330 | 3239 | 1178 | 1436 | 2614 |
| Church St, south of North Rocks Rd | 1199 | 2128 | 3327 | 2355 | 1328 | 3683 | 1534 | 1443 | 2977 |
| Church St, south of J a mes Ruse Dr | 1149 | 2105 | 3254 | 2379 | 1495 | 3874 | 1737 | 1726 | 3463 |
| Church St, north of J a mes Ruse Dr | 1001 | 3111 | 4112 | 2467 | 1802 | 4269 | 1836 | 2052 | 3888 |
| O'Connell St, south of George St | 2168 | 1552 | 3720 | 1660 | 1448 | 3108 | 1286 | 1284 | 2570 |
| O 'Connell St, south of Vic toria Rd | 1525 | 1812 | 3337 | 1535 | 1548 | 3083 | 1057 | 1343 | 2400 |
| O 'Connell St, south of Grose St | 1078 | 1558 | 2636 | 1400 | 1010 | 2410 | 887 | 1065 | 1952 |
| O 'Connell St, south of Albert St | 608 | 1283 | 1891 | 1141 | 677 | 1818 | 643 | 838 | 1481 |
| O 'Connell St, south of Bamey St | 433 | 1146 | 1579 | 959 | 483 | 1442 | 538 | 701 | 1239 |
| O'Connell St, south of Board St | 364 | 15 | 379 | 400 | 23 | 423 | 362 | 23 | 385 |
| Fleet St, south of Albert St | 332 | 100 | 432 | 158 | 334 | 492 | 177 | 147 | 324 |
| Fleet St, south of Factory St | 97 | 108 | 205 | 122 | 99 | 221 | 76 | 68 | 144 |
| Ma rist St, south of Market St | 505 | 596 | 1101 | 777 | 590 | 1367 | 648 | 603 | 1251 |
| Marist St, south of Vic toria Rd | 329 | 303 | 632 | 553 | 355 | 908 | 482 | 297 | 779 |
| Wilde Ave, south of Victoria Rd | 383 | 1132 | 1515 | 878 | 537 | 1415 | 364 | 379 | 743 |
| Market St, east of Ma rist St | 200 | 332 | 532 | 272 | 267 | 539 | 216 | 345 | 561 |
| Victoria Rd, east of O'Connell St | 736 | 722 | 1458 | 706 | 831 | 1537 | 457 | 527 | 984 |
| Vic toria Rd, east of Marist St | 799 | 793 | 1592 | 868 | 930 | 1798 | 684 | 669 | 1353 |
| Victoria Rd, east of Church St | 1579 | 1034 | 2613 | 1149 | 1520 | 2669 | 1022 | 998 | 2020 |
| Grose St, east of O 'C onnell St | 305 | 248 | 553 | 231 | 257 | 488 | 132 | 157 | 289 |
| Grose St, west of Church St | 285 | 519 | 804 | 340 | 343 | 683 | 200 | 290 | 490 |
| Fennell St, west of O'C onnell St | 242 | 296 | 538 | 233 | 285 | 518 | 204 | 208 | 412 |
| Fennell St, east of O 'C onnell St | 74 | 20 | 94 | 68 | 40 | 108 | 34 | 21 | 55 |
| Albert St, west of Fleet St | 134 | 233 | 367 | 261 | 162 | 423 | 86 | 84 | 170 |
| Albert St, east of O 'Connell St | 353 | 368 | 721 | 332 | 545 | 877 | 259 | 404 | 663 |
| Pennant Hills Rd, east of Church St | 736 | 899 | 1635 | 768 | 825 | 1593 | 639 | 699 | 1338 |
| Factory St, east of Fleet St | 178 | 59 | 237 | 80 | 152 | 232 | 108 | 98 | 206 |
| Fac tory St, east of O'C onnell St | 391 | 67 | 458 | 125 | 123 | 248 | 244 | 99 | 343 |
| Dunlop St, west of O 'Connell St | 112 | 137 | 249 | 132 | 99 | 231 | 79 | 57 | 136 |
| Dunlop St, east of O'Connell St | 58 | 33 | 91 | 38 | 36 | 74 | 49 | 23 | 72 |
| Ba mey St, east of O 'Connell St | 137 | 904 | 1041 | 410 | 654 | 1064 | 148 | 667 | 815 |
| Board St, east of O'Connell St | 352 | 16 | 368 | 401 | 18 | 419 | 365 | 23 | 388 |
| North Rocks Rd, east of Church St | 663 | 830 | 1493 | 713 | 680 | 1393 | 665 | 700 | 1365 |

NOTE: NB - Northbound; EB - Eastbound; SB - Southbound; WB - Westbound

The comparison with the existing and future mid-block flows indic atesthat the following additional traffic would be using the key roads in the vic inity of the site during the Thursday AM/PM peak hours:

- Church Street: 310 to 590 additional vehic les per hour (vph)
- O'Connell Street: 280 to 600 additional vph
- Victoria Road: 380 to 500 additional vph
- Pennant Hills Road: about 460 additional vph
- Factory Street: 200 to 410 additional vph
- Albert Street: 120 to 330 additional vph
- Bamey Street: 100 to 270 additional vph
- Fennell Street: 240 to 270 additional vph
- Fleet Street: 100 to 250 additional vph
- Dunlop Street: 130 to 150 additional vph
- Grose Street: about 90 additional vph
- Board Street: 20 to 80 additional vph.

The greatest inc rease in volumes would occur on O'Connell Street, Church Street, Victoria Road and Pennant Hills Road. The local streets in the vicinity of the site with an increase of more than 200 vph would be Factory Street, Albert Street, Bamey Street, Fennell Street and Fleet Street.

Table 7.9 presents the maximum hourly flow in the peak direction (i.e. one-way peak hour flow). These figures are compared against the theoretical lane capacity for urban roadscontained in RMS guidelines without consideration to the type of roads.

Austroads Guide to Traffic Management Part 3 states that the peak period mid-block traffic capacities are between 1200 to 1400 vph. Recent Studies by GTA have utilised a similar figure of 1,350 (or 1,320 ) vph for a sub-arterial type road. On O'C onnell Street, north of Vic toria Road, a nominal capacity of $1,200 \mathrm{vph}$ has been adopted. In addition, the Austroads Guide to Traffic Engineering Practice also stated the nominal capacity of a traffic lane on an undivided road is 900 vph . Hence, this is adopted for all other local roads.

The comparison results are presented in Table 7.9.

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Table 7.9: Peak Direction Post Development Maximum Hourly How and Theoretic al Capacity Comparisons

| Location | Capacity per <br> Lane | Lanes in Peak <br> Direction | Max. Hourly <br> Rows | Demand/ <br> Capacity Ratio |
| :--- | :---: | :---: | :---: | :---: |
| Church St, north of Vic toria Rd | 1350 | 2 | 1240 | 0.5 |
| Church St, south of Pennant Hills Rd | 1350 | 1 | 1596 | 1.2 |
| Church St, south of North Rocks Rd | 1350 | 2 | 2128 | 0.8 |
| O'Connell St, south of George St | 1350 | 2 | 2168 | 0.8 |
| O'Connell St, south of Grose St | 1200 | 2 | 1558 | 0.6 |
| O'Connell St, south of Albert St | 1200 | 2 | 1283 | 0.5 |
| O'Connell St, south of Bamey St | 1200 | 1 | 1146 | 1.0 |
| Victoria Rd, east of Church St | 1350 | 2 | 1579 | 0.6 |
| Pennant Hills Rd, east of Church St | 1350 | 2 | 899 | 0.3 |
| Factory St, east of O'Connell St | 900 | 1 | 391 | 0.4 |
| Albert St, east of O'Connell St | 900 | 1 | 545 | 0.6 |
| Bamey St, east of O'Connell St | 900 | 1 | 904 | 1.0 |
| Fennell St, west of O'Connell St | 900 | 1 | 296 | 0.3 |
| Fleet St, south of Albert St | 900 | 1 | 334 | 0.4 |

The comparison of post development flows and the theoretic al capacity presented in Table 7.9 indic ates that Church Street, south of Pennant Hills Road which is the section along the Church Street with only one travelling lane in each direction (i.e. excluding the buslane) would exceed its theoretic al capacity. O'Connell Street, south of Bamey Street and Bamey Street, east of O'Connell Street would also reach the theoretic al capacities under the current configurations.

### 7.5 Intersection Operation

The a nalysis results for future conditions (including development traffic and background growth) are presented in Table 7.10. It is noted that the intersections have been optimised in Linsig model in terms of signal timing for the future operations.

Linsig modelling process undertaken and detailed outputs are also included in Appendix D.

Table 7.10: Future Intersection Operating Conditions

|  | Intersections | Control Type | Thursday AM |  | Thursday PM |  | Saturday Midday |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Level of Service | Average Delay (sec) | Level of Service | Average Delay (sec) | Level of Service | Average Delay (sec) |
| (LinSig) | Windsor Rd/ Cumberland Hwy | Signal | F | 109 | F | 150 | F | 106 |
| (LinSig) | Church St/ The J unction Access | Signal | A | 10 | A | 13 | A | 15 |
| (LinSig) | Church St/ North RocksRd | Signal | D | 47 | B | 26 | C | 29 |
| (SIDRA) | Church St/ Board St/ Seville St | Priority | E | 69 | F | 199 | B | 23 |
| (LinSig) | Church St/ Bamey St | Signal | C | 39 | F | 109 | C | 40 |
| (LinSig) | Church St/ Factory St | Signal | F | 139 | B | 21 | B | 25 |
| (LinSig) | Church St/ Albert St/ Pennant Hills Rd | Signal | F | 148 | F | 173 | C | 35 |
| (LinSig) | Church St/ Grose St | Signal | D | 54 | E | 71 | B | 28 |
| (SIDRA) | Church St/ Market St | Priority | A | 9 | A | 12 | A | 9 |
| (SIDRA) | O'Connell St/ Board St | Priority | A | 9 | A | 9 | A | 9 |
| (SIDRA) | O 'Connell St/ Bamey St | Priority | F | 122 | E | 66 | F | 173 |
| (SIDRA) | O 'Connell St/ Dunlop St | Priority | F | 82 | C | 30 | B | 21 |
| (SIDRA) | O 'Connell St/ Factory St | Priority | F | $>5$ minutes | C | 36 | F | $>5$ minutes |
| (SIDRA) | O'Connell St/ Fennell St | Priority | F | $>5$ minutes | F | $>5$ minutes | F | $>5$ minutes |
| (SIDRA) | O 'Connell St/George St | Signal | B | 26 | A | 10 | A | 13 |
| (LinSig) | O'Connell St/ Albert St | Signal | B | 28 | B | 24 | B | 22 |
| (LinSig) | O'Connell St/ Grose St | Signal | B | 19 | B | 21 | A | 13 |
| (LinSig) | O 'Connell St/ Victoria Rd | Signal | F | 72 | C | 32 | B | 26 |
| (LinSig) | Church St/ Victoria Rd | Signal | D | 47 | E | 69 | C | 33 |
| (LinSig) | Victoria Rd/ Marsden St | Signal | C | 34 | C | 41 | C | 34 |
| (LinSig) | Vic toria Rd/ Wilde Ave | Signal | C | 32 | C | 37 | B | 27 |
| (SIDRA) | Factory St/ New St | Priority | A | 9 | A | 9 | A | 9 |
| (SIDRA) | Greenup Dr/ Fleet St | Priority | A | 11 | A | 10 | A | 9 |
| (SIDRA) | Marsden St/ Market St | Priority | A | 10 | A | 11 | A | 10 |


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The results indicate that Windsor Road/Cumberland Highway intersection would continue to operate with level of service (LOS) F for all three peak periods. As desc ribed previously, the Windsor Road bridge overthe Cumberland Highway is proposed to be widened as part of the proposed Westem Sydney Regional Ring Road to address this existing issue.

The intersections with LOSE/F under the future conditions have been tested further with additional ca pacities. This is disc ussed in detail in the following section.

### 7.6 Possible Intersection/Road Improvements

Historic discussions with RMS and Pa rramatta City Council have suggested that a number of intersection upgrades are being considered in the vicinity of the site. These include:

- Removal of the median strip at the Factory Street intersection to allow cross traffic across $O^{\prime}$ Connell Street and replacement of existing intersection a rrangement with a roundabout.
- The intersection at O'C onnell Street with Fennell Street is being considered for an upgrade to either traffic signals (Council's preferred choice) or a roundabout (RMS' preferred choice) to address road safety concems
- The Windsor Road bridge overthe Cumberla nd Highway is proposed to be widened as part of the proposed Westem Sydney Regional Ring Road.

In general, the above upgrades are adopted as being included in the intersection improvement options.

The intersections that would require additional capacity under the future conditions are listed below along with the upgrade options:

- Church Street/Board Street/Seville Street is currently a priority controlled intersection. The partially signalised option has been tested with only Board Street approach being signalised.
- Church Street/Ba mey Street is c urrently a signalised intersection. Church Street southbound wastested with additional right tum bay (i.e. dual right tum lanes).
- For the intersections on Church Street between Factory Street and Grose Street, an additional through lane option has been tested for southbound traffic in the AM peak. For the PM peak, an additional northbound through lane option has been tested for the intersections on Church Street between east of Ba mey Street and Grose Street.
- O'Connell Street intersections at Ba mey Street, Dunlop Street, Fa c tory Street and Fennell Street are all currently priority controlled intersections. These intersections have been tested as one lane roundabouts.
- O'Connell Street intersections at Bamey Street and Factory Street have been further tested as signa lised intersections
- O'Connell Street/Victoria Road intersection hasbeen tested with revised lane configuration on the O'Connell Street south approach (i.e. one shared left and through lane, one shared through and right tum lane and one designated right tum lane).

The results of the possible intersection improvements described above are presented in Table 7.11.

Table 7.11: Future Intersection Operating Conditions with Upgrades

|  | Intersections | Upgraded Control Type | Thursday AM |  | Thursday PM |  | Saturday Midday |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Level of Senvice | Average Delay (sec) | Level of Senvice | Average Delay (sec) | Level of Service | Average Delay (sec) |
| (LinSig) | Church St/ Board St/ Seville St | Partially Signalised - Church St/ Board St | A | 9 | B | 17 | A | 9 |
| (LinSig) | Church St/ Bamey St | Additional right tum bay (min. of 50m) on Church St southbound | C | 35 | D | 49 | C | 39 |
| (LinSig) | Church St/ Factory St | Additional through lane on peak direction on Church St (between Factory St and Grose St) <br> - AM Peak, additional through lane for Southbound <br> - PM Peak, additional through lane for Northbound | C | 35 | B | 21 | B | 25 |
| (LinSig) | Church St/ Pennant Hills Rd |  | C | 32 | C | 37 | C | 35 |
| (LinSig) | Church St/ Grose St |  | B | 26 | B | 26 | B | 28 |
| (SIDRA) | O 'Connell St/ Bamey St | Upgrade to Roundabout | F | 92 | D | 48 | B | 16 |
|  |  | Upgrade to Signalised intersection | B | 23 | B | 24 | B | 18 |
| (SIDRA) | O'Connell St/ Dunlop St | Upgrade to Roundabout | C | 31 | B | 23 | B | 17 |
| (SIDRA) | O'Connell St/ Factory St | Upgrade to Roundabout | D | 54 | B | 23 | B | 16 |
|  |  | Upgrade to Signalised intersection | B | 17 | B | 18 | A | 11 |
| (SIDRA) | O 'Connell St/ Fennell St | Upgrade to Roundabout | B | 19 | B | 17 | B | 16 |
| (LinSig) | O 'Connell St/ Victoria Rd | Lane Reconfiguration on O'Connell St south approach | D | 44 | C | 34 | B | 26 |


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## 8. Proposed Infrastruc ture Improvements

### 8.1 Road Improvements

As discussed in Section 7.6, the following intersection upgrades would be required to ac commodate the additional traffic generated by the proposed development and the future background growth on key extemal roads:

- Church Street/Board Street - Upgrade to a signal (partially - west side of Church Street only).
- Church Street/Ba mey Street - Additional right tum bay (i.e. dual right tum lanes) from Church Street southbound.
- For the intersections on Church Street between Factory Street and Grose Street, an additional through lane would be required for southbound traffic in the AM peak. For the PM peak, an additional northbound through lane would be required for the intersections on Church Street between east of Ba mey Street a nd Grose Street.
- Thisproposal in particular would require detailed consideration aswhilst only one lane is required for southbound traffic in the AM peak and northbound traffic in the PM peak, as simple tidal flow system might not be appropriate as with such an a rrangement it may be diffic ult to accommodate right tuming traffic.
- O'Connell Street/Bamey Street - Upgrade to a signal
- O'Connell Street/Dunlop Street - Upgrade to a one-lane roundabout
- O'Connell Street/Factory Street - Upgrade to a signal
- O’Connell Street/Fennell Street - Upgrade to a one-lane roundabout
- O'Connell Street/Victoria Road - Revise lane configuration.

In addition to the above, the Windsor Road bridge over the Cumberland Highway is proposed to be widened aspart of the proposed Westem Sydney Regional Ring Road.

Figure 8.1 presents the indic ative intersection configuration of the existing and proposed upgrades desc ribed above.

Figure 8.1: Existing and Proposed Intersection Configurations


Proposed Layout


Proposed Infrastruc ture Improvements

Existing Layout



### 8.2 Public Transport Improvements

As described earlier in the report, it is the intention to provide a good quality shuttle bus between the subject site and the Parramatta interchange. The possible shuttle bus route is shown in Figure 4.6. The proposed shuttle bus would provide services at every 10 minutes in the weekday peak periods and every 20 to 30 minutes during other times of the day and at the weekend.

In addition to the shuttle bus service, the potential future introduction of light rail into the precinct would have the ability to signific antly reduce the travel by car mode.

The recent introduction of Opal cardsmeans that transfers/ changing modesat such interchanges is much easier a nd without the historic cost implic ations.

### 8.3 Pedestria $n$ \& Cyc lewa y Improvements

The provision of a new cycleway along the waterfront which will run from north of the site to south of the Sports Precinct would enhance the pedestrian and cycleway network signific antly. It will also tie into existing and proposed cycle ways in the vicinity of the site. Figure 4.5 presented the proposed pedestrian and cycleway for the proposed development as well asthe existing and proposed cycleway.

The assessment of bic ycle parking provision (referto Section 5.2) indicated that the proposed development would require about 3,095 spaces.

As per the Council's DCP, trip end facilities including showers and lockers would need to be provided forcommercial and retail developments.

## 9. Conclusion

Based on the analysis and disc ussions presented within this report, the following conclusions are made:

- The Parramatta North Urban Renewal (PNUR) in its end state proposesto provide about 5,600 residential dwellings, $35,000 \mathrm{~m}^{2}$ of a daptive reuse of historic build ings and $4,000 \mathrm{~m}^{2}$ of retail use in the Cumberland Precinct. It is also proposed to include $46,000 \mathrm{~m}^{2}$ of mixed use developments in the Sports and Leisure Precinct (which would be predomina ntly commercial use).
- The staging of the works is overa 15 to 20 year period.
- The Parramatta Gaol and SES land do not form part of this rezoning proposal.
- Using the current DCP parking rates, the proposal would need to provide 8,820 to 9,770 car parking spaces. In addition, the required bicycle parking provision for the overall development would be about 3,250 spaces.
- A number of measures will be incorporated into the proposal to minimise the cartravel. The potential measures are:
- Limited parking ratios-One of the most effective ways to reduce traffic congestion and pollution, and encourage a shift to sustainable transportation modes, is through parking reform.
- Bus improvements - It is the intention to provide a good quality shuttle bus between the subject site and the Parramatta interchange.
- Cycle parking /facilities-It is intended to construct a cycleway along the waterfront which will run from north of the site to south of the Sports Precinct. This will tie into existing cycle ways and those being planned by others.
- Carsharing/carclub cars - The subject site would be ideal location for the introduction of carshare spaces and it is not unrealistic to suggest that up to 100 carscould be provided within the site.
- Green travel plan (GTP) - A GTP is a package of measuresaimed at promoting and encouraging sustainable travel and reducing reliance on the private car. GTPscan provide measures which encourage reduced caruse and support sustainable travel.
- On the basis of all such measures being fully incorporated into the development, it is antic ipated that the subject site would generate signific antly less traffic than other residential sites in the vicinity. This will ha ve the positive effect of reducing traffic impact.
- The resultant increase in traffic generated by the proposal is 1,880 vehic les per hour (vph) for Thursday AM, 1,825 vph for Thursd ay PM and 1,300 vph for Saturday midday.
- The comparison of post development flows a nd the theoretical capacity indic ates that Church Street, south of Pennant Hills Road which is the section along the Church Street with only one travelling lane in each direction (i.e. excluding the bus lane) would exceed its theoreticalcapacity. O'C onnell Street, south of Bamey Street and Bamey Street, east of O'Connell Street would also reach the theoretic al capacities under the current configurations
- The following intersection upgrades would be required to accommodate the additional traffic generated by the proposed development and the future background growth on key extemal roads:
- Church Street/Board Street - Upgrade to a partial signal (west side of Church Street only)
- Church Street/Bamey Street - Additional right tum bay (i.e. dual right tum lanes) from Church Street southbound
- For the intersections on Church Street between Factory Street and Grose Street, an additional through lane would be required for southbound traffic in the AM peak. Forthe PM peak, an additional northbound through lane would be required for the intersections on Church Street between east of Ba mey Street and Grose Street
- O'Connell Street intersections at Bamey Street \& Factory Street - Upgrade to a signal
- O'Connell Street intersec tions at Dunlop Street \& Fennell Street - Upgrade to a one-lane roundabout
- O'Connell Street/Victoria Road signalised intersection - Revise lane configuration.
- In addition to the above, the WindsorRoad bridge overthe Cumberland Highway is proposed to be widened aspart of the proposed Westem Sydney Regional Ring Road and this imp rovement will be necessary to address existing/ future traffic problems.
- The provision of a new cycleway along the waterfront which will run from north of the site to south of the Sports Precinct would enhance the pedestrian and cycleway network significantly. It will also tie into existing and proposed cycle ways in the vicinity of the site.
- It is proposed to provide a good quality shuttle bus service between the subject site and the Parramatta interchange. The proposed shuttle buswould provide services at every 10 minutes in the weekday peak periods and every 20 to 30 minutes during other times of the day and at the weekend.
- In addition to the shuttle bus service, the potential future introduction of light rail into the precinct would have the ability to signific antly reduce the travel by carmode. The recent introduction of Opal cards meansthat transfers/ changing modes at such interchanges is much easier and without the historic cost implic ations.

In summary, the traffic impacts of the proposed development could be mitigated by the list of measures desc ribed above.

Appendix A

## AppendixA

## Survey Results

## A. 1 Intersection Tuming Movement Diagrams

## A. 2 Origin-Destination Survey

## A. 3 Parking Occupancy \& Duration

## A. 4 Travel Time Survey

Appendix A

## A. 1 Intersection Tuming Movement Diagrams





## A. 2 Origin-Destination Survey



# 6439 - North Parramatta Origin Destination Survey 

August 2014

| JOB NUMBER | 6439 |
| :---: | :---: |
| JOB NAME | North Parramatta |
| CLIENT | GTA |
| SURVEY LOCATIONS | P2. Green Up Dr, West of Fleet St |
|  | P3. Bridge Rd, East of Paringa Ave |
| SURVEY TYPE | Origin Destination Survey |
| VEHICLE CLASS | 1. Light Vehicles |
|  | 2. Heavy Vehicles |
| MATCH TIME | Open |
| SURVEY TIME | 07:00 AM - 09:00 AM (THU); 4:00PM - 6:00PM (THU); 12:00PM - 2:00PM (SAT) |
| SURVEY DATE | Thursday 14/08/2014 \& Saturday 09/08/2 |
| WEATHER | Fine |

## 6439 - North Parramatta OD - Matrix

AUSTRAFFIC
Date
14/08/2014
Start Time
7:00
End Time
9:00
Match Time Open

Origin - Destination Matches - Class 1 - Light Vehicles


Origin - Destination Matches - Class 2 - Heavy Vehicles


Origin - Destination Matches - Total Vehicles


## 6439 - North Parramatta OD - Matrix

## AUSTRAFFIC

Date
14/08/2014
$\begin{array}{llll}\text { Start Time } & \text { 7:00 } & \text { End Time } & \text { 8:00 } \\ \text { Match Time } & \text { Open } & & \end{array}$

Origin - Destination Matches - Class 1 - Light Vehicles

| Survey Time | Destination | P2F | P3W | Total | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 7:00 8:00 |  |  |  |  |  |  |
| Origin | Recorded | 31 | 78 | 109 |  |  |
| P2W | 167 | 8 | 65 | 73 | 43.7\% | 94 |
| P3E | 111 | 9 | 4 | 13 | 11.7\% | 98 |
| Total | 278 | 17 | 69 | 86 | 30.9\% | 192 |

Origin - Destination Matches - Class 2 - Heavy Vehicles

$\left.$| Survey Time <br> $\mathbf{7 : 0 0}$ |  | Destination | P2E | P3W | Total | \% Matched |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | | Local |
| :---: |
| Destination | \right\rvert\,

Origin - Destination Matches - Total Vehicles

| Survey Time | Destination |  |  |  | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 7:00 8:00 |  |  |  |  |  |  |
| Origin | Recorded | 31 | 78 | 109 |  |  |
| P2W | 167 | 8 | 65 | 73 | 43.7\% | 94 |
| P3E | 111 | 9 | 4 | 13 | 11.7\% | 98 |
| Total | 278 | 17 | 69 | 86 | 30.9\% | 192 |

## 6439 - North Parramatta OD - Matrix

## AUSTRAFFIC

Date
14/08/2014
$\begin{array}{llll}\text { Start Time } & \text { 8:00 } & \text { End Time } & \text { 9:00 } \\ \text { Match Time } & \text { Open } & & \end{array}$

Origin - Destination Matches - Class 1 - Light Vehicles

| Survey Time | Destination | P2F | P3W | Total | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 8:00 9:00 |  |  |  |  |  |  |
| Origin | Recorded | 41 | 82 | 123 |  |  |
| P2W | 215 | 8 | 41 | 49 | 22.8\% | 166 |
| P3E | 140 | 20 | 8 | 28 | 20.0\% | 112 |
| Total | 355 | 28 | 49 | 77 | 21.7\% | 278 |

Origin - Destination Matches - Class 2 - Heavy Vehicles

$\left.$| Survey Time <br> $\mathbf{8 : 0 0}$ |  | Destination | P2E | P3W | Total | \% Matched |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | | Local |
| :---: |
| Destination | \right\rvert\,

Origin - Destination Matches - Total Vehicles

| Survey Time | Destination | P2E | P3W | Total | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 8:00 9:00 | Destination | P2E |  |  |  |  |
| Origin | Recorded | 42 | 82 | 124 |  |  |
| P2W | 215 | 8 | 41 | 49 | 22.8\% | 166 |
| P3E | 141 | 21 | 8 | 29 | 20.6\% | 112 |
| Total | 356 | 29 | 49 | 78 | 21.9\% | 278 |

## 6439 - North Parramatta OD - Matrix

## AUSTRAFFIC

Date
14/08/2014
Start Time
16:00
End Time
18:00
Match Time Open

Origin - Destination Matches - Class 1 - Light Vehicles

| Survey Time | Destination | P2F | P3W | Total |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16:00 18:00 |  |  |  |  | \% Matched |  |
| Origin | Recorded | 357 | 290 | 647 |  |  |
| P2W | 116 | 9 | 97 | 106 | 91.4\% | 10 |
| P3E | 164 | 125 | 5 | 130 | 79.3\% | 34 |
| Total | 280 | 134 | 102 | 236 | 84.3\% | 44 |
| \% Matched |  | 37.5\% | 35.2\% | 36.5\% |  |  |
| Local Origin |  | 223 | 188 | 411 |  |  |

Origin - Destination Matches - Class 2 - Heavy Vehicles

| Survey Time | Destination | P2E | P3W | Total | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16:00 18:00 |  |  |  |  |  |  |
| Origin | Recorded | 1 | 1 | 2 |  |  |
| P2W | 1 | 0 | 1 | 1 | 100.0\% | 0 |
| P3E | 0 | 0 | 0 | 0 | 0.0\% | 0 |
| Total | 1 | 0 | 1 | 1 | 100.0\% | 0 |
| \% Matched |  | 0.0\% | 100.0\% | 50.0\% |  |  |
| Local Origin |  | 1 | 0 | 1 |  |  |

Origin - Destination Matches - Total Vehicles


## 6439 - North Parramatta OD - Matrix

## AUSTRAFFIC

Date
14/08/2014

| Start Time | 16:00 | End Time | 17:00 |
| :--- | :--- | :--- | :--- |
| Match Time | Open |  |  |

Origin - Destination Matches - Class 1 - Light Vehicles

| Survey Time | Destination | P2E | P3W | Total | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16:00 17:00 | Destination |  |  |  |  |  |
| Origin | Recorded | 182 | 148 | 330 |  |  |
| P2W | 53 | 5 | 44 | 49 | 92.5\% | 4 |
| P3E | 81 | 63 | 4 | 67 | 82.7\% | 14 |
| Total | 134 | 68 | 48 | 116 | 86.6\% | 18 |

Origin - Destination Matches - Class 2 - Heavy Vehicles

| Survey Time | Destination |  |  |  | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16:00 17:00 | Destination | P2E | P3W | Total |  |  |
| Origin | Recorded | 1 | 1 | 2 |  |  |
| P2W | 1 | 0 | 1 | 1 | 100.0\% | 0 |
| P3E | 0 | 0 | 0 | 0 | 0.0\% | 0 |
| Total | 1 | 0 | 1 | 1 | 100.0\% | 0 |

Origin - Destination Matches - Total Vehicles

| Survey Time | Destination |  |  |  | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16:00 17:00 |  |  |  |  |  |  |
| Origin | Recorded | 183 | 149 | 332 |  |  |
| P2W | 54 | 5 | 45 | 50 | 92.6\% | 4 |
| P3E | 81 | 63 | 4 | 67 | 82.7\% | 14 |
| Total | 135 | 68 | 49 | 117 | 86.7\% | 18 |

## 6439 - North Parramatta OD - Matrix

## AUSTRAFFIC

Date
14/08/2014

| Start Time | 17:00 | End Time | 18:00 |
| :--- | :--- | :--- | :--- |
| Match Time | Open |  |  |

Origin - Destination Matches - Class 1 - Light Vehicles

| Survey Time | Destination | P2E | P3W | Total | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 17:00 18:00 | Destination |  |  |  |  |  |
| Origin | Recorded | 175 | 142 | 317 |  |  |
| P2W | 63 | 4 | 53 | 57 | 90.5\% | 6 |
| P3E | 83 | 62 | 1 | 63 | 75.9\% | 20 |
| Total | 146 | 66 | 54 | 120 | 82.2\% | 26 |

Origin - Destination Matches - Class 2 - Heavy Vehicles

| Survey Time | Destination |  |  |  | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 17:00 18:00 | Destination | P2E | P3W | Total |  |  |
| Origin | Recorded | 0 | 0 | 0 |  |  |
| P2W | 0 | 0 | 0 | 0 | 0.0\% | 0 |
| P3E | 0 | 0 | 0 | 0 | 0.0\% | 0 |
| Total | 0 | 0 | 0 | 0 | 0.0\% | 0 |

Origin - Destination Matches - Total Vehicles

| Survey Time | Destination | P2F | P3W | Total | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 17:00 18:00 |  |  |  |  |  |  |
| Origin | Recorded | 175 | 142 | 317 |  |  |
| P2W | 63 | 4 | 53 | 57 | 90.5\% | 6 |
| P3E | 83 | 62 | 1 | 63 | 75.9\% | 20 |
| Total | 146 | 66 | 54 | 120 | 82.2\% | 26 |

## 6439 - North Parramatta OD - Matrix

## AUSTRAFFIC

Date
9/08/2014
Start Time 12:00

$$
\text { End Time } \quad 14: 00
$$

Match Time 5 Minutes

Origin - Destination Matches - Class 1 - Light Vehicles


Origin - Destination Matches - Class 2 - Heavy Vehicles

| Survey Time | Destination | P2E | P3W | Total |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:00 14:00 |  |  |  |  | \% Matched | Destination |
| Origin | Recorded | 0 | 0 | 0 |  |  |
| P2W | 0 | 0 | 0 | 0 | 0.0\% | 0 |
| P3E | 0 | 0 | 0 | 0 | 0.0\% | 0 |
| Total | 0 | 0 | 0 | 0 | 0.0\% | 0 |
| \% Matched |  | 0.0\% | 0.0\% | 0.0\% |  |  |
| Local Origin |  | 0 | 0 | 0 |  |  |

Origin - Destination Matches - Total Vehicles


# 6439 - North Parramatta OD - Matrix 

## AUSTRAFFIC

Date
9/08/2014
Start Time 12:00
End Time 13:00
Match Time 5 Minutes

Origin - Destination Matches - Class 1 - Light Vehicles

| Survey Time | Destination |  |  |  | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:00 13:00 | Destination | P2E | P3W | otal |  |  |
| Origin | Recorded | 28 | 25 | 53 |  |  |
| P2W | 25 | 4 | 17 | 21 | 84.0\% | 4 |
| P3E | 22 | 13 | 2 | 15 | 68.2\% | 7 |
| Total | 47 | 17 | 19 | 36 | 76.6\% | 11 |

Origin - Destination Matches - Class 2 - Heavy Vehicles

| Survey Time | Destination | P2E | P3W | Total | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:00 13:00 |  |  |  |  |  |  |
| Origin | Recorded | 0 | 0 | 0 |  |  |
| P2W | 0 | 0 | 0 | 0 | 0.0\% | 0 |
| P3E | 0 | 0 | 0 | 0 | 0.0\% | 0 |
| Total | 0 | 0 | 0 | 0 | 0.0\% | 0 |

Origin - Destination Matches - Total Vehicles

| Survey Time |  |  |  |  | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 12:00 13:00 | Destination | P2E | P3W | Total |  |  |
| Origin | Recorded | 28 | 25 | 53 |  |  |
| P2W | 25 | 4 | 17 | 21 | 84.0\% | 4 |
| P3E | 22 | 13 | 2 | 15 | 68.2\% | 7 |
| Total | 47 | 17 | 19 | 36 | 76.6\% | 11 |

## 6439 - North Parramatta OD - Matrix

## AUSTRAFFIC

Date
9/08/2014
Start Time 13:00

$$
\text { End Time } \quad 14: 00
$$

Match Time 5 Minutes

Origin - Destination Matches - Class 1 - Light Vehicles

| Survey Time | Destination |  |  |  | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 13:00 14:00 | Destination | P2E | P3W | Total |  |  |
| Origin | Recorded | 21 | 26 | 47 |  |  |
| P2W | 40 | 4 | 23 | 27 | 67.5\% | 13 |
| P3E | 24 | 14 | 2 | 16 | 66.7\% | 8 |
| Total | 64 | 18 | 25 | 43 | 67.2\% | 21 |

Origin - Destination Matches - Class 2 - Heavy Vehicles

| Survey Time | Destination | P2E | P3W | Total | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 13:00 14:00 |  |  |  |  |  |  |
| Origin | Recorded | 0 | 0 | 0 |  |  |
| P2W | 0 | 0 | 0 | 0 | 0.0\% | 0 |
| P3E | 0 | 0 | 0 | 0 | 0.0\% | 0 |
| Total | 0 | 0 | 0 | 0 | 0.0\% | 0 |

Origin - Destination Matches - Total Vehicles

| Survey Time | Destination | P2E | P3W | Total | \% Matched | Local Destination |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 13:00 14:00 | Destination |  |  |  |  |  |
| Origin | Recorded | 21 | 26 | 47 |  |  |
| P2W | 40 | 4 | 23 | 27 | 67.5\% | 13 |
| P3E | 24 | 14 | 2 | 16 | 66.7\% | 8 |
| Total | 64 | 18 | 25 | 43 | 67.2\% | 21 |

Appendix A

## A. 3 Parking Occupancy \& Duration

| NsW | VIC | OLD | SA | wa | NT | ACT | TAS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |

## 6439 - Surveys at North Parramatta - PO \& PD

Aug-14

| JOB NUMBER | 6439 |
| :---: | :---: |
| JOB NAME | Surveys at North Parramatta |
| CLIENT | GTA |
| SURVEY TYPE | Parking Occupancy and Parking Duration |
| SURVEY DATE | Thursday 14/08/2014 \& Saturday 9/08/2014 |
| SURVEY PERIOD | 7:00 AM - 07:00 PM (THU); 9:00 AM - 05:00 PM (SAT) |
| WEATHER | Fine |



North Parramatta
GTA
14-08-14 - Thursday
Zone Inventory Summary

| Id | Location | Side of Street | Parking Type | Adjacent Land Use | Restrictions | Supply |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | Fleet Street |  |  |  |  |  |
| 2 | Fleet St, btw Fennel St \& Greenup Drive | West Side | Kerbside |  | 4P 8am-6pm Mon-Fri; Area 7 Resident Permit Excepted | 24 |
| 5 | Fleet St, btw Greenup Drive \& Factory St | West Side | Kerbside |  | 4P 8am-6pm Mon-Fri; Area 7 Resident Permit Excepted | 20 |
| 7 | Fleet St, No. 5A | West Side | Off Street |  | Private | 3 |
| 8 | Fleet St, No. 7 | West Side | Off Street |  | Private | 3 |
| 9 | Fleet St, No. 9, Chip Cottage | West Side | Off Street |  | Private, 90 Degree | 9 |
| A | Fleet Street |  |  |  |  | 59 |
| B | New Street |  |  |  |  |  |
| 11 | New St, btw Factory St \& Dunlop St | West Side | Kerbside |  | Unrestricted | 16 |
| 12 | New St, btw Factory St \& Dunlop St | West Side | Kerbside |  | No Parking; Authorised Vehicles Excepted | 2 |
| 13 | New St, No. 1 | West Side | Off Street |  | Private | 2 |
| 14 | New St, No. 3 | West Side | Off Street |  | Private | 2 |
| 15 | New St, No. 5 | West Side | Off Street |  | Private | 2 |
| 16 | New St, No. 9 | West Side | Off Street |  | Private | 2 |
| 17 | New St, No. 11 | West Side | Off Street |  | Private | 2 |
| B | New Street |  |  |  |  | 28 |
| c | Car Park 1 |  |  |  |  |  |
| 18 | Car Park 1 |  | Off Street |  | Authorised Parking only | 31 |
| c | Car Park 1 |  |  |  |  | 31 |
| D | Car Park 2 |  |  |  |  |  |
| 19 | Car Park 2 |  | Off Street |  | Authorised Parking only | 25 |
| D | Car Park 2 |  |  |  |  | 25 |
| E | Car Park 3 |  |  |  |  |  |
| 20 | Car Park 3 |  | Off Street |  | Unrestricted | 29 |
| E | Car Park 3 |  |  |  |  | 29 |
| F | Car Park 4 |  |  |  |  |  |
| 21 | Car Park 4 | Staff Parking Zone | Off Street |  | Staff Parking 90 Degree | 11 |
| 22 | Car Park 4 | Disabled Zone | Off Street |  | Disabled | 1 |
| 23 | Car Park 4 | Unrestricted Zone | Off Street |  | Unrestricted | 7 |
| F | Car Park 4 |  |  |  |  | 19 |
| G | Car Park 5 |  |  |  |  |  |
| 24 | Car Park 5, NSW Institute of Phsycology |  | Off Street |  | Unrestricted | 12 |
| G | Car Park 5 |  |  |  |  | 12 |
| H | Car Park 6 |  |  |  |  |  |
| 25 | Car Park 6, Warrinya Ave |  | Off Street |  | Unrestricted | 6 |
| H | Car Park 6 |  |  |  |  | 6 |
| 1 | Car Park 7 |  |  |  |  |  |
| 26 | Car Park 7, Warrinya Ave |  | Off Street |  | Unrestricted | 3 |
| 1 | Car Park 7 |  |  |  |  | 3 |
| 1 | Car Park 8 |  |  |  |  |  |
| 28 | Car Park 8, Warrinya Ave | Unrestricted Zone 1 | Off Street |  | Unrestricted | 3 |
| 30 | Car Park 8, Warrinya Ave | Unrestricted Zone 2 | Off Street |  | Unrestricted | 3 |
| 31 | Car Park 8, Warrinya Ave | Private | Off Street |  | Private | 2 |
| 32 | Car Park 8, Warrinya Ave | Credit Union Parking Zone | Off Street |  | Credit Union Parking Only | 6 |
| 33 | Car Park 8, Warrinya Ave | Disabled Zone | Off Street |  | Disabled | 1 |
| J | Car Park 8 |  |  |  |  | 15 |
| K | Car Park 9 |  |  |  |  |  |
| 34 | Car Park 9, Warrinya Ave |  | Off Street |  | Unrestricted | 10 |
| K | Car Park 9 |  |  |  |  | 10 |
| L | Car Park 10 |  |  |  |  |  |
| 35 | Car Park 10, WSAMHS |  | Off Street |  | Unrestricted | 12 |
| L | Car Park 10 |  |  |  |  | 12 |
| M | Car Park 11 |  |  |  |  |  |
| 36 | Car Park 11, WSAMHS |  | Off Street |  | Unrestricted | 3 |
| M | Car Park 11 |  |  |  |  | 3 |
| N | Car Park 12 |  |  |  |  |  |
| 37 | Car Park 12 |  | Off Street |  | Unrestricted | 16 |
| N | Car Park 12 |  |  |  |  | 16 |
| 0 | Car Park 13 |  |  |  |  |  |
| 38 | Car Park 13, Post Acute Care |  | Off Street |  | Unrestricted | 43 |
| 0 | Car Park 13 |  |  |  |  | 43 |
| P | Car Park 14 |  |  |  |  |  |
| 39 | Car Park 14, Post Acute Care |  | Off Street |  | Unrestricted | 32 |
| P | Car Park 14 |  |  |  |  | 32 |
| Q | Car Park 15 |  |  |  |  |  |
| 40 | Car Park 15, IT Services |  | Off Street |  | Unrestricted | 44 |
| Q | Car Park 15 |  |  |  |  | 44 |
| R | Car Park 16-River Road |  |  |  |  |  |
| 42 | River Rd, btw Eastern Circuit \& Warrinya Ave | North Side | Kerbside |  | Unrestricted | 5 |
| 44 | River Rd, btw Eastern Circuit \& Warrinya Ave | North Side | Kerbside |  | Unrestricted | 11 |

## austraffic

North Parramatta
GTA
14-08-14 - Thursday

| Id | Location | Side of Street | Parking Type | Adjacent Land Use | Restrictions | Supply |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| R | Car Park 16 - River Road |  |  |  |  | 16 |
| S | Car Park 17 |  |  |  |  |  |
| 46 | Car Park 17, Kalindi |  | Off Street |  | Unrestricted | 7 |
| 47 | Car Park 17, 68a | Unrestricted Zone | Off Street |  | Unrestricted | 15 |
| 5 | Car Park 17 |  |  |  |  | 22 |
| T | Car Park 18-Warrinya Avenue |  |  |  |  |  |
| 50 | Warrinya Ave, btw River Rd \& lane to Bunya | East Side | Kerbside |  | Unrestricted | 9 |
| 51 | Car Park 18 | Delivery Zone | Off Street |  | Delivery Zone | 2 |
| 52 | Car Park 18 | Unrestricted Zone | Off Street |  | Unrestricted | 22 |
| T | Car Park 18 - Warrinya Avenue |  |  |  |  | 33 |
| U | Car Park 19-Warrinya Avenue |  |  |  |  |  |
| 54 | Warrinya Ave, lane to Bunya \& Bridge St | East Side | Kerbside |  | Unrestricted | 3 |
| 55 | Car Park 19 | Unrestricted Zone | Off Street |  | Unrestricted | 7 |
| 56 | Car Park 19 | Loading Zone | Off Street |  | Loading Zone | 2 |
| $u$ | Car Park 19 - Warrinya Avenue |  |  |  |  | 12 |
| v | Car Park 20 |  |  |  |  |  |
| 57 | Car Park 20, Bunya | North Side | Kerbside |  | Unrestricted | 7 |
| v | Car Park 20 |  |  |  |  | 7 |
| w | Car Park 21 |  |  |  |  |  |
| 58 | Car Park 21, Bunya | South Side | Kerbside |  | Unrestricted | 8 |
| w | Car Park 21 |  |  |  |  | 8 |
| X | Car Park 22 |  |  |  |  |  |
| 60 | Car Park 22, Life Skills |  | Off Street |  | Unrestricted | 6 |
| x | Car Park 22 |  |  |  |  | 6 |
| Y | Car Park 23 |  |  |  |  |  |
| 61 | Eastern Circuit, Wirrabilla |  | Off Street |  | Unrestricted | 5 |
| 62 | Car Park 23, Wirrabilla |  | Off Street |  | Unrestricted | 3 |
| Y | Car Park 23 |  |  |  |  | 8 |
| z | Car Park 24 |  |  |  |  |  |
| 63 | Car Park 24, Gungura |  | Off Street |  | Risk Management Unit | 3 |
| 64 | Car Park 24, Gungura | Unrestricted (on grass) Zone | Off Street |  | Unrestricted | 8 |
| z | Car Park 24 |  |  |  |  | 11 |
| AA | Car Park 25 |  |  |  |  |  |
| 65 | Car Park 25, Bridgeway Cetnre | Unrestricted Zone | Off Street |  | Unrestricted | 24 |
| AA | Car Park 25 |  |  |  |  | 24 |
| AB | Car Park 26 |  |  |  |  |  |
| 67 | Car Park 26, Wattle Cottage | Unrestricted Zone | Off Street |  | Unrestricted | 4 |
| 68 | Car Park 26, Wattle Cottage | Delivery Zone | Off Street |  | Delivery Zone | 3 |
| AB | Car Park 26 |  |  |  |  | 7 |
| AC | Car Park 27 |  |  |  |  |  |
| 69 | Car Park 27 |  | Off Street |  | Unrestricted | 25 |
| AC | Car Park 27 |  |  |  |  | 25 |
| AD | Car Park 28 |  |  |  |  |  |
| 70 | Car Park 28, large grass area |  | Off Street |  | Unrestricted | 75 |
| AD | Car Park 28 |  |  |  |  | 75 |
| AE | Car Park 29 |  |  |  |  |  |
| 71 | Car Park 29, Centre for Addiction Medicine |  | Off Street |  | Ward Car only | 6 |
| AE | Car Park 29 |  |  |  |  | 6 |
| AF | Car Park 30 |  |  |  |  |  |
| 73 | Car Park 30 | Unrestricted Zone | Off Street |  | Unrestricted | 5 |
| AF | Car Park 30 |  |  |  |  | 5 |
| AG | Car Park 31 |  |  |  |  |  |
| 74 | Car Park 31, Health Support Services | Staff Parking Zone | Off Street |  | Staff Parking Only | 10 |
| 75 | Car Park 31, Health Support Services | Staff Parking Zone (under cover) | Off Street |  | Staff Parking Only | 13 |
| 76 | Car Park 31, Health Support Services | Staff Parking Zone (at back) | Off Street |  | Staff Parking Only | 47 |
| AG | Car Park 31 |  |  |  |  | 70 |
| AH | Car Park 32 |  |  |  |  |  |
| 77 | Car Park 32, Palm Circuit (on grass Sth) |  | Off Street |  | Unrestricted | 16 |
| 78 | Car Park 32, Palm Circuit (on East side) | -umberland Campus Staff Parkin, | Off Street |  | Cumberland Campus Staff Parking | 6 |
| 79 | Car Park 32, Palm Circuit (on East side) | Unrestricted Zone | Off Street |  | Unrestricted | 15 |
| AH | Car Park 32 |  |  |  |  | 37 |
| Al | Car Park 34 (Incl Car Park 33) |  |  |  |  |  |
| 80 | Car Park 34 (incl 33), large grass area |  | Off Street |  | Unrestricted | 51 |
| Al | Car Park 34 (Incl Car Park 33) |  |  |  |  | 51 |
| A | Car Park 35 |  |  |  |  |  |
| 81 | Car Park 35, Pine Cottage | Disabled Zone | Off Street |  | Disabled | 1 |
| 82 | Car Park 35, Pine Cottage | Unrestricted Zone | Off Street |  | Unrestricted | 17 |
| AJ | Car Park 35 |  |  |  |  | 18 |
| AK | Car Park 36 |  |  |  |  |  |
| 83 | Car Park 36, New Street | Unrestricted Zone | Off Street |  | Unrestricted | 35 |
| АK | Car Park 36 |  |  |  |  | 35 |

## austraffic

North Parramatta
GTA
14-08-14 - Thursday

| Id | Location | Side of Street | Parking Type | Adjacent Land Use | Restrictions | Supply |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AL | Car Park 37 |  |  |  |  |  |
| 84 | Car Park 37, Multicultural Health Unit | Unrestricted Zone | Off Street |  | Unrestricted | 12 |
| AL | Car Park 37 |  |  |  |  | 12 |
| AM | Car Park 38 |  |  |  |  |  |
| 85 | Car Park 38, behind gate | Unrestricted Zone | Off Street |  | Unrestricted | 4 |
| AM | Car Park 38 |  |  |  |  | 4 |
| AN | Car Park 39 |  |  |  |  |  |
| 86 | Car Park 39, Womens Health at Work | Unrestricted Zone | Off Street |  | Unrestricted | 2 |
| AN | Car Park 39 |  |  |  |  | 2 |
| AO | Car Park 40 |  |  |  |  |  |
| 88 | Car Park 40 | Unrestricted Zone | Off Street |  | Unrestricted | 6 |
| AO | Car Park 40 |  |  |  |  | 6 |
| AP | Car Park 41 |  |  |  |  |  |
| 89 | Car Park 41 | Ward Car Only Zone | Off Street |  | Ward Car only | 1 |
| 90 | Car Park 41 | Unrestricted Zone | Off Street |  | Unrestricted | 12 |
| AP | Car Park 41 |  |  |  |  | 13 |
| AQ | Car Park 42 |  |  |  |  |  |
| 91 | Car Park 42 | Area Pool Car only Zone | Off Street |  | Area Pool Car only | 6 |
| 92 | Car Park 42 | Unrestricted Zone | Off Street |  | Unrestricted | 2 |
| AQ | Car Park 42 |  |  |  |  | 8 |
| AR | Car Park 43 |  |  |  |  |  |
| 93 | Car Park 43, Transcultural Mental Health Services | Unrestricted (on grass) Zone | Off Street |  | Unrestricted | 2 |
| 94 | Car Park 43, Transcultural Mental Health Services | Unrestricted Zone | Off Street |  | Unrestricted | 28 |
| AR | Car Park 43 |  |  |  |  | 30 |
| AS | Car Park 44 |  |  |  |  |  |
| 95 | Car Park 44, Diversity Health Institute | Unrestricted Zone | Off Street |  | Unrestricted | 8 |
| AS | Car Park 44 |  |  |  |  | 8 |
| AT | Car Park 45 |  |  |  |  |  |
| 96 | Car Park 45, Innovation redesign | Unrestricted Zone | Off Street |  | Unrestricted | 8 |
| AT | Car Park 45 |  |  |  |  | 8 |
| AU | Car Park 46 |  |  |  |  |  |
| 97 | Car Park 46, Health Support Services | Unrestricted Zone | Off Street |  | Unrestricted | 2 |
| AU | Car Park 46 |  |  |  |  | 2 |
| AV | Car Park 47 |  |  |  |  |  |
| 98 | Car Park 47 | Unrestricted Zone | Off Street |  | Unrestricted | 4 |
| AV | Car Park 47 |  |  |  |  | 4 |
| AW | Car Park 48 |  |  |  |  |  |
| 99 | Car Park 48, Parramatta Linen Services | Authorised Parking only Zone | Off Street |  | Authorised Parking only | 18 |
| AW | Car Park 48 |  |  |  |  | 18 |
| AX | Car Park 49 |  |  |  |  |  |
| 100 | Car Park 49, Parramatta Linen Services (west side) | Authorised Parking only Zone | Off Street |  | Authorised Parking only | 6 |
| AX | Car Park 49 |  |  |  |  | 6 |
| AY | Car Park 50 |  |  |  |  |  |
| 101 | Car Park 50 | Unrestricted Zone | Off Street |  | Unrestricted | 20 |
| AY | Car Park 50 |  |  |  |  | 20 |


| $\begin{array}{\|c\|} \hline \text { Zone Group } \\ \text { Id } \end{array}$ | Location | Supply | Average Occupancy (\%) | Maximum Occupancy (\%) | Average Duration of Stay (minutes) | Maximum Duration of Stay (minutes) | Total Users (Vehicles) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | Fleet Street | 59 | 46\% | 75\% | 238 | 600 | 81 |
| B | New Street | 28 | 32\% | 46\% | 300 | 720 | 22 |
| c | Car Park 1 | 31 | 0\% | 0\% | 0 | 0 | 0 |
| D | Car Park 2 | 25 | 4\% | 12\% | 140 | 180 | 3 |
| E | Car Park 3 | 29 | 21\% | 45\% | 317 | 480 | 14 |
| F | Car Park 4 | 19 | 47\% | 79\% | 278 | 720 | 24 |
| G | Car Park 5 | 12 | 58\% | 100\% | 277 | 540 | 18 |
| H | Car Park 6 | 6 | 50\% | 100\% | 308 | 480 | 8 |
| 1 | Car Park 7 | 3 | 33\% | 100\% | 204 | 480 | 5 |
| J | Car Park 8 | 15 | 47\% | 80\% | 330 | 540 | 16 |
| к | Car Park 9 | 10 | 50\% | 90\% | 254 | 420 | 13 |
| L | Car Park 10 | 12 | 58\% | 92\% | 198 | 720 | 26 |
| M | Car Park 11 | 3 | 67\% | 100\% | 400 | 480 | 3 |
| N | Car Park 12 | 16 | 62\% | 94\% | 300 | 600 | 24 |
| 0 | Car Park 13 | 43 | 63\% | 86\% | 226 | 720 | 85 |
| P | Car Park 14 | 32 | 69\% | 88\% | 236 | 480 | 67 |
| Q | Car Park 15 | 44 | 73\% | 95\% | 227 | 720 | 101 |
| R | Car Park 16 - River Road | 16 | 56\% | 81\% | 297 | 480 | 22 |
| S | Car Park 17 | 22 | 64\% | 100\% | 264 | 720 | 38 |
| T | Car Park 18 - Warrinya Avenue | 33 | 70\% | 100\% | 285 | 480 | 57 |
| U | Car Park 19 - Warrinya Avenue | 12 | 50\% | 83\% | 218 | 420 | 19 |
| v | Car Park 20 | 7 | 71\% | 100\% | 325 | 480 | 12 |
| w | Car Park 21 | 8 | 62\% | 88\% | 248 | 480 | 15 |
| x | Car Park 22 | 6 | 50\% | 100\% | 189 | 540 | 13 |
| Y | Car Park 23 | 8 | 88\% | 100\% | 397 | 720 | 13 |
| z | Car Park 24 | 11 | 64\% | 100\% | 218 | 420 | 24 |
| AA | Car Park 25 | 24 | 54\% | 96\% | 237 | 480 | 40 |
| AB | Car Park 26 | 7 | 14\% | 29\% | 180 | 360 | 3 |
| AC | Car Park 27 | 25 | 44\% | 72\% | 298 | 540 | 26 |
| AD | Car Park 28 | 75 | 45\% | 72\% | 185 | 480 | 131 |
| AE | Car Park 29 | 6 | 83\% | 100\% | 549 | 720 | 7 |
| AF | Car Park 30 | 5 | 80\% | 100\% | 394 | 540 | 7 |
| AG | Car Park 31 | 70 | 64\% | 79\% | 230 | 540 | 140 |
| AH | Car Park 32 | 37 | 57\% | 84\% | 198 | 720 | 75 |
| Al | Car Park 34 (Incl Car Park 33) | 51 | 51\% | 84\% | 228 | 420 | 81 |
| AJ | Car Park 35 | 18 | 67\% | 100\% | 274 | 540 | 32 |
| AK | Car Park 36 | 35 | 9\% | 23\% | 136 | 360 | 15 |
| AL | Car Park 37 | 12 | 42\% | 92\% | 236 | 720 | 16 |
| AM | Car Park 38 | 4 | 25\% | 25\% | 210 | 240 | 2 |
| AN | Car Park 39 | 2 | 50\% | 100\% | 450 | 540 | 2 |
| AO | Car Park 40 | 6 | 83\% | 100\% | 300 | 540 | 11 |
| AP | Car Park 41 | 13 | 69\% | 92\% | 321 | 600 | 20 |
| AQ | Car Park 42 | 8 | 62\% | 88\% | 221 | 720 | 16 |
| AR | Car Park 43 | 30 | 67\% | 93\% | 277 | 600 | 53 |
| AS | Car Park 44 | 8 | 50\% | 75\% | 318 | 660 | 10 |
| AT | Car Park 45 | 8 | 62\% | 88\% | 214 | 420 | 16 |
| AU | Car Park 46 | 2 | 0\% | 50\% | 300 | 300 | 1 |
| AV | Car Park 47 | 4 | 25\% | 50\% | 330 | 480 | 2 |
| AW | Car Park 48 | 18 | 39\% | 61\% | 267 | 480 | 18 |
| AX | Car Park 49 | 6 | 33\% | 67\% | 250 | 420 | 6 |
| AY | Car Park 50 | 20 | 0\% | 5\% | 120 | 120 | 1 |
| TOTAL STUDY AREA |  | 1004 | 49\% | 70\% | 244 | 720 | 1454 |







| $\begin{array}{\|c\|} \hline \text { Zone Group } \\ \text { Id } \end{array}$ | Location | Supply | Average Occupancy (\%) | Maximum Occupancy (\%) | Average Duration of Stay (minutes) | Maximum Duration of Stay (minutes) | Total Users (Vehicles) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | Fleet Street | 59 | 39\% | 56\% | 113 | 480 | 96 |
| B | New Street | 28 | 21\% | 29\% | 294 | 480 | 10 |
| c | Car Park 1 | 31 | 13\% | 23\% | 249 | 360 | 7 |
| D | Car Park 2 | 25 | 4\% | 8\% | 150 | 180 | 2 |
| E | Car Park 3 | 29 | 0\% | 0\% | 0 | 0 | 0 |
| F | Car Park 4 | 19 | 0\% | 0\% | 0 | 0 | 0 |
| G | Car Park 5 | 12 | 0\% | 0\% | 0 | 0 | 0 |
| H | Car Park 6 | 6 | 0\% | 0\% | 0 | 0 | 0 |
| 1 | Car Park 7 | 3 | 0\% | 0\% | 0 | 0 | 0 |
| J | Car Park 8 | 15 | 0\% | 0\% | 0 | 0 | 0 |
| K | Car Park 9 | 10 | 10\% | 10\% | 480 | 480 | 1 |
| L | Car Park 10 | 12 | 17\% | 17\% | 360 | 480 | 2 |
| M | Car Park 11 | 3 | 0\% | 0\% | 0 | 0 | 0 |
| N | Car Park 12 | 16 | 25\% | 31\% | 348 | 480 | 5 |
| o | Car Park 13 | 43 | 35\% | 42\% | 393 | 480 | 18 |
| P | Car Park 14 | 32 | 34\% | 38\% | 348 | 480 | 15 |
| Q | Car Park 15 | 44 | 0\% | 0\% | 0 | 0 | 0 |
| R | Car Park 16 - River Road | 16 | 6\% | 6\% | 480 | 480 | 1 |
| S | Car Park 17 | 22 | 5\% | 5\% | 480 | 480 | 1 |
| T | Car Park 18 - Warrinya Avenue | 33 | 12\% | 12\% | 280 | 480 | 6 |
| U | Car Park 19 - Warrinya Avenue | 12 | 0\% | 8\% | 60 | 60 | 2 |
| v | Car Park 20 | 7 | 86\% | 100\% | 251 | 480 | 11 |
| w | Car Park 21 | 8 | 50\% | 88\% | 213 | 480 | 9 |
| x | Car Park 22 | 6 | 0\% | 0\% | 0 | 0 | 0 |
| Y | Car Park 23 | 8 | 38\% | 50\% | 390 | 480 | 4 |
| z | Car Park 24 | 11 | 0\% | 0\% | 0 | 0 | 0 |
| AA | Car Park 25 | 24 | 4\% | 4\% | 480 | 480 | 1 |
| AB | Car Park 26 | 7 | 0\% | 0\% | 0 | 0 | 0 |
| AC | Car Park 27 | 25 | 8\% | 8\% | 480 | 480 | 2 |
| AD | Car Park 28 | 75 | 7\% | 9\% | 369 | 480 | 7 |
| AE | Car Park 29 | 6 | 83\% | 83\% | 267 | 480 | 9 |
| AF | Car Park 30 | 5 | 0\% | 0\% | 0 | 0 | 0 |
| AG | Car Park 31 | 70 | 50\% | 53\% | 444 | 480 | 38 |
| AH | Car Park 32 | 37 | 11\% | 22\% | 225 | 480 | 8 |
| AI | Car Park 34 (Incl Car Park 33) | 51 | 0\% | 2\% | 60 | 60 | 1 |
| AJ | Car Park 35 | 18 | 6\% | 6\% | 480 | 480 | 1 |
| AK | Car Park 36 | 35 | 0\% | 0\% | 0 | 0 | 0 |
| AL | Car Park 37 | 12 | 25\% | 67\% | 188 | 480 | 8 |
| AM | Car Park 38 | 4 | 25\% | 25\% | 480 | 480 | 1 |
| AN | Car Park 39 | 2 | 0\% | 0\% | 0 | 0 | 0 |
| AO | Car Park 40 | 6 | 33\% | 50\% | 280 | 480 | 3 |
| AP | Car Park 41 | 13 | 31\% | 31\% | 480 | 480 | 4 |
| AQ | Car Park 42 | 8 | 75\% | 75\% | 480 | 480 | 6 |
| AR | Car Park 43 | 30 | 13\% | 13\% | 336 | 480 | 5 |
| AS | Car Park 44 | 8 | 0\% | 0\% | 0 | 0 | 0 |
| AT | Car Park 45 | 8 | 0\% | 0\% | 0 | 0 | 0 |
| AU | Car Park 46 | 2 | 0\% | 0\% | 0 | 0 | 0 |
| AV | Car Park 47 | 4 | 25\% | 25\% | 360 | 360 | 1 |
| AW | Car Park 48 | 18 | 56\% | 89\% | 296 | 360 | 16 |
| AX | Car Park 49 | 6 | 33\% | 50\% | 280 | 360 | 3 |
| AY | Car Park 50 | 20 | 5\% | 10\% | 270 | 360 | 2 |
| TOTAL STUDY AREA |  | 1004 | 17\% | 19\% | 266 | 480 | 306 |




austraffic


| total study area | 1004 | 159 | 187 | 193 | 185 | 170 | 170 | 157 | 136 | 169 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 16\% | 19\% | 19\% | 18\% | 17\% | 17\% | 16\% | 14\% | 17\% |





Appendix A

## A. 4 Travel Time Survey



Road Name: Church Street/Windsor Road between Campbell St and George St
Direction: Northbound

No of Runs: 4 runs
AM: 7:00-9:00

| Section | Road | Average |  |  | Average Delay |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Distance (km) | Time | Speed (km/h) | Mid Section | End Section |
| 1 | Church Street and George Street |  |  |  |  |  |
| 2 | Church Street and Victoria Road | 0.787 | 0:03:09 | 15.80 | 0:00:58 | 0:00:25 |
| 3 | Church Street and Pennant Hills Road | 0.574 | 0:01:26 | 24.36 | 0:00:21 | 0:00:00 |
| 4 | Church Street and Barney Street | 0.781 | 0:01:29 | 35.19 | 0:00:00 | 0:00:19 |
| 5 | Church Street and North Rocks Road | 0.449 | 0:00:50 | 35.71 | 0:00:11 | 0:00:00 |
| 6 | Church Street and Cumberland Highway | 0.304 | 0:01:35 | 14.18 | 0:00:01 | 0:00:53 |
| 7 | Windsor Road and Campbell St | 0.385 | 0:02:57 | 10.23 | 0:01:19 | 0:00:00 |
|  | Total | 3.279 | 0:11:24 | 17.25 | 0:02:48 | 0:01:37 |

Road Name: Church Street/Windsor Road between Campbell St and George St
No of Runs: 4 runs Direction: Southbound AM: 7:00-9:00

| Section | Road | Average |  |  | Average Delay |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Distance (km) | Time | Speed (km/h) | Mid Section | End Section |
| 7 | Windsor Road and Campbell St |  |  |  |  |  |
| 6 | Church Street and Cumberland Highway | 0.374 | 0:01:23 | 32.59 | 0:00:23 | 0:00:19 |
| 5 | Church Street and North Rocks Road | 0.303 | 0:00:29 | 40.57 | 0:00:00 | 0:00:00 |
| 4 | Church Street and Barney Street | 0.444 | 0:00:37 | 44.64 | 0:00:00 | 0:00:00 |
| 3 | Church Street and Pennant Hills Road | 0.788 | 0:02:27 | 20.55 | 0:00:35 | 0:00:21 |
| 2 | Church Street and Victoria Road | 0.578 | 0:02:10 | 16.93 | 0:00:07 | 0:00:56 |
| 1 | Church Street and George Street | 0.813 | 0:01:58 | 25.90 | 0:00:19 | 0:00:00 |
|  | Total | 3.300 | 0:09:05 | 21.79 | 0:01:23 | 0:01:36 |

Please Note:Delay is when the vehicle is travelling at less than $5 \mathrm{~km} / \mathrm{h}$. End delay is within 100 metres of the node. Mid delay is more than 100 metres from the node.

Road Name: Church Street/Windsor Road between Campbell St and George St
Direction: Northbound

No of Runs: 5 runs
PM: 16:00-18:00

| Section | Road | Average |  |  | Average Delay |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Distance (km) | Time | Speed (km/h) | Mid Section | End Section |
| 1 | Church Street and George Street |  |  |  |  |  |
| 2 | Church Street and Victoria Road | 0.811 | 0:03:13 | 15.84 | 0:00:52 | 0:00:27 |
| 3 | Church Street and Pennant Hills Road | 0.566 | 0:01:07 | 32.25 | 0:00:02 | 0:00:04 |
| 4 | Church Street and Barney Street | 0.786 | 0:01:54 | 26.48 | 0:00:08 | 0:00:31 |
| 5 | Church Street and North Rocks Road | 0.446 | 0:00:50 | 36.09 | 0:00:01 | 0:00:06 |
| 6 | Church Street and Cumberland Highway | 0.307 | 0:02:37 | 7.33 | 0:00:46 | 0:01:01 |
| 7 | Windsor Road and Campbell St | 0.369 | 0:00:31 | 43.66 | 0:00:00 | 0:00:00 |
|  | Total | 3.286 | 0:10:11 | 19.36 | 0:01:48 | 0:02:08 |

Road Name: Church Street/Windsor Road between Campbell St and George St
No of Runs: 5 runs PM: 16:00-18:00

| Section | Road | Average |  |  | Average Delay |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Distance (km) | Time | Speed (km/h) | Mid Section | End Section |
| 7 | Windsor Road and Campbell St |  |  |  |  |  |
| 6 | Church Street and Cumberland Highway | 0.364 | 0:00:27 | 48.88 | 0:00:00 | 0:00:00 |
| 5 | Church Street and North Rocks Road | 0.306 | 0:00:34 | 46.86 | 0:00:00 | 0:00:11 |
| 4 | Church Street and Barney Street | 0.442 | 0:00:46 | 42.83 | 0:00:00 | 0:00:12 |
| 3 | Church Street and Pennant Hills Road | 0.788 | 0:01:25 | 34.33 | 0:00:08 | 0:00:09 |
| 2 | Church Street and Victoria Road | 0.574 | 0:02:09 | 16.61 | 0:00:28 | 0:00:32 |
| 1 | Church Street and George Street | 0.829 | 0:02:56 | 18.27 | 0:00:53 | 0:00:02 |
|  | Total | 3.303 | 0:08:17 | 23.90 | 0:01:29 | 0:01:06 |

Please Note:Delay is when the vehicle is travelling at less than $5 \mathrm{~km} / \mathrm{h}$. End delay is within 100 metres of the node. Mid delay is more than 100 metres from the node.

Road Name: Church Street/Windsor Road between Campbell St and George St
Direction: Northbound

No of Runs: 5 runs
Mid: 12:00-14:00

| Section | Road | Average |  |  | Average Delay |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Distance (km) | Time | Speed (km/h) | Mid Section | End Section |
| 1 | Church Street and George Street |  |  |  |  |  |
| 2 | Church Street and Victoria Road | 0.814 | 0:03:03 | 16.49 | 0:00:55 | 0:00:10 |
| 3 | Church Street and Pennant Hills Road | 0.561 | 0:01:07 | 31.16 | 0:00:05 | 0:00:02 |
| 4 | Church Street and Barney Street | 0.786 | 0:01:31 | 34.83 | 0:00:11 | 0:00:14 |
| 5 | Church Street and North Rocks Road | 0.436 | 0:00:30 | 53.37 | 0:00:00 | 0:00:00 |
| 6 | Church Street and Cumberland Highway | 0.311 | 0:01:18 | 14.90 | 0:00:13 | 0:00:22 |
| 7 | Windsor Road and Campbell St | 0.368 | 0:00:28 | 47.09 | 0:00:00 | 0:00:00 |
|  | Total | 3.277 | 0:07:57 | 24.74 | 0:01:25 | 0:00:49 |

Road Name: Church Street/Windsor Road between Campbell St and George St
No of Runs: 5 runs Mid: 12:00-14:00

| Section | Road | Average |  |  | Average Delay |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Distance (km) | Time | Speed (km/h) | Mid Section | End Section |
| 7 | Windsor Road and Campbell St |  |  |  |  |  |
| 6 | Church Street and Cumberland Highway | 0.365 | 0:01:14 | 29.61 | 0:00:08 | 0:00:25 |
| 5 | Church Street and North Rocks Road | 0.326 | 0:01:01 | 25.20 | 0:00:00 | 0:00:23 |
| 4 | Church Street and Barney Street | 0.446 | 0:00:41 | 44.24 | 0:00:00 | 0:00:07 |
| 3 | Church Street and Pennant Hills Road | 0.784 | 0:01:50 | 25.85 | 0:00:15 | 0:00:19 |
| 2 | Church Street and Victoria Road | 0.578 | 0:01:45 | 21.85 | 0:00:00 | 0:00:42 |
| 1 | Church Street and George Street | 0.802 | 0:02:33 | 19.67 | 0:00:43 | 0:00:03 |
|  | Total | 3.301 | 0:09:04 | 21.84 | 0:01:07 | 0:01:59 |

Please Note:Delay is when the vehicle is travelling at less than $5 \mathrm{~km} / \mathrm{h}$. End delay is within 100 metres of the node. Mid delay is more than 100 metres from the node.


| Road Name: O'Connell Street between Macquarie St and Board St <br> Direction: <br> Northbound |
| :--- |
| \begin{tabular}{\|c|l|l|l|l|l|l|}
\hline
\end{tabular} |

Road Name: O'Connell Street between Macquarie St and Board St No of Runs: 12 runs Direction: Southbound

AM: 7:00-9:00

| Section | Road | Average |  |  | Average Delay |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Distance (km) | Time | Speed (km/h) | Mid Section | End Section |
| 5 | O'Connelll Street and Board Street |  |  |  |  |  |
| 4 | O'Connelll Street and Albert Street | 0.849 | 0:01:16 | 41.47 | 0:00:01 | 0:00:04 |
| 3 | O'Connelll Street and Victoria Road | 0.578 | 0:01:37 | 26.86 | 0:00:20 | 0:00:13 |
| 2 | O'Connelll Street and George Street | 0.538 | 0:01:01 | 33.86 | 0:00:02 | 0:00:10 |
| 1 | O'Connelll Street and Macquarie Street | 0.164 | 0:00:18 | 36.44 | 0:00:00 | 0:00:01 |
|  | Total | 2.129 | 0:04:12 | 30.47 | 0:00:24 | 0:00:28 |

Please Note:Delay is when the vehicle is travelling at less than $5 \mathrm{~km} / \mathrm{h}$. End delay is within 100 metres of the node. Mid delay is more than 100 metres from the node.

Road Name: O'Connell Street between Macquarie St and Board St Direction: Northbound

No of Runs: 11 runs
PM: 16:00-18:00

| Section | Road | Average |  |  | Average Delay |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Distance (km) | Time | Speed (km/h) | Mid Section | End Section |
| 1 | O'Connelll Street and Macquarie Street |  |  |  |  |  |
| 2 | O'Connelll Street and George Street | 0.188 | 0:00:17 | 40.54 | 0:00:00 | 0:00:00 |
| 3 | O'Connelll Street and Victoria Road | 0.533 | 0:00:37 | 52.52 | 0:00:00 | 0:00:01 |
| 4 | O'Connelll Street and Albert Street | 0.568 | 0:00:56 | 37.99 | 0:00:06 | 0:00:01 |
| 5 | O'Connelll Street and Board Street | 0.893 | 0:01:27 | 37.92 | 0:00:02 | 0:00:06 |
|  | Total | 2.181 | 0:03:17 | 39.96 | 0:00:08 | 0:00:08 |

Road Name: O'Connell Street between Macquarie St and Board St No of Runs: 11 runs Direction: Southbound

PM: 16:00-18:00

| Section | Road | Average |  |  | Average Delay |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Distance (km) | Time | Speed (km/h) | Mid Section | End Section |
| 5 | O'Connelll Street and Board Street |  |  |  |  |  |
| 4 | O'Connelll Street and Albert Street | 0.855 | 0:01:26 | 38.03 | 0:00:09 | 0:00:05 |
| 3 | O'Connelll Street and Victoria Road | 0.565 | 0:01:07 | 35.14 | 0:00:02 | 0:00:18 |
| 2 | O'Connelll Street and George Street | 0.532 | 0:01:17 | 32.25 | 0:00:11 | 0:00:13 |
| 1 | O'Connelll Street and Macquarie Street | 0.171 | 0:00:28 | 35.53 | 0:00:05 | 0:00:01 |
|  | Total | 2.124 | 0:04:18 | 29.65 | 0:00:27 | 0:00:37 |

Please Note:Delay is when the vehicle is travelling at less than $5 \mathrm{~km} / \mathrm{h}$. End delay is within 100 metres of the node. Mid delay is more than 100 metres from the node.

Road Name: O'Connell Street between Macquarie St and Board St Direction: Northbound

No of Runs: 13 runs
Mid: 12:00-14:00

| Section | Road | Average |  |  | Average Delay |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Distance (km) | Time | Speed (km/h) | Mid Section | End Section |
| 1 | O'Connelll Street and Macquarie Street |  |  |  |  |  |
| 2 | O'Connelll Street and George Street | 0.187 | 0:00:33 | 32.42 | 0:00:14 | 0:00:00 |
| 3 | O'Connelll Street and Victoria Road | 0.536 | 0:00:36 | 53.64 | 0:00:00 | 0:00:01 |
| 4 | O'Connelll Street and Albert Street | 0.565 | 0:00:46 | 46.65 | 0:00:02 | 0:00:00 |
| 5 | O'Connelll Street and Board Street | 0.863 | 0:01:15 | 42.72 | 0:00:01 | 0:00:05 |
|  | Total | 2.152 | 0:03:09 | 40.95 | 0:00:17 | 0:00:06 |

Road Name: O'Connell Street between Macquarie St and Board St
Direction: Southbound $\quad \begin{array}{r}\text { No of Runs: } \\ \text { Mid: } \\ \text { 12:00-1 }\end{array}$
Mid: 12:00-14:00

| Section | Road | Average |  |  | Average Delay |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Distance (km) | Time | Speed (km/h) | Mid Section | End Section |
| 5 | O'Connelll Street and Board Street |  |  |  |  |  |
| 4 | O'Connelll Street and Albert Street | 0.851 | 0:01:20 | 40.15 | 0:00:01 | 0:00:07 |
| 3 | O'Connelll Street and Victoria Road | 0.570 | 0:01:04 | 36.10 | 0:00:07 | 0:00:11 |
| 2 | O'Connelll Street and George Street | 0.536 | 0:00:40 | 49.59 | 0:00:01 | 0:00:01 |
| 1 | O'Connelll Street and Macquarie Street | 0.169 | 0:00:20 | 41.97 | 0:00:00 | 0:00:05 |
|  | Total | 2.126 | 0:03:24 | 37.53 | 0:00:09 | 0:00:23 |

Please Note:Delay is when the vehicle is travelling at less than $5 \mathrm{~km} / \mathrm{h}$. End delay is within 100 metres of the node. Mid delay is more than 100 metres from the node.

## Appendix B

Modelling Results

## B. 1 Existing Conditions

## B. 2 Future Conditions

B. 3 Future Conditions with Intersection Upgrades

Appendix B

## B. 1 Existing Conditions

14S1091200 PNUR
Existing Thursday AM
O'Connell St/ Fennell St
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. <br> Satn <br> v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 179 | 0.0 | 0.195 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.83 | 49.0 |
| 2 | T | 572 | 0.0 | 0.195 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 3 | R | 39 | 0.0 | 0.087 | 15.3 | LOS B | 0.3 | 1.8 | 0.71 | 0.90 | 42.2 |
| Approa |  | 789 | 0.0 | 0.195 | 2.6 | NA | 0.3 | 1.8 | 0.03 | 0.23 | 56.0 |
| East: Fennell Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 9 | 0.0 | 0.028 | 19.5 | LOS B | 0.1 | 0.6 | 0.72 | 0.98 | 40.1 |
| 5 | T | 11 | 0.0 | 0.297 | 118.3 | LOS F | 0.9 | 6.0 | 0.97 | 1.02 | 14.4 |
| 6 | R | 1 | 0.0 | 0.297 | 117.6 | LOS F | 0.9 | 6.0 | 0.97 | 1.02 | 14.4 |
| Approa |  | 21 | 0.0 | 0.297 | 73.8 | LOS F | 0.9 | 6.0 | 0.86 | 1.00 | 20.2 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 27 | 0.0 | 0.283 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.06 | 49.0 |
| 8 | T | 1075 | 0.0 | 0.283 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 9 | R | 61 | 0.0 | 0.082 | 11.5 | LOS A | 0.3 | 2.0 | 0.50 | 0.76 | 45.6 |
| Approa |  | 1163 | 0.0 | 0.283 | 0.8 | NA | 0.3 | 2.0 | 0.03 | 0.06 | 58.7 |
| West: Fennell Street |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 19 | 0.0 | 0.031 | 13.3 | LOS A | 0.1 | 0.6 | 0.49 | 0.88 | 44.8 |
| 11 | T | 13 | 0.0 | 0.566 | 82.3 | LOS F | 2.2 | 15.4 | 0.97 | 1.09 | 18.8 |
| 12 | R | 38 | 0.0 | 0.566 | 81.6 | LOS F | 2.2 | 15.4 | 0.97 | 1.09 | 18.8 |
| Approach |  | 69 | 0.0 | 0.566 | 63.1 | LOS E | 2.2 | 15.4 | 0.84 | 1.03 | 22.3 |
| All Vehicles |  | 2043 | 0.0 | 0.566 | 4.4 | NA | 2.2 | 15.4 | 0.07 | 0.17 | 53.7 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Thursday AM
Marsden St/ Market St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Marsden Street |  |  |  |  |  |  |  |  |  |  |
| 2 T | 358 | 0.0 | 0.322 | 1.4 | LOS A | 2.3 | 16.2 | 0.41 | 0.00 | 51.8 |
| 3 R | 196 | 0.0 | 0.322 | 9.9 | LOS A | 2.3 | 16.2 | 0.41 | 0.83 | 48.4 |
| Approach | 554 | 0.0 | 0.322 | 4.4 | NA | 2.3 | 16.2 | 0.41 | 0.29 | 50.6 |
| East: Market Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 356 | 0.0 | 0.277 | 9.4 | LOS A | 1.5 | 10.3 | 0.41 | 0.66 | 47.2 |
| 6 R | 3 | 0.0 | 0.277 | 9.6 | LOS A | 1.5 | 10.3 | 0.41 | 0.82 | 47.3 |
| Approach | 359 | 0.0 | 0.277 | 9.4 | LOS A | 1.5 | 10.3 | 0.41 | 0.66 | 47.2 |
| North: Marsden Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 34 | 0.0 | 0.149 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.01 | 49.0 |
| 8 T | 255 | 0.0 | 0.149 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 288 | 0.0 | 0.149 | 1.0 | NA | 0.0 | 0.0 | 0.00 | 0.12 | 58.5 |
| All Vehicles | 1201 | 0.0 | 0.322 | 5.1 | NA | 2.3 | 16.2 | 0.31 | 0.36 | 51.1 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Thursday AM
Church St/ Market St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Church Street 0 |  |  |  |  |  |  |  |  |  |  |
| 1 L | 21 | 0.0 | 0.024 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.84 | 49.0 |
| 2 T | 25 | 0.0 | 0.024 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 46 | 0.0 | 0.024 | 3.7 | NA | 0.0 | 0.0 | 0.00 | 0.38 | 54.4 |
| North: Church Street |  |  |  |  |  |  |  |  |  |  |
| 8 T | 18 | 0.0 | 0.009 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 9 R | 335 | 0.0 | 0.289 | 8.8 | LOS A | 1.0 | 6.8 | 0.29 | 0.59 | 47.5 |
| Approach | 353 | 0.0 | 0.289 | 8.4 | NA | 1.0 | 6.8 | 0.27 | 0.56 | 48.0 |
| West: Market Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 207 | 0.0 | 0.138 | 8.4 | LOS A | 0.7 | 4.7 | 0.10 | 0.63 | 48.5 |
| 12 R | 6 | 0.0 | 0.138 | 8.6 | LOS A | 0.7 | 4.7 | 0.10 | 0.73 | 48.3 |
| Approach | 214 | 0.0 | 0.138 | 8.4 | LOS A | 0.7 | 4.7 | 0.10 | 0.63 | 48.5 |
| All Vehicles | 613 | 0.0 | 0.289 | 8.0 | NA | 1.0 | 6.8 | 0.19 | 0.57 | 48.6 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Thursday AM
Church St/ Board St/ Seville St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Church Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 13 | 0.0 | 0.198 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.07 | 49.0 |
| 2 T | 759 | 0.0 | 0.198 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 772 | 0.0 | 0.198 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.02 | 59.8 |
| East: Seville Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 3 | 0.0 | 0.030 | 39.5 | LOS C | 0.1 | 0.6 | 0.92 | 0.97 | 28.7 |
| Approach | 3 | 0.0 | 0.030 | 39.5 | LOS C | 0.1 | 0.6 | 0.92 | 0.97 | 28.7 |
| North: Church Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 22 | 0.0 | 0.493 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.07 | 49.0 |
| 8 T | 1901 | 0.0 | 0.493 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 1923 | 0.0 | 0.493 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 59.8 |
| West: Board Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 302 | 0.0 | 0.451 | 14.2 | LOS A | 2.6 | 18.2 | 0.66 | 0.98 | 43.1 |
| Approach | 302 | 0.0 | 0.451 | 14.2 | LOS A | 2.6 | 18.2 | 0.66 | 0.98 | 43.1 |
| All Vehicles | 3000 | 0.0 | 0.493 | 1.6 | NA | 2.6 | 18.2 | 0.07 | 0.11 | 57.5 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

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SIDRA INTERSECTION 5.1.13.2093

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Project: \lgta-syd-ss1\project files\14S1000-1099\14S1091200 PNUR - Rezoning\ModellingISIDRA
14S1091200sid_SIDRA Existing Thursday AM.sip
8000056, GTA CONSULTANTS, ENTERPRISE

14S1091200 PNUR
Existing Thursday AM
O'Connell St/ Barney St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| $2 \quad \mathrm{~T}$ | 296 | 0.0 | 0.217 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 3 R | 121 | 0.0 | 0.217 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.99 | 48.6 |
| Approach | 417 | 0.0 | 0.217 | 2.5 | NA | 0.0 | 0.0 | 0.00 | 0.29 | 56.2 |
| East: Barney Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 852 | 0.0 | 0.474 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.67 | 49.0 |
| 6 R | 29 | 0.0 | 0.474 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.73 | 48.6 |
| Approach | 881 | 0.0 | 0.474 | 8.2 | NA | 0.0 | 0.0 | 0.00 | 0.67 | 48.9 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 6 | 0.0 | 0.033 | 12.5 | LOS A | 0.1 | 0.9 | 0.40 | 0.58 | 44.8 |
| 8 T | 15 | 0.0 | 0.033 | 11.3 | LOS A | 0.1 | 0.9 | 0.40 | 0.70 | 45.7 |
| Approach | 21 | 0.0 | 0.033 | 11.6 | LOS A | 0.1 | 0.9 | 0.40 | 0.67 | 45.4 |
| All Vehicles | 1319 | 0.0 | 0.474 | 6.4 | NA | 0.1 | 0.9 | 0.01 | 0.55 | 51.0 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Thursday AM
O'Connell St/ Dunlop St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
|  |  |  |  |  |  |  |  |  |  |  |
| 1 L | 3 | 0.0 | 0.216 | 13.5 | LOS A | 2.3 | 16.3 | 0.76 | 0.26 | 46.8 |
| 2 T | 393 | 0.0 | 0.216 | 5.3 | LOS A | 2.3 | 16.3 | 0.76 | 0.00 | 47.7 |
| 3 R | 9 | 0.0 | 0.216 | 13.8 | LOS A | 2.3 | 16.3 | 0.76 | 1.04 | 46.8 |
| Approach | 405 | 0.0 | 0.216 | 5.6 | NA | 2.3 | 16.3 | 0.76 | 0.03 | 47.7 |
| East: Dunlop Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 17 | 0.0 | 0.040 | 13.6 | LOS A | 0.1 | 0.9 | 0.65 | 0.83 | 43.7 |
| 5 T | 3 | 0.0 | 0.040 | 12.4 | LOS A | 0.1 | 0.9 | 0.65 | 0.85 | 44.3 |
| $6 \quad \mathrm{R}$ | 1 | 0.0 | 0.040 | 13.9 | LOS A | 0.1 | 0.9 | 0.65 | 0.91 | 43.6 |
| Approach | 21 | 0.0 | 0.040 | 13.4 | LOS A | 0.1 | 0.9 | 0.65 | 0.84 | 43.8 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 3 | 0.0 | 0.465 | 11.5 | LOS A | 6.6 | 46.3 | 0.71 | 0.29 | 48.4 |
| 8 T | 773 | 0.0 | 0.465 | 3.3 | LOS A | 6.6 | 46.3 | 0.71 | 0.00 | 48.1 |
| 9 R | 84 | 0.0 | 0.465 | 11.7 | LOS A | 6.6 | 46.3 | 0.71 | 0.98 | 48.4 |
| Approach | 860 | 0.0 | 0.465 | 4.1 | NA | 6.6 | 46.3 | 0.71 | 0.10 | 48.1 |
| West: Dunlop Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 15 | 0.0 | 0.040 | 13.0 | LOS A | 0.1 | 0.9 | 0.53 | 0.67 | 44.3 |
| 11 T | 5 | 0.0 | 0.040 | 11.7 | LOS A | 0.1 | 0.9 | 0.53 | 0.79 | 45.0 |
| 12 R | 2 | 0.0 | 0.040 | 13.2 | LOS A | 0.1 | 0.9 | 0.53 | 0.88 | 44.2 |
| Approach | 22 | 0.0 | 0.040 | 12.7 | LOS A | 0.1 | 0.9 | 0.53 | 0.72 | 44.4 |
| All Vehicles | 1308 | 0.0 | 0.465 | 4.9 | NA | 6.6 | 46.3 | 0.72 | 0.10 | 47.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Thursday AM
New St/ Factory St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: New Street |  |  |  |  |  |  |  |  |  |  |
| 2 T | 23 | 0.0 | 0.014 | 0.2 | LOS A | 0.1 | 0.5 | 0.18 | 0.00 | 56.3 |
| 3 R | 3 | 0.0 | 0.014 | 8.7 | LOS A | 0.1 | 0.5 | 0.18 | 0.96 | 48.8 |
| Approach | 26 | 0.0 | 0.014 | 1.2 | NA | 0.1 | 0.5 | 0.18 | 0.11 | 55.3 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 15 | 0.0 | 0.013 | 8.4 | LOS A | 0.1 | 0.4 | 0.17 | 0.60 | 48.2 |
| 6 R | 4 | 0.0 | 0.013 | 8.7 | LOS A | 0.1 | 0.4 | 0.17 | 0.67 | 48.0 |
| Approach | 19 | 0.0 | 0.013 | 8.5 | LOS A | 0.1 | 0.4 | 0.17 | 0.62 | 48.2 |
| North: New Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 9 | 0.0 | 0.044 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.01 | 49.0 |
| 8 T | 76 | 0.0 | 0.044 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 85 | 0.0 | 0.044 | 0.9 | NA | 0.0 | 0.0 | 0.00 | 0.11 | 58.5 |
| All Vehicles | 131 | 0.0 | 0.044 | 2.1 | NA | 0.1 | 0.5 | 0.06 | 0.19 | 56.1 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Thursday AM
O'Connell St/ Factory St
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective <br> Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 17 | 0.0 | 0.209 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.06 | 49.0 |
| 2 T | 391 | 0.0 | 0.209 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 407 | 0.0 | 0.209 | 0.3 | NA | 0.0 | 0.0 | 0.00 | 0.04 | 59.4 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 42 | 0.0 | 0.080 | 16.2 | LOS B | 0.2 | 1.6 | 0.65 | 1.00 | 42.6 |
| Approach | 42 | 0.0 | 0.080 | 16.2 | LOS B | 0.2 | 1.6 | 0.65 | 1.00 | 42.6 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 8 | 0.0 | 0.401 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.08 | 49.0 |
| 8 T | 774 | 0.0 | 0.401 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 782 | 0.0 | 0.401 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 59.9 |
| West: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 14 | 0.0 | 0.014 | 12.4 | LOS A | 0.1 | 0.4 | 0.43 | 0.83 | 45.6 |
| Approach | 14 | 0.0 | 0.014 | 12.4 | LOS A | 0.1 | 0.4 | 0.43 | 0.83 | 45.6 |
| All Vehicles | 1245 | 0.0 | 0.401 | 0.8 | NA | 0.2 | 1.6 | 0.03 | 0.06 | 58.7 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

Processed: Tuesday, 26 August 2014 3:04:01 PM
SIDRA INTERSECTION 5.1.13.2093
Project: Ilgta-syd-ss1\project files\14S1000-1099\14S1091200 PNUR - Rezoning\ModellingISIDRA
114S1091200sid_SIDRA Existing Thursday AM.sip
8000056, GTA CONSULTANTS, ENTERPRISE

SIDRA
INTERSECTION

14S1091200 PNUR
Existing Thursday AM
O'Connell St/ Board St/ Property Access
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue <br> Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 2 T | 23 | 0.0 | 0.169 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 3 R | 293 | 0.0 | 0.169 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.72 | 48.6 |
| Approach | 316 | 0.0 | 0.169 | 7.8 | NA | 0.0 | 0.0 | 0.00 | 0.67 | 49.3 |
| East: Board Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 15 | 0.0 | 0.012 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.66 | 49.0 |
| 6 R | 7 | 0.0 | 0.012 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.72 | 48.6 |
| Approach | 22 | 0.0 | 0.012 | 8.3 | NA | 0.0 | 0.0 | 0.00 | 0.68 | 48.8 |
| North: Access Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 11 | 0.0 | 0.011 | 8.9 | LOS A | 0.0 | 0.3 | 0.12 | 0.66 | 48.2 |
| 8 T | 3 | 0.0 | 0.011 | 7.6 | LOS A | 0.0 | 0.3 | 0.12 | 0.52 | 49.5 |
| Approach | 14 | 0.0 | 0.011 | 8.6 | LOS A | 0.0 | 0.3 | 0.12 | 0.63 | 48.5 |
| All Vehicles | 352 | 0.0 | 0.169 | 7.9 | NA | 0.0 | 0.3 | 0.00 | 0.67 | 49.2 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Thursday AM
New St/ Greenup Drive
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \mathrm{HV} \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: New Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 148 | 0.0 | 0.092 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.70 | 49.0 |
| 2 T | 23 | 0.0 | 0.092 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 172 | 0.0 | 0.092 | 7.1 | NA | 0.0 | 0.0 | 0.00 | 0.61 | 50.2 |
| North: New Street |  |  |  |  |  |  |  |  |  |  |
| 8 T | 31 | 0.0 | 0.063 | 0.5 | LOS A | 0.3 | 2.1 | 0.28 | 0.00 | 53.5 |
| 9 R | 73 | 0.0 | 0.063 | 9.0 | LOS A | 0.3 | 2.1 | 0.28 | 0.71 | 47.9 |
| Approach | 103 | 0.0 | 0.063 | 6.5 | NA | 0.3 | 2.1 | 0.28 | 0.50 | 49.5 |
| West: Greenup Drive |  |  |  |  |  |  |  |  |  |  |
| 10 L | 15 | 0.0 | 0.034 | 8.8 | LOS A | 0.1 | 0.9 | 0.23 | 0.59 | 47.9 |
| 12 R | 27 | 0.0 | 0.034 | 9.0 | LOS A | 0.1 | 0.9 | 0.23 | 0.67 | 47.8 |
| Approach | 42 | 0.0 | 0.034 | 8.9 | LOS A | 0.1 | 0.9 | 0.23 | 0.64 | 47.8 |
| All Vehicles | 317 | 0.0 | 0.092 | 7.1 | NA | 0.3 | 2.1 | 0.12 | 0.58 | 49.6 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Thursday PM
O'Connell St/ Fennell St
Stop (Two-Way)


Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Thursday PM
Marsden St/ Market St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Marsden Street |  |  |  |  |  |  |  |  |  |  |
| 2 T | 558 | 0.0 | 0.479 | 2.3 | LOS A | 5.4 | 38.1 | 0.51 | 0.00 | 50.3 |
| 3 R | 268 | 0.0 | 0.479 | 10.8 | LOS A | 5.4 | 38.1 | 0.51 | 0.86 | 47.8 |
| Approach | 826 | 0.0 | 0.479 | 5.1 | NA | 5.4 | 38.1 | 0.51 | 0.28 | 49.5 |
| East: Market Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 274 | 0.0 | 0.235 | 9.6 | LOS A | 1.2 | 8.2 | 0.41 | 0.66 | 47.2 |
| $6 \quad \mathrm{R}$ | 9 | 0.0 | 0.235 | 9.9 | LOS A | 1.2 | 8.2 | 0.41 | 0.84 | 47.3 |
| Approach | 283 | 0.0 | 0.235 | 9.6 | LOS A | 1.2 | 8.2 | 0.41 | 0.67 | 47.2 |
| North: Marsden Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 28 | 0.0 | 0.156 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.03 | 49.0 |
| 8 T | 275 | 0.0 | 0.156 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 303 | 0.0 | 0.156 | 0.8 | NA | 0.0 | 0.0 | 0.00 | 0.10 | 58.8 |
| All Vehicles | 1413 | 0.0 | 0.479 | 5.0 | NA | 5.4 | 38.1 | 0.38 | 0.32 | 50.7 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Thursday PM
Church St/ Market St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Church Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 12 | 0.0 | 0.016 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.88 | 49.0 |
| 2 T | 20 | 0.0 | 0.016 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 32 | 0.0 | 0.016 | 3.0 | NA | 0.0 | 0.0 | 0.00 | 0.32 | 55.4 |
| North: Church Street |  |  |  |  |  |  |  |  |  |  |
| 8 T | 12 | 0.0 | 0.006 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 9 R | 269 | 0.0 | 0.233 | 8.7 | LOS A | 0.7 | 5.2 | 0.26 | 0.59 | 47.6 |
| Approach | 281 | 0.0 | 0.233 | 8.4 | NA | 0.7 | 5.2 | 0.25 | 0.56 | 48.0 |
| West: Market Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 286 | 0.0 | 0.183 | 8.3 | LOS A | 0.9 | 6.6 | 0.09 | 0.63 | 48.5 |
| 12 R | 4 | 0.0 | 0.183 | 8.6 | LOS A | 0.9 | 6.6 | 0.09 | 0.73 | 48.3 |
| Approach | 291 | 0.0 | 0.183 | 8.3 | LOS A | 0.9 | 6.6 | 0.09 | 0.63 | 48.5 |
| All Vehicles | 603 | 0.0 | 0.233 | 8.1 | NA | 0.9 | 6.6 | 0.16 | 0.58 | 48.6 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Thursday PM
Church St/ Board St/ Seville St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Church Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 5 | 0.0 | 0.461 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.09 | 49.0 |
| 2 T | 894 | 0.0 | 0.461 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 899 | 0.0 | 0.461 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 59.9 |
| East: Seville Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 9 | 0.0 | 0.022 | 14.9 | LOS B | 0.1 | 0.5 | 0.68 | 0.84 | 42.5 |
| Approach | 9 | 0.0 | 0.022 | 14.9 | LOS B | 0.1 | 0.5 | 0.68 | 0.84 | 42.5 |
| North: Church Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 14 | 0.0 | 0.283 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.07 | 49.0 |
| 8 T | 1088 | 0.0 | 0.283 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 1102 | 0.0 | 0.283 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 59.8 |
| West: Board Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 402 | 0.0 | 0.871 | 33.2 | LOS C | 9.7 | 67.9 | 0.94 | 1.61 | 31.3 |
| Approach | 402 | 0.0 | 0.871 | 33.2 | LOS C | 9.7 | 67.9 | 0.94 | 1.61 | 31.3 |
| All Vehicles | 2413 | 0.0 | 0.871 | 5.6 | NA | 9.7 | 67.9 | 0.16 | 0.28 | 51.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

Processed: Wednesday, 27 August 2014 9:40:36 AM
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I14S1091200sid_SIDRA Existing Thursday PM.sip
8000056, GTA CONSULTANTS, ENTERPRISE

14S1091200 PNUR
Existing Thursday PM
O'Connell St/ Barney St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 2 T | 379 | 0.0 | 0.411 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 3 R | 402 | 0.0 | 0.411 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.87 | 48.6 |
| Approach | 781 | 0.0 | 0.411 | 4.3 | NA | 0.0 | 0.0 | 0.00 | 0.45 | 53.6 |
| East: Barney Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 409 | 0.0 | 0.234 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.67 | 49.0 |
| 6 R | 25 | 0.0 | 0.234 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.73 | 48.6 |
| Approach | 435 | 0.0 | 0.234 | 8.2 | NA | 0.0 | 0.0 | 0.00 | 0.67 | 48.9 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 16 | 0.0 | 0.040 | 9.9 | LOS A | 0.2 | 1.2 | 0.45 | 0.69 | 47.3 |
| 8 T | 25 | 0.0 | 0.040 | 8.7 | LOS A | 0.2 | 1.2 | 0.45 | 0.62 | 47.8 |
| Approach | 41 | 0.0 | 0.040 | 9.2 | LOS A | 0.2 | 1.2 | 0.45 | 0.65 | 47.6 |
| All Vehicles | 1257 | 0.0 | 0.411 | 5.8 | NA | 0.2 | 1.2 | 0.01 | 0.53 | 51.7 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Thursday PM
O'Connell St/ Dunlop St
Giveway / Yield (Two-Way)


Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Thursday PM
New St/ Factory St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
|  |  |  |  |  |  |  |  |  |  |  |
| 2 T | 76 | 0.0 | 0.055 | 0.1 | LOS A | 0.3 | 1.9 | 0.09 | 0.00 | 58.1 |
| 3 R | 28 | 0.0 | 0.055 | 8.5 | LOS A | 0.3 | 1.9 | 0.09 | 0.93 | 48.6 |
| Approach | 104 | 0.0 | 0.055 | 2.4 | NA | 0.3 | 1.9 | 0.09 | 0.25 | 55.1 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 6 | 0.0 | 0.005 | 8.3 | LOS A | 0.0 | 0.1 | 0.07 | 0.63 | 48.6 |
| $6 \quad \mathrm{R}$ | 1 | 0.0 | 0.005 | 8.6 | LOS A | 0.0 | 0.1 | 0.07 | 0.70 | 48.4 |
| Approach | 7 | 0.0 | 0.005 | 8.3 | LOS A | 0.0 | 0.1 | 0.07 | 0.64 | 48.6 |
| North: New Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 5 | 0.0 | 0.013 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.95 | 49.0 |
| 8 T | 19 | 0.0 | 0.013 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 24 | 0.0 | 0.013 | 1.8 | NA | 0.0 | 0.0 | 0.00 | 0.21 | 57.2 |
| All Vehicles | 136 | 0.0 | 0.055 | 2.6 | NA | 0.3 | 1.9 | 0.07 | 0.27 | 55.1 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Thursday PM
O'Connell St/ Factory St
Stop (Two-Way)

| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| South: O'Connell Street 0.0 |  |  |  |  |  |  |  |  |  |  |
| 1 L | 5 | 0.0 | 0.372 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.09 | 49.0 |
| 2 T | 719 | 0.0 | 0.372 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 724 | 0.0 | 0.372 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 59.9 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 28 | 0.0 | 0.031 | 12.6 | LOS A | 0.1 | 0.7 | 0.38 | 0.90 | 45.2 |
| Approach | 28 | 0.0 | 0.031 | 12.6 | LOS A | 0.1 | 0.7 | 0.38 | 0.90 | 45.2 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 14 | 0.0 | 0.223 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.07 | 49.0 |
| 8 T | 421 | 0.0 | 0.223 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 435 | 0.0 | 0.223 | 0.3 | NA | 0.0 | 0.0 | 0.00 | 0.03 | 59.6 |
| West: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 38 | 0.0 | 0.060 | 15.1 | LOS B | 0.2 | 1.6 | 0.59 | 0.95 | 43.4 |
| Approach | 38 | 0.0 | 0.060 | 15.1 | LOS B | 0.2 | 1.6 | 0.59 | 0.95 | 43.4 |
| All Vehicles | 1225 | 0.0 | 0.372 | 0.9 | NA | 0.2 | 1.6 | 0.03 | 0.07 | 58.7 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

Processed: Tuesday, 26 August 2014 3:12:41 PM
SIDRA INTERSECTION 5.1.13.2093

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Project: \lgta-syd-ss1\project_files\14S1000-1099\14S1091200 PNUR - Rezoning\ModellingISIDRA
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8000056, GTA CONSULTANTS, ENTERPRISE

SIDRA
INTERSECTION

14S1091200 PNUR
Existing Thursday PM
O'Connell St/ Board St/ Property Access
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 2 T | 4 | 0.0 | 0.216 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 3 R | 398 | 0.0 | 0.216 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.70 | 48.6 |
| Approach | 402 | 0.0 | 0.216 | 8.4 | NA | 0.0 | 0.0 | 0.00 | 0.70 | 48.7 |
| East: Board Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 21 | 0.0 | 0.012 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.67 | 49.0 |
| 6 R | 1 | 0.0 | 0.012 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.73 | 48.6 |
| Approach | 22 | 0.0 | 0.012 | 8.2 | NA | 0.0 | 0.0 | 0.00 | 0.67 | 48.9 |
| North: Access Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 6 | 0.0 | 0.010 | 8.9 | LOS A | 0.0 | 0.3 | 0.07 | 0.69 | 48.2 |
| 8 T | 6 | 0.0 | 0.010 | 7.6 | LOS A | 0.0 | 0.3 | 0.07 | 0.55 | 49.5 |
| Approach | 13 | 0.0 | 0.010 | 8.3 | LOS A | 0.0 | 0.3 | 0.07 | 0.62 | 48.9 |
| All Vehicles | 437 | 0.0 | 0.216 | 8.3 | NA | 0.0 | 0.3 | 0.00 | 0.69 | 48.7 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Thursday PM
New St/ Greenup Drive
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: New Street 0 |  |  |  |  |  |  |  |  |  |  |
| 1 L | 58 | 0.0 | 0.042 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.74 | 49.0 |
| 2 T | 21 | 0.0 | 0.042 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 79 | 0.0 | 0.042 | 6.0 | NA | 0.0 | 0.0 | 0.00 | 0.54 | 51.5 |
| North: New Street |  |  |  |  |  |  |  |  |  |  |
| 8 T | 16 | 0.0 | 0.016 | 0.2 | LOS A | 0.1 | 0.5 | 0.17 | 0.00 | 56.1 |
| 9 R | 14 | 0.0 | 0.016 | 8.7 | LOS A | 0.1 | 0.5 | 0.17 | 0.79 | 48.4 |
| Approach | 29 | 0.0 | 0.016 | 4.1 | NA | 0.1 | 0.5 | 0.17 | 0.37 | 52.3 |
| West: Greenup Drive |  |  |  |  |  |  |  |  |  |  |
| 10 L | 86 | 0.0 | 0.187 | 8.5 | LOS A | 0.8 | 5.6 | 0.17 | 0.60 | 48.2 |
| 12 R | 164 | 0.0 | 0.187 | 8.7 | LOS A | 0.8 | 5.6 | 0.17 | 0.67 | 48.0 |
| Approach | 251 | 0.0 | 0.187 | 8.6 | LOS A | 0.8 | 5.6 | 0.17 | 0.64 | 48.1 |
| All Vehicles | 359 | 0.0 | 0.187 | 7.7 | NA | 0.8 | 5.6 | 0.13 | 0.60 | 49.1 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Saturday
O'Connell St/ Fennell St
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 46 | 0.0 | 0.146 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.98 | 49.0 |
| 2 | T | 519 | 0.0 | 0.146 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 3 | R | 11 | 0.0 | 0.015 | 11.5 | LOS A | 0.0 | 0.3 | 0.50 | 0.71 | 45.5 |
| Approac |  | 576 | 0.0 | 0.146 | 0.9 | NA | 0.0 | 0.3 | 0.01 | 0.09 | 58.6 |
| East: Fennell Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 13 | 0.0 | 0.099 | 25.5 | LOS B | 0.3 | 2.2 | 0.76 | 0.97 | 36.1 |
| 5 | T | 7 | 0.0 | 0.099 | 26.3 | LOS B | 0.3 | 2.2 | 0.76 | 1.00 | 35.8 |
| 6 | R | 2 | 0.0 | 0.099 | 25.6 | LOS B | 0.3 | 2.2 | 0.76 | 0.99 | 36.1 |
| Approac |  | 22 | 0.0 | 0.099 | 25.8 | LOS B | 0.3 | 2.2 | 0.76 | 0.98 | 36.0 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 9 | 0.0 | 0.186 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.07 | 49.0 |
| 8 | T | 717 | 0.0 | 0.186 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 9 | R | 23 | 0.0 | 0.026 | 10.4 | LOS A | 0.1 | 0.6 | 0.40 | 0.68 | 46.7 |
| Approac |  | 749 | 0.0 | 0.186 | 0.4 | NA | 0.1 | 0.6 | 0.01 | 0.03 | 59.3 |
| West: Fennell Street |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 27 | 0.0 | 0.677 | 58.3 | LOS E | 3.3 | 23.3 | 0.90 | 1.28 | 23.5 |
| 11 | T | 16 | 0.0 | 0.677 | 59.1 | LOS E | 3.3 | 23.3 | 0.90 | 1.19 | 23.4 |
| 12 | R | 55 | 0.0 | 0.677 | 58.4 | LOS E | 3.3 | 23.3 | 0.90 | 1.18 | 23.5 |
| Approach |  | 98 | 0.0 | 0.677 | 58.5 | LOS E | 3.3 | 23.3 | 0.90 | 1.21 | 23.5 |
| All Vehicles |  | 1445 | 0.0 | 0.677 | 4.9 | NA | 3.3 | 23.3 | 0.08 | 0.15 | 53.0 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Saturday
Marsden St/ Market St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
|  |  |  |  |  |  |  |  |  |  |  |
| 2 T | 492 | 0.0 | 0.396 | 1.8 | LOS A | 3.3 | 23.1 | 0.47 | 0.00 | 51.0 |
| 3 R | 198 | 0.0 | 0.396 | 10.3 | LOS A | 3.3 | 23.1 | 0.47 | 0.85 | 48.3 |
| Approach | 689 | 0.0 | 0.396 | 4.2 | NA | 3.3 | 23.1 | 0.47 | 0.24 | 50.2 |
| East: Market Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 357 | 0.0 | 0.294 | 9.6 | LOS A | 1.6 | 10.9 | 0.43 | 0.68 | 47.1 |
| 6 R | 7 | 0.0 | 0.294 | 9.9 | LOS A | 1.6 | 10.9 | 0.43 | 0.84 | 47.2 |
| Approach | 364 | 0.0 | 0.294 | 9.6 | LOS A | 1.6 | 10.9 | 0.43 | 0.68 | 47.1 |
| North: Marsden Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 33 | 0.0 | 0.163 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.02 | 49.0 |
| 8 T | 283 | 0.0 | 0.163 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 316 | 0.0 | 0.163 | 0.8 | NA | 0.0 | 0.0 | 0.00 | 0.11 | 58.6 |
| All Vehicles | 1369 | 0.0 | 0.396 | 4.9 | NA | 3.3 | 23.1 | 0.35 | 0.33 | 51.0 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Saturday
Church St/ Market St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Church Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 4 | 0.0 | 0.005 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.84 | 49.0 |
| 2 T | 5 | 0.0 | 0.005 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 9 | 0.0 | 0.005 | 3.6 | NA | 0.0 | 0.0 | 0.00 | 0.38 | 54.5 |
| North: Church Street |  |  |  |  |  |  |  |  |  |  |
| 8 T | 26 | 0.0 | 0.013 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 9 R | 354 | 0.0 | 0.303 | 8.9 | LOS A | 1.0 | 7.1 | 0.57 | 0.40 | 46.5 |
| Approach | 380 | 0.0 | 0.303 | 8.2 | NA | 1.0 | 7.1 | 0.53 | 0.37 | 47.2 |
| West: Market Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 220 | 0.0 | 0.142 | 8.3 | LOS A | 0.7 | 4.9 | 0.03 | 0.65 | 48.8 |
| 12 R | 5 | 0.0 | 0.142 | 8.5 | LOS A | 0.7 | 4.9 | 0.03 | 0.73 | 48.5 |
| Approach | 225 | 0.0 | 0.142 | 8.3 | LOS A | 0.7 | 4.9 | 0.03 | 0.65 | 48.8 |
| All Vehicles | 615 | 0.0 | 0.303 | 8.2 | NA | 1.0 | 7.1 | 0.34 | 0.47 | 47.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Saturday
Church St/ Board St/ Seville St
Giveway / Yield (Two-Way)

Movement Performance - Vehicles

| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| South: Church Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 18 | 0.0 | 0.286 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.07 | 49.0 |
| 2 T | 1098 | 0.0 | 0.286 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 1116 | 0.0 | 0.286 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.02 | 59.8 |
| East: Seville Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 25 | 0.0 | 0.081 | 18.7 | LOS B | 0.3 | 1.8 | 0.78 | 0.93 | 39.6 |
| Approach | 25 | 0.0 | 0.081 | 18.7 | LOS B | 0.3 | 1.8 | 0.78 | 0.93 | 39.6 |
| North: Church Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 32 | 0.0 | 0.342 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.06 | 49.0 |
| 8 T | 1301 | 0.0 | 0.342 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 1333 | 0.0 | 0.342 | 0.2 | NA | 0.0 | 0.0 | 0.00 | 0.03 | 59.7 |
| West: Board Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 340 | 0.0 | 0.758 | 25.5 | LOS B | 5.8 | 40.7 | 0.89 | 1.30 | 35.2 |
| Approach | 340 | 0.0 | 0.758 | 25.5 | LOS B | 5.8 | 40.7 | 0.89 | 1.30 | 35.2 |
| All Vehicles | 2814 | 0.0 | 0.758 | 3.4 | NA | 5.8 | 40.7 | 0.11 | 0.18 | 54.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

Processed: Tuesday, 26 August 2014 1:07:20 PM
SIDRA INTERSECTION 5.1.13.2093

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Project: \lgta-syd-ss1\project files\14S1000-1099\14S1091200 PNUR - Rezoning\ModellingISIDRA
14S1091200sid_SIDRA Existing Saturday.sip
8000056, GTA CONSULTANTS, ENTERPRISE

SIDRA
INTERSECTION

14S1091200 PNUR
Existing Saturday
O'Connell St/ Barney St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 2 T | 287 | 0.0 | 0.222 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 3 R | 139 | 0.0 | 0.222 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.96 | 48.6 |
| Approach | 426 | 0.0 | 0.222 | 2.8 | NA | 0.0 | 0.0 | 0.00 | 0.31 | 55.8 |
| East: Barney Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 505 | 0.0 | 0.296 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.66 | 49.0 |
| 6 R | 44 | 0.0 | 0.296 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.73 | 48.6 |
| Approach | 549 | 0.0 | 0.296 | 8.2 | NA | 0.0 | 0.0 | 0.00 | 0.67 | 48.9 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 9 | 0.0 | 0.030 | 10.1 | LOS A | 0.1 | 0.9 | 0.36 | 0.60 | 47.1 |
| 8 T | 20 | 0.0 | 0.030 | 8.9 | LOS A | 0.1 | 0.9 | 0.36 | 0.63 | 48.1 |
| Approach | 29 | 0.0 | 0.030 | 9.3 | LOS A | 0.1 | 0.9 | 0.36 | 0.62 | 47.7 |
| All Vehicles | 1005 | 0.0 | 0.296 | 5.9 | NA | 0.1 | 0.9 | 0.01 | 0.52 | 51.6 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Saturday
O'Connell St/ Dunlop St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. <br> Satn <br> v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street South |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 5 | 0.0 | 0.221 | 10.9 | LOS A | 1.8 | 12.5 | 0.55 | 0.47 | 48.6 |
| 2 | T | 402 | 0.0 | 0.221 | 2.7 | LOS A | 1.8 | 12.5 | 0.55 | 0.00 | 50.5 |
| 3 | R | 13 | 0.0 | 0.221 | 11.1 | LOS A | 1.8 | 12.5 | 0.55 | 1.01 | 48.5 |
| Approa |  | 420 | 0.0 | 0.221 | 3.0 | NA | 1.8 | 12.5 | 0.55 | 0.04 | 50.4 |
| East: Dunlop Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 9 | 0.0 | 0.017 | 11.2 | LOS A | 0.1 | 0.4 | 0.52 | 0.69 | 45.9 |
| 5 | T | 2 | 0.0 | 0.017 | 9.9 | LOS A | 0.1 | 0.4 | 0.52 | 0.73 | 46.7 |
| 6 | R | 1 | 0.0 | 0.017 | 11.4 | LOS A | 0.1 | 0.4 | 0.52 | 0.83 | 45.8 |
| Approa |  | 13 | 0.0 | 0.017 | 11.0 | LOS A | 0.1 | 0.4 | 0.52 | 0.71 | 46.0 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 5 | 0.0 | 0.277 | 10.3 | LOS A | 2.5 | 17.3 | 0.58 | 0.44 | 49.3 |
| 8 | T | 511 | 0.0 | 0.277 | 2.1 | LOS A | 2.5 | 17.3 | 0.58 | 0.00 | 50.1 |
| 9 | R | 16 | 0.0 | 0.277 | 10.5 | LOS A | 2.5 | 17.3 | 0.58 | 0.95 | 49.3 |
| Approa |  | 532 | 0.0 | 0.277 | 2.4 | NA | 2.5 | 17.3 | 0.58 | 0.03 | 50.1 |
| West: Dunlop Street |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 20 | 0.0 | 0.047 | 11.6 | LOS A | 0.2 | 1.2 | 0.50 | 0.68 | 45.5 |
| 11 | T | 6 | 0.0 | 0.047 | 10.3 | LOS A | 0.2 | 1.2 | 0.50 | 0.77 | 46.3 |
| 12 | R | 6 | 0.0 | 0.047 | 11.9 | LOS A | 0.2 | 1.2 | 0.50 | 0.87 | 45.4 |
| Approach |  | 33 | 0.0 | 0.047 | 11.4 | LOS A | 0.2 | 1.2 | 0.50 | 0.73 | 45.6 |
| All Vehicles |  | 997 | 0.0 | 0.277 | 3.1 | NA | 2.5 | 17.3 | 0.57 | 0.07 | 50.0 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Saturday
New St/ Factory St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
|  |  |  |  |  |  |  |  |  |  |  |
| 2 T | 25 | 0.0 | 0.017 | 0.0 | LOS A | 0.1 | 0.6 | 0.07 | 0.00 | 58.5 |
| 3 R | 6 | 0.0 | 0.017 | 8.5 | LOS A | 0.1 | 0.6 | 0.07 | 0.98 | 48.6 |
| Approach | 32 | 0.0 | 0.017 | 1.7 | NA | 0.1 | 0.6 | 0.07 | 0.20 | 56.2 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 6 | 0.0 | 0.006 | 8.3 | LOS A | 0.0 | 0.1 | 0.07 | 0.63 | 48.6 |
| 6 R | 2 | 0.0 | 0.006 | 8.5 | LOS A | 0.0 | 0.1 | 0.07 | 0.69 | 48.4 |
| Approach | 8 | 0.0 | 0.006 | 8.3 | LOS A | 0.0 | 0.1 | 0.07 | 0.64 | 48.6 |
| North: New Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 1 | 0.0 | 0.009 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.05 | 49.0 |
| 8 T | 17 | 0.0 | 0.009 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 18 | 0.0 | 0.009 | 0.5 | NA | 0.0 | 0.0 | 0.00 | 0.06 | 59.2 |
| All Vehicles | 58 | 0.0 | 0.017 | 2.3 | NA | 0.1 | 0.6 | 0.05 | 0.22 | 55.8 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Saturday
O'Connell St/ Factory St
Stop (Two-Way)

| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |
| 1 L | 11 | 0.0 | 0.217 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.07 | 49.0 |
| 2 T | 412 | 0.0 | 0.217 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 422 | 0.0 | 0.217 | 0.2 | NA | 0.0 | 0.0 | 0.00 | 0.03 | 59.7 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 41 | 0.0 | 0.052 | 13.4 | LOS A | 0.2 | 1.1 | 0.46 | 0.92 | 44.7 |
| Approach | 41 | 0.0 | 0.052 | 13.4 | LOS A | 0.2 | 1.1 | 0.46 | 0.92 | 44.7 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 11 | 0.0 | 0.271 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.08 | 49.0 |
| 8 T | 518 | 0.0 | 0.271 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 528 | 0.0 | 0.271 | 0.2 | NA | 0.0 | 0.0 | 0.00 | 0.02 | 59.7 |
| West: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 11 | 0.0 | 0.011 | 12.4 | LOS A | 0.0 | 0.3 | 0.44 | 0.83 | 45.5 |
| Approach | 11 | 0.0 | 0.011 | 12.4 | LOS A | 0.0 | 0.3 | 0.44 | 0.83 | 45.5 |
| All Vehicles | 1002 | 0.0 | 0.271 | 0.9 | NA | 0.2 | 1.1 | 0.02 | 0.07 | 58.7 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

Processed: Tuesday, 26 August 2014 1:07:21 PM
SIDRA INTERSECTION 5.1.13.2093

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Project: \lgta-syd-ss1\project files\14S1000-1099\14S1091200 PNUR - Rezoning\ModellingISIDRA
114S1091200sid_SIDRA Existing Saturday.sip
8000056, GTA CONSULTANTS, ENTERPRISE

14S1091200 PNUR
Existing Saturday
O'Connell St/ Board St/ Property Access
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue <br> Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 2 T | 12 | 0.0 | 0.181 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 3 R | 325 | 0.0 | 0.181 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.71 | 48.6 |
| Approach | 337 | 0.0 | 0.181 | 8.2 | NA | 0.0 | 0.0 | 0.00 | 0.69 | 48.9 |
| East: Board Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 23 | 0.0 | 0.015 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.66 | 49.0 |
| 6 R | 4 | 0.0 | 0.015 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.73 | 48.6 |
| Approach | 27 | 0.0 | 0.015 | 8.2 | NA | 0.0 | 0.0 | 0.00 | 0.67 | 48.9 |
| North: Access Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 13 | 0.0 | 0.012 | 9.1 | LOS A | 0.0 | 0.3 | 0.17 | 0.65 | 48.0 |
| 8 T | 2 | 0.0 | 0.012 | 7.8 | LOS A | 0.0 | 0.3 | 0.17 | 0.50 | 49.2 |
| Approach | 15 | 0.0 | 0.012 | 8.9 | LOS A | 0.0 | 0.3 | 0.17 | 0.63 | 48.2 |
| All Vehicles | 379 | 0.0 | 0.181 | 8.2 | NA | 0.0 | 0.3 | 0.01 | 0.68 | 48.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Existing Saturday
New St/ Greenup Drive
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: New Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 20 | 0.0 | 0.030 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.88 | 49.0 |
| 2 T | 37 | 0.0 | 0.030 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 57 | 0.0 | 0.030 | 2.9 | NA | 0.0 | 0.0 | 0.00 | 0.31 | 55.6 |
| North: New Street |  |  |  |  |  |  |  |  |  |  |
| 8 T | 29 | 0.0 | 0.019 | 0.2 | LOS A | 0.1 | 0.7 | 0.15 | 0.00 | 57.0 |
| 9 R | 6 | 0.0 | 0.019 | 8.6 | LOS A | 0.1 | 0.7 | 0.15 | 0.95 | 48.7 |
| Approach | 36 | 0.0 | 0.019 | 1.6 | NA | 0.1 | 0.7 | 0.15 | 0.17 | 55.4 |
| West: Greenup Drive |  |  |  |  |  |  |  |  |  |  |
| 10 L | 9 | 0.0 | 0.022 | 8.4 | LOS A | 0.1 | 0.6 | 0.14 | 0.59 | 48.3 |
| 12 R | 20 | 0.0 | 0.022 | 8.7 | LOS A | 0.1 | 0.6 | 0.14 | 0.66 | 48.1 |
| Approach | 29 | 0.0 | 0.022 | 8.6 | LOS A | 0.1 | 0.6 | 0.14 | 0.64 | 48.1 |
| All Vehicles | 122 | 0.0 | 0.030 | 3.9 | NA | 0.1 | 0.7 | 0.08 | 0.35 | 53.5 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

Appendix B

## B. 2 Future Conditions

14S1091200 PNUR
Post Development Thursday AM
O'Connell St/ Fennell St
Stop (Two-Way)


Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday AM
Marsden St/ Market St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Marsden Street |  |  |  |  |  |  |  |  |  |  |
| 2 T | 358 | 0.0 | 0.328 | 1.7 | LOS A | 2.4 | 16.7 | 0.44 | 0.00 | 51.3 |
| 3 R | 196 | 0.0 | 0.328 | 10.1 | LOS A | 2.4 | 16.7 | 0.44 | 0.84 | 48.2 |
| Approach | 554 | 0.0 | 0.328 | 4.7 | NA | 2.4 | 16.7 | 0.44 | 0.30 | 50.2 |
| East: Market Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 356 | 0.0 | 0.287 | 9.6 | LOS A | 1.5 | 10.6 | 0.44 | 0.68 | 47.1 |
| 6 R | 3 | 0.0 | 0.287 | 9.8 | LOSA | 1.5 | 10.6 | 0.44 | 0.83 | 47.2 |
| Approach | 359 | 0.0 | 0.287 | 9.6 | LOS A | 1.5 | 10.6 | 0.44 | 0.68 | 47.1 |
| North: Marsden Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 34 | 0.0 | 0.168 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.02 | 49.0 |
| 8 T | 293 | 0.0 | 0.168 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 326 | 0.0 | 0.168 | 0.8 | NA | 0.0 | 0.0 | 0.00 | 0.11 | 58.6 |
| All Vehicles | 1239 | 0.0 | 0.328 | 5.1 | NA | 2.4 | 16.7 | 0.32 | 0.36 | 51.1 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday AM
Church St/ Market St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Church Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 21 | 0.0 | 0.024 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.84 | 49.0 |
| 2 T | 25 | 0.0 | 0.024 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 46 | 0.0 | 0.024 | 3.7 | NA | 0.0 | 0.0 | 0.00 | 0.38 | 54.4 |
| North: Church Street |  |  |  |  |  |  |  |  |  |  |
| 8 T | 18 | 0.0 | 0.009 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 9 R | 335 | 0.0 | 0.289 | 8.8 | LOS A | 1.0 | 6.8 | 0.29 | 0.59 | 47.5 |
| Approach | 353 | 0.0 | 0.289 | 8.4 | NA | 1.0 | 6.8 | 0.27 | 0.56 | 48.0 |
| West: Market Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 207 | 0.0 | 0.138 | 8.4 | LOS A | 0.7 | 4.7 | 0.10 | 0.63 | 48.5 |
| 12 R | 6 | 0.0 | 0.138 | 8.6 | LOS A | 0.7 | 4.7 | 0.10 | 0.73 | 48.3 |
| Approach | 214 | 0.0 | 0.138 | 8.4 | LOS A | 0.7 | 4.7 | 0.10 | 0.63 | 48.5 |
| All Vehicles | 613 | 0.0 | 0.289 | 8.0 | NA | 1.0 | 6.8 | 0.19 | 0.57 | 48.6 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday AM
Church St/ Board St/ Seville St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Church Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 13 | 0.0 | 0.259 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.07 | 49.0 |
| 2 T | 996 | 0.0 | 0.259 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 1008 | 0.0 | 0.259 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 59.8 |
| East: Seville Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 3 | 0.0 | 0.059 | 69.2 | LOS E | 0.2 | 1.1 | 0.96 | 0.99 | 20.6 |
| Approach | 3 | 0.0 | 0.059 | 69.2 | LOS E | 0.2 | 1.1 | 0.96 | 0.99 | 20.6 |
| North: Church Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 22 | 0.0 | 0.568 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.08 | 49.0 |
| 8 T | 2192 | 0.0 | 0.568 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 2214 | 0.0 | 0.568 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 59.9 |
| West: Board Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 389 | 0.0 | 0.763 | 23.5 | LOS B | 6.4 | 45.0 | 0.87 | 1.32 | 36.4 |
| Approach | 389 | 0.0 | 0.763 | 23.5 | LOS B | 6.4 | 45.0 | 0.87 | 1.32 | 36.4 |
| All Vehicles | 3615 | 0.0 | 0.763 | 2.7 | NA | 6.4 | 45.0 | 0.09 | 0.15 | 55.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

Processed: Wednesday, 1 October 2014 10:06:30 AM
SIDRA INTERSECTION 5.1.13.2093
Project: P:\14S1000-1099\14S1091200 PNUR - RezoninglModellingISIDRAI14S1091200sid SIDRA Future
Thursday AM.sip
8000056, GTA CONSULTANTS, ENTERPRISE

14S1091200 PNUR
Post Development Thursday AM
O'Connell St/ Barney St
Giveway / Yield (Two-Way)

| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 37 | 0.0 | 0.280 | 9.0 | LOS A | 1.9 | 13.1 | 0.34 | 0.58 | 48.5 |
| 2 T | 318 | 0.0 | 0.280 | 0.8 | LOS A | 1.9 | 13.1 | 0.34 | 0.00 | 53.1 |
| 3 R | 121 | 0.0 | 0.280 | 9.3 | LOS A | 1.9 | 13.1 | 0.34 | 0.84 | 48.5 |
| Approach | 476 | 0.0 | 0.280 | 3.6 | NA | 1.9 | 13.1 | 0.34 | 0.26 | 51.5 |
| East: Barney Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 852 | 0.0 | 0.771 | 12.5 | LOS A | 11.9 | 83.4 | 0.37 | 0.75 | 44.6 |
| 5 T | 65 | 0.0 | 0.771 | 11.2 | LOS A | 11.9 | 83.4 | 0.37 | 0.80 | 45.5 |
| 6 R | 48 | 0.0 | 0.771 | 12.7 | LOS A | 11.9 | 83.4 | 0.37 | 0.91 | 44.5 |
| Approach | 965 | 0.0 | 0.771 | 12.4 | LOS A | 11.9 | 83.4 | 0.37 | 0.76 | 44.7 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 6 | 0.0 | 0.080 | 9.9 | LOS A | 0.6 | 4.4 | 0.49 | 0.54 | 49.2 |
| 8 T | 147 | 0.0 | 0.080 | 1.7 | LOS A | 0.6 | 4.4 | 0.49 | 0.00 | 51.4 |
| 9 R | 1 | 0.0 | 0.080 | 10.1 | LOS A | 0.6 | 4.4 | 0.49 | 0.95 | 49.3 |
| Approach | 155 | 0.0 | 0.080 | 2.1 | NA | 0.6 | 4.4 | 0.49 | 0.03 | 51.3 |
| West: New Road From Development |  |  |  |  |  |  |  |  |  |  |
| 10 L | 55 | 0.0 | 1.059 | 121.7 | LOS F | 22.6 | 158.3 | 1.00 | 3.00 | 13.8 |
| 11 T | 23 | 0.0 | 1.059 | 120.4 | LOS F | 22.6 | 158.3 | 1.00 | 2.91 | 13.8 |
| 12 R | 217 | 0.0 | 1.059 | 121.9 | LOS F | 22.6 | 158.3 | 1.00 | 2.41 | 13.7 |
| Approach | 295 | 0.0 | 1.059 | 121.7 | LOS F | 22.6 | 158.3 | 1.00 | 2.56 | 13.7 |
| All Vehicles | 1891 | 0.0 | 1.059 | 26.4 | NA | 22.6 | 158.3 | 0.47 | 0.86 | 34.2 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday AM
O'Connell St/ Dunlop St
Giveway / Yield (Two-Way)


Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday AM
New St/ Factory St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: New Street 0 en men |  |  |  |  |  |  |  |  |  |  |
| 2 T | 23 | 0.0 | 0.065 | 0.6 | LOS A | 0.3 | 2.2 | 0.29 | 0.00 | 53.2 |
| 3 R | 82 | 0.0 | 0.065 | 9.0 | LOS A | 0.3 | 2.2 | 0.29 | 0.69 | 47.8 |
| Approach | 105 | 0.0 | 0.065 | 7.2 | NA | 0.3 | 2.2 | 0.29 | 0.54 | 48.9 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 39 | 0.0 | 0.048 | 8.8 | LOS A | 0.2 | 1.3 | 0.24 | 0.60 | 47.8 |
| 6 R | 23 | 0.0 | 0.048 | 9.0 | LOS A | 0.2 | 1.3 | 0.24 | 0.69 | 47.8 |
| Approach | 62 | 0.0 | 0.048 | 8.9 | LOS A | 0.2 | 1.3 | 0.24 | 0.64 | 47.8 |
| North: New Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 105 | 0.0 | 0.096 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.79 | 49.0 |
| 8 T | 76 | 0.0 | 0.096 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 181 | 0.0 | 0.096 | 4.8 | NA | 0.0 | 0.0 | 0.00 | 0.46 | 53.0 |
| All Vehicles | 348 | 0.0 | 0.096 | 6.2 | NA | 0.3 | 2.2 | 0.13 | 0.52 | 50.8 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday AM
O'Connell St/ Factory St
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 29 | 0.0 | 0.269 | 26.0 | LOS B | 6.3 | 44.0 | 1.00 | 0.00 | 37.4 |
| 2 T | 487 | 0.0 | 0.269 | 17.8 | LOS B | 6.3 | 44.0 | 1.00 | 0.00 | 37.6 |
| 3 R | 1 | 0.0 | 0.269 | 26.3 | LOS B | 6.3 | 44.0 | 1.00 | 1.06 | 37.5 |
| Approach | 518 | 0.0 | 0.269 | 18.3 | NA | 6.3 | 44.0 | 1.00 | 0.00 | 37.6 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 42 | 0.0 | 0.297 | 26.4 | LOS B | 0.8 | 5.9 | 0.87 | 1.03 | 35.7 |
| 5 T | 31 | 0.0 | 0.297 | 26.0 | LOS B | 0.8 | 5.9 | 0.87 | 1.03 | 35.9 |
| $6 \quad \mathrm{R}$ | 1 | 0.0 | 0.297 | 26.2 | LOS B | 0.8 | 5.9 | 0.87 | 1.03 | 35.9 |
| Approach | 74 | 0.0 | 0.297 | 26.3 | LOS B | 0.8 | 5.9 | 0.87 | 1.03 | 35.8 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 262 | 0.0 | 0.616 | 15.6 | LOS B | 17.6 | 123.0 | 1.00 | 0.00 | 44.8 |
| 8 T | 923 | 0.0 | 0.616 | 7.5 | LOS A | 17.6 | 123.0 | 1.00 | 0.00 | 44.0 |
| 9 R | 1 | 0.0 | 0.616 | 15.9 | LOS B | 17.6 | 123.0 | 1.00 | 1.17 | 44.9 |
| Approach | 1186 | 0.0 | 0.616 | 9.3 | NA | 17.6 | 123.0 | 1.00 | 0.00 | 44.2 |
| West: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 14 | 0.0 | 1.294 | 334.9 | LOS F | 32.6 | 228.4 | 1.00 | 4.38 | 5.9 |
| 11 T | 149 | 0.0 | 1.294 | 334.4 | LOS F | 32.6 | 228.4 | 1.00 | 2.98 | 5.9 |
| 12 R | 25 | 0.0 | 1.294 | 334.6 | LOS F | 32.6 | 228.4 | 1.00 | 3.07 | 5.9 |
| Approach | 188 | 0.0 | 1.294 | 334.5 | LOS F | 32.6 | 228.4 | 1.00 | 3.10 | 5.9 |
| All Vehicles | 1966 | 0.0 | 1.294 | 43.4 | NA | 32.6 | 228.4 | 1.00 | 0.34 | 26.4 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday AM
O'Connell St/ Board St/ Property Access
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \text { v/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
|  |  |  |  |  |  |  |  |  |  |  |
| 2 T | 23 | 0.0 | 0.216 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 3 R | 380 | 0.0 | 0.216 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.72 | 48.6 |
| Approach | 403 | 0.0 | 0.216 | 8.0 | NA | 0.0 | 0.0 | 0.00 | 0.68 | 49.1 |
| East: Board Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 15 | 0.0 | 0.012 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.66 | 49.0 |
| $6 \quad \mathrm{R}$ | 7 | 0.0 | 0.012 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.72 | 48.6 |
| Approach | 22 | 0.0 | 0.012 | 8.3 | NA | 0.0 | 0.0 | 0.00 | 0.68 | 48.8 |
| North: Access Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 11 | 0.0 | 0.011 | 9.2 | LOS A | 0.0 | 0.3 | 0.11 | 0.67 | 47.9 |
| 8 T | 3 | 0.0 | 0.011 | 7.9 | LOS A | 0.0 | 0.3 | 0.11 | 0.52 | 49.2 |
| Approach | 14 | 0.0 | 0.011 | 8.9 | LOS A | 0.0 | 0.3 | 0.11 | 0.64 | 48.2 |
| All Vehicles | 439 | 0.0 | 0.216 | 8.0 | NA | 0.0 | 0.3 | 0.00 | 0.67 | 49.1 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday AM
Fleet St/ Greenup Drive
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: New Street |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 156 | 0.0 | 0.193 | 8.5 | LOS A | 1.1 | 7.8 | 0.26 | 0.55 | 48.1 |
| 2 | T | 98 | 0.0 | 0.193 | 0.3 | LOSA | 1.1 | 7.8 | 0.26 | 0.00 | 54.1 |
| 3 | R | 99 | 0.0 | 0.193 | 8.7 | LOS A | 1.1 | 7.8 | 0.26 | 0.70 | 48.0 |
| Approa |  | 353 | 0.0 | 0.193 | 6.3 | NA | 1.1 | 7.8 | 0.26 | 0.44 | 49.6 |
| East: Albert Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 25 | 0.0 | 0.043 | 9.3 | LOS A | 0.2 | 1.1 | 0.17 | 0.63 | 47.8 |
| 5 | T | 17 | 0.0 | 0.043 | 8.1 | LOSA | 0.2 | 1.1 | 0.17 | 0.60 | 48.9 |
| 6 | R | 4 | 0.0 | 0.043 | 9.6 | LOSA | 0.2 | 1.1 | 0.17 | 0.76 | 47.5 |
| Approa |  | 46 | 0.0 | 0.043 | 8.9 | LOSA | 0.2 | 1.1 | 0.17 | 0.63 | 48.2 |
| North: New Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 25 | 0.0 | 0.092 | 9.1 | LOS A | 0.5 | 3.4 | 0.38 | 0.46 | 47.8 |
| 8 | T | 54 | 0.0 | 0.092 | 0.9 | LOS A | 0.5 | 3.4 | 0.38 | 0.00 | 51.6 |
| 9 | R | 73 | 0.0 | 0.092 | 9.3 | LOS A | 0.5 | 3.4 | 0.38 | 0.74 | 47.9 |
| Approa |  | 152 | 0.0 | 0.092 | 6.3 | NA | 0.5 | 3.4 | 0.38 | 0.43 | 49.1 |
| West: Greenup Drive |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 15 | 0.0 | 0.162 | 10.3 | LOS A | 0.6 | 4.3 | 0.43 | 0.64 | 47.0 |
| 11 | T | 99 | 0.0 | 0.162 | 9.0 | LOSA | 0.6 | 4.3 | 0.43 | 0.69 | 47.9 |
| 12 | R | 27 | 0.0 | 0.162 | 10.5 | LOSA | 0.6 | 4.3 | 0.43 | 0.82 | 46.8 |
| Approach |  | 141 | 0.0 | 0.162 | 9.4 | LOS A | 0.6 | 4.3 | 0.43 | 0.71 | 47.6 |
| All Vehicles |  | 692 | 0.0 | 0.193 | 7.1 | NA | 1.1 | 7.8 | 0.31 | 0.50 | 49.0 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday PM
O'Connell St/ Fennell St
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 277 | 0.0 | 0.370 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.87 | 49.0 |
| 2 T | 1152 | 0.0 | 0.370 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 3 R | 16 | 0.0 | 0.023 | 11.7 | LOS A | 0.1 | 0.5 | 0.52 | 0.73 | 45.3 |
| Approach | 1444 | 0.0 | 0.370 | 1.7 | NA | 0.1 | 0.5 | 0.01 | 0.17 | 57.3 |
| East: Fennell Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 35 | 0.0 | 0.375 | 51.1 | LOS D | 1.2 | 8.6 | 0.86 | 1.07 | 25.4 |
| 5 T | 1 | 0.0 | 0.375 | 51.8 | LOS D | 1.2 | 8.6 | 0.86 | 1.05 | 25.4 |
| 6 R | 6 | 0.0 | 0.375 | 51.1 | LOS D | 1.2 | 8.6 | 0.86 | 1.04 | 25.4 |
| Approach | 42 | 0.0 | 0.375 | 51.1 | LOS D | 1.2 | 8.6 | 0.86 | 1.07 | 25.4 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 14 | 0.0 | 0.192 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.07 | 49.0 |
| 8 T | 736 | 0.0 | 0.192 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 9 R | 23 | 0.0 | 0.071 | 18.9 | LOS B | 0.2 | 1.5 | 0.78 | 0.93 | 39.4 |
| Approach | 773 | 0.0 | 0.192 | 0.7 | NA | 0.2 | 1.5 | 0.02 | 0.05 | 58.8 |
| West: Fennell Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 60 | 0.0 | 3.572 | 2394.1 | LOS F | 120.2 | 841.4 | 1.00 | 4.60 | 0.9 |
| 11 T | 43 | 0.0 | 3.572 | 2394.9 | LOS F | 120.2 | 841.4 | 1.00 | 3.97 | 0.9 |
| 12 R | 142 | 0.0 | 3.572 | 2394.2 | LOS F | 120.2 | 841.4 | 1.00 | 3.89 | 0.9 |
| Approach | 245 | 0.0 | 3.572 | 2394.3 | LOS F | 120.2 | 841.4 | 1.00 | 4.08 | 0.9 |
| All Vehicles | 2504 | 0.0 | 3.572 | 236.6 | NA | 120.2 | 841.4 | 0.12 | 0.53 | 8.0 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday PM
Marsden St/ Market St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
|  |  |  |  |  |  |  |  |  |  |  |
| 2 T | 558 | 0.0 | 0.495 | 3.3 | LOS A | 6.4 | 45.1 | 0.59 | 0.00 | 49.0 |
| 3 R | 268 | 0.0 | 0.495 | 11.8 | LOS A | 6.4 | 45.1 | 0.59 | 0.93 | 47.0 |
| Approach | 826 | 0.0 | 0.495 | 6.1 | NA | 6.4 | 45.1 | 0.59 | 0.30 | 48.4 |
| East: Market Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 274 | 0.0 | 0.254 | 10.1 | LOS A | 1.2 | 8.7 | 0.47 | 0.70 | 46.9 |
| 6 R | 9 | 0.0 | 0.254 | 10.3 | LOS A | 1.2 | 8.7 | 0.47 | 0.86 | 46.8 |
| Approach | 283 | 0.0 | 0.254 | 10.1 | LOS A | 1.2 | 8.7 | 0.47 | 0.71 | 46.9 |
| North: Marsden Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 28 | 0.0 | 0.198 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.04 | 49.0 |
| 8 T | 357 | 0.0 | 0.198 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 385 | 0.0 | 0.198 | 0.6 | NA | 0.0 | 0.0 | 0.00 | 0.08 | 59.0 |
| All Vehicles | 1495 | 0.0 | 0.495 | 5.4 | NA | 6.4 | 45.1 | 0.41 | 0.32 | 50.4 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday PM
Church St/ Market St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Church Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 12 | 0.0 | 0.016 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.88 | 49.0 |
| 2 T | 20 | 0.0 | 0.016 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 32 | 0.0 | 0.016 | 3.0 | NA | 0.0 | 0.0 | 0.00 | 0.32 | 55.4 |
| North: Church Street |  |  |  |  |  |  |  |  |  |  |
| 8 T | 12 | 0.0 | 0.006 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 9 R | 269 | 0.0 | 0.233 | 8.7 | LOS A | 0.7 | 5.2 | 0.26 | 0.59 | 47.6 |
| Approach | 281 | 0.0 | 0.233 | 8.4 | NA | 0.7 | 5.2 | 0.25 | 0.56 | 48.0 |
| West: Market Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 286 | 0.0 | 0.183 | 8.3 | LOS A | 0.9 | 6.6 | 0.09 | 0.63 | 48.5 |
| 12 R | 4 | 0.0 | 0.183 | 8.6 | LOS A | 0.9 | 6.6 | 0.09 | 0.73 | 48.3 |
| Approach | 291 | 0.0 | 0.183 | 8.3 | LOS A | 0.9 | 6.6 | 0.09 | 0.63 | 48.5 |
| All Vehicles | 603 | 0.0 | 0.233 | 8.1 | NA | 0.9 | 6.6 | 0.16 | 0.58 | 48.6 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday PM
Church St/ Board St/ Seville St
Giveway / Yield (Two-Way)

| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| South: Church Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 5 | 0.0 | 0.528 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.09 | 49.0 |
| 2 T | 1024 | 0.0 | 0.528 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 1029 | 0.0 | 0.528 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 59.9 |
| East: Seville Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 9 | 0.0 | 0.036 | 20.8 | LOS B | 0.1 | 0.8 | 0.81 | 0.94 | 38.1 |
| Approach | 9 | 0.0 | 0.036 | 20.8 | LOS B | 0.1 | 0.8 | 0.81 | 0.94 | 38.1 |
| North: Church Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 14 | 0.0 | 0.370 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.08 | 49.0 |
| 8 T | 1429 | 0.0 | 0.370 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 1443 | 0.0 | 0.370 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 59.9 |
| West: Board Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 424 | 0.0 | 1.177 | 198.9 | LOS F | 50.1 | 350.6 | 1.00 | 3.96 | 9.2 |
| Approach | 424 | 0.0 | 1.177 | 198.9 | LOS F | 50.1 | 350.6 | 1.00 | 3.96 | 9.2 |
| All Vehicles | 2906 | 0.0 | 1.177 | 29.2 | NA | 50.1 | 350.6 | 0.15 | 0.59 | 33.3 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

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14S1091200 PNUR
Post Development Thursday PM
O'Connell St/ Barney St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
|  |  |  |  |  |  |  |  |  |  |  |
| 1 L | 148 | 0.0 | 0.620 | 9.0 | LOS A | 7.1 | 49.4 | 0.40 | 0.47 | 48.0 |
| 2 T | 471 | 0.0 | 0.620 | 0.8 | LOS A | 7.1 | 49.4 | 0.40 | 0.00 | 51.6 |
| 3 R | 402 | 0.0 | 0.620 | 9.3 | LOS A | 7.1 | 49.4 | 0.40 | 0.71 | 48.0 |
| Approach | 1021 | 0.0 | 0.620 | 5.3 | NA | 7.1 | 49.4 | 0.40 | 0.35 | 49.6 |
| East: Barney Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 409 | 0.0 | 1.015 | 65.7 | LOS E | 49.8 | 348.7 | 1.00 | 1.17 | 21.4 |
| 5 T | 191 | 0.0 | 1.015 | 64.4 | LOS E | 49.8 | 348.7 | 1.00 | 1.46 | 21.4 |
| $6 \quad \mathrm{R}$ | 99 | 0.0 | 1.015 | 65.9 | LOS E | 49.8 | 348.7 | 1.00 | 1.46 | 21.3 |
| Approach | 699 | 0.0 | 1.015 | 65.4 | LOS E | 49.8 | 348.7 | 1.00 | 1.29 | 21.4 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 16 | 0.0 | 0.040 | 11.8 | LOS A | 0.4 | 2.8 | 0.68 | 0.30 | 47.5 |
| 8 T | 58 | 0.0 | 0.040 | 3.6 | LOS A | 0.4 | 2.8 | 0.68 | 0.00 | 48.1 |
| 9 R | 1 | 0.0 | 0.040 | 12.0 | LOS A | 0.4 | 2.8 | 0.68 | 0.91 | 47.5 |
| Approach | 75 | 0.0 | 0.040 | 5.4 | NA | 0.4 | 2.8 | 0.68 | 0.08 | 48.0 |
| West: New Road From Development |  |  |  |  |  |  |  |  |  |  |
| 10 L | 14 | 0.0 | 0.369 | 25.2 | LOS B | 1.4 | 9.5 | 0.83 | 0.99 | 35.4 |
| 11 T | 24 | 0.0 | 0.369 | 24.0 | LOS B | 1.4 | 9.5 | 0.83 | 0.99 | 35.7 |
| 12 R | 54 | 0.0 | 0.369 | 25.5 | LOS B | 1.4 | 9.5 | 0.83 | 1.01 | 35.3 |
| Approach | 92 | 0.0 | 0.369 | 25.0 | LOS B | 1.4 | 9.5 | 0.83 | 1.00 | 35.4 |
| All Vehicles | 1886 | 0.0 | 1.015 | 28.5 | NA | 49.8 | 348.7 | 0.66 | 0.72 | 32.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday PM
O'Connell St/ Dunlop St
Giveway / Yield (Two-Way)


Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday PM
New St/ Factory St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: New Street 0.0 |  |  |  |  |  |  |  |  |  |  |
| 2 T | 76 | 0.0 | 0.069 | 0.1 | LOS A | 0.4 | 2.5 | 0.14 | 0.00 | 56.9 |
| 3 R | 53 | 0.0 | 0.069 | 8.6 | LOS A | 0.4 | 2.5 | 0.14 | 0.83 | 48.5 |
| Approach | 128 | 0.0 | 0.069 | 3.6 | NA | 0.4 | 2.5 | 0.14 | 0.34 | 53.1 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 85 | 0.0 | 0.119 | 8.6 | LOS A | 0.5 | 3.4 | 0.12 | 0.61 | 48.4 |
| 6 R | 75 | 0.0 | 0.119 | 8.8 | LOS A | 0.5 | 3.4 | 0.12 | 0.70 | 48.2 |
| Approach | 160 | 0.0 | 0.119 | 8.7 | LOS A | 0.5 | 3.4 | 0.12 | 0.65 | 48.3 |
| North: New Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 32 | 0.0 | 0.027 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.78 | 49.0 |
| 8 T | 19 | 0.0 | 0.027 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 51 | 0.0 | 0.027 | 5.1 | NA | 0.0 | 0.0 | 0.00 | 0.49 | 52.6 |
| All Vehicles | 339 | 0.0 | 0.119 | 6.2 | NA | 0.5 | 3.4 | 0.11 | 0.51 | 50.7 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday PM
O'Connell St/ Factory St
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
|  |  |  |  |  |  |  |  |  |  |  |
| 1 L | 56 | 0.0 | 0.549 | 14.7 | LOS B | 10.9 | 76.0 | 1.00 | 0.00 | 46.5 |
| 2 T | 1011 | 0.0 | 0.549 | 6.5 | LOS A | 10.9 | 76.0 | 1.00 | 0.00 | 44.8 |
| 3 R | 1 | 0.0 | 0.549 | 14.9 | LOS B | 10.9 | 76.0 | 1.00 | 1.15 | 46.6 |
| Approach | 1067 | 0.0 | 0.549 | 6.9 | NA | 10.9 | 76.0 | 1.00 | 0.00 | 44.9 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 28 | 0.0 | 0.603 | 35.7 | LOS C | 2.1 | 15.0 | 0.90 | 1.16 | 31.0 |
| 5 T | 102 | 0.0 | 0.603 | 35.3 | LOS C | 2.1 | 15.0 | 0.90 | 1.10 | 31.1 |
| 6 R | 1 | 0.0 | 0.603 | 35.5 | LOS C | 2.1 | 15.0 | 0.90 | 1.11 | 31.1 |
| Approach | 132 | 0.0 | 0.603 | 35.4 | LOS C | 2.1 | 15.0 | 0.90 | 1.12 | 31.1 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 87 | 0.0 | 0.295 | 20.2 | LOS B | 6.9 | 48.3 | 1.00 | 0.00 | 41.2 |
| 8 T | 480 | 0.0 | 0.295 | 12.0 | LOS A | 6.9 | 48.3 | 1.00 | 0.00 | 41.4 |
| 9 R | 1 | 0.0 | 0.295 | 20.5 | LOS B | 6.9 | 48.3 | 1.00 | 1.07 | 41.3 |
| Approach | 568 | 0.0 | 0.295 | 13.3 | NA | 6.9 | 48.3 | 1.00 | 0.00 | 41.4 |
| West: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 38 | 0.0 | 0.375 | 29.5 | LOS C | 1.4 | 9.6 | 0.88 | 1.06 | 34.0 |
| 11 T | 44 | 0.0 | 0.375 | 29.1 | LOS C | 1.4 | 9.6 | 0.88 | 1.05 | 34.2 |
| 12 R | 6 | 0.0 | 0.375 | 29.3 | LOS C | 1.4 | 9.6 | 0.88 | 1.06 | 34.1 |
| Approach | 88 | 0.0 | 0.375 | 29.3 | LOS C | 1.4 | 9.6 | 0.88 | 1.06 | 34.1 |
| All Vehicles | 1856 | 0.0 | 0.603 | 12.0 | NA | 10.9 | 76.0 | 0.99 | 0.13 | 41.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday PM
O'Connell St/ Board St/ Property Access
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue <br> Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
|  |  |  |  |  |  |  |  |  |  |  |
| 2 T | 4 | 0.0 | 0.228 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 3 R | 420 | 0.0 | 0.228 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.70 | 48.6 |
| Approach | 424 | 0.0 | 0.228 | 8.4 | NA | 0.0 | 0.0 | 0.00 | 0.70 | 48.7 |
| East: Board Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 21 | 0.0 | 0.012 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.67 | 49.0 |
| 6 R | 1 | 0.0 | 0.012 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.73 | 48.6 |
| Approach | 22 | 0.0 | 0.012 | 8.2 | NA | 0.0 | 0.0 | 0.00 | 0.67 | 48.9 |
| North: Access Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 6 | 0.0 | 0.010 | 8.9 | LOS A | 0.0 | 0.3 | 0.07 | 0.69 | 48.2 |
| 8 T | 6 | 0.0 | 0.010 | 7.7 | LOS A | 0.0 | 0.3 | 0.07 | 0.55 | 49.5 |
| Approach | 13 | 0.0 | 0.010 | 8.3 | LOS A | 0.0 | 0.3 | 0.07 | 0.62 | 48.8 |
| All Vehicles | 459 | 0.0 | 0.228 | 8.4 | NA | 0.0 | 0.3 | 0.00 | 0.69 | 48.7 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Post Development Thursday PM
Fleet St/ Greenup Drive
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \text { v/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | f Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: New Street 0.0 Sec 0 |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 89 | 0.0 | 0.090 | 8.5 | LOS A | 0.5 | 3.4 | 0.28 | 0.54 | 48.1 |
| 2 | T | 53 | 0.0 | 0.090 | 0.3 | LOS A | 0.5 | 3.4 | 0.28 | 0.00 | 53.6 |
| 3 | R | 25 | 0.0 | 0.090 | 8.7 | LOS A | 0.5 | 3.4 | 0.28 | 0.70 | 48.1 |
| Approa |  | 167 | 0.0 | 0.090 | 5.9 | NA | 0.5 | 3.4 | 0.28 | 0.39 | 49.7 |
| East: Albert Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 99 | 0.0 | 0.151 | 9.0 | LOS A | 0.6 | 4.3 | 0.22 | 0.63 | 48.0 |
| 5 | T | 67 | 0.0 | 0.151 | 7.7 | LOS A | 0.6 | 4.3 | 0.22 | 0.60 | 49.0 |
| 6 | R | 17 | 0.0 | 0.151 | 9.2 | LOS A | 0.6 | 4.3 | 0.22 | 0.75 | 47.9 |
| Approa |  | 183 | 0.0 | 0.151 | 8.5 | LOS A | 0.6 | 4.3 | 0.22 | 0.63 | 48.4 |
| North: New Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 6 | 0.0 | 0.057 | 8.6 | LOS A | 0.3 | 2.2 | 0.26 | 0.71 | 48.9 |
| 8 | T | 88 | 0.0 | 0.057 | 0.4 | LOS A | 0.3 | 2.2 | 0.26 | 0.00 | 54.8 |
| 9 | R | 14 | 0.0 | 0.057 | 8.9 | LOS A | 0.3 | 2.2 | 0.26 | 0.90 | 48.7 |
| Approa |  | 108 | 0.0 | 0.057 | 2.0 | NA | 0.3 | 2.2 | 0.26 | 0.15 | 53.6 |
| West: Greenup Drive |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 86 | 0.0 | 0.298 | 10.0 | LOS A | 1.2 | 8.7 | 0.32 | 0.60 | 47.0 |
| 11 | T | 24 | 0.0 | 0.298 | 8.8 | LOS A | 1.2 | 8.7 | 0.32 | 0.60 | 48.0 |
| 12 | R | 164 | 0.0 | 0.298 | 10.3 | LOS A | 1.2 | 8.7 | 0.32 | 0.78 | 46.8 |
| Approach |  | 275 | 0.0 | 0.298 | 10.1 | LOS A | 1.2 | 8.7 | 0.32 | 0.71 | 46.9 |
| All Vehicles |  | 734 | 0.0 | 0.298 | 7.5 | NA | 1.2 | 8.7 | 0.28 | 0.53 | 48.8 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Future Saturday
O'Connell St/ Fennell St
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 188 | 0.0 | 0.217 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.84 | 49.0 |
| 2 | T | 648 | 0.0 | 0.217 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 3 | R | 11 | 0.0 | 0.018 | 12.7 | LOS A | 0.1 | 0.4 | 0.59 | 0.76 | 44.4 |
| Approa |  | 847 | 0.0 | 0.217 | 2.0 | NA | 0.1 | 0.4 | 0.01 | 0.20 | 56.9 |
| East: Fennell Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 13 | 0.0 | 0.185 | 40.8 | LOS C | 0.6 | 3.9 | 0.87 | 1.01 | 28.9 |
| 5 | T | 7 | 0.0 | 0.185 | 41.6 | LOS C | 0.6 | 3.9 | 0.87 | 1.01 | 28.8 |
| 6 | R | 2 | 0.0 | 0.185 | 40.9 | LOS C | 0.6 | 3.9 | 0.87 | 1.00 | 28.9 |
| Approa |  | 22 | 0.0 | 0.185 | 41.1 | LOS C | 0.6 | 3.9 | 0.87 | 1.01 | 28.9 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 9 | 0.0 | 0.225 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.08 | 49.0 |
| 8 | T | 868 | 0.0 | 0.225 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 9 | R | 23 | 0.0 | 0.034 | 12.0 | LOSA | 0.1 | 0.8 | 0.53 | 0.75 | 45.0 |
| Approa |  | 901 | 0.0 | 0.225 | 0.4 | NA | 0.1 | 0.8 | 0.01 | 0.03 | 59.4 |
| West: Fennell Street |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 27 | 0.0 | 3.127 | 2004.1 | LOS F | 101.0 | 707.2 | 1.00 | 4.86 | 1.1 |
| 11 | T | 16 | 0.0 | 3.127 | 2004.9 | LOS F | 101.0 | 707.2 | 1.00 | 3.72 | 1.1 |
| 12 | R | 172 | 0.0 | 3.127 | 2004.2 | LOS F | 101.0 | 707.2 | 1.00 | 3.64 | 1.1 |
| Approach |  | 215 | 0.0 | 3.127 | 2004.2 | LOS F | 101.0 | 707.2 | 1.00 | 3.80 | 1.1 |
| All Vehicles |  | 1985 | 0.0 | 3.127 | 218.3 | NA | 101.0 | 707.2 | 0.13 | 0.52 | 8.6 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Future Saturday
Marsden St/ Market St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Marsden Street |  |  |  |  |  |  |  |  |  |  |
| 2 T | 492 | 0.0 | 0.396 | 1.8 | LOS A | 3.3 | 23.3 | 0.47 | 0.00 | 51.0 |
| 3 R | 198 | 0.0 | 0.396 | 10.3 | LOS A | 3.3 | 23.3 | 0.47 | 0.85 | 48.3 |
| Approach | 689 | 0.0 | 0.396 | 4.3 | NA | 3.3 | 23.3 | 0.47 | 0.24 | 50.2 |
| East: Market Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 357 | 0.0 | 0.295 | 9.6 | LOS A | 1.6 | 10.9 | 0.44 | 0.68 | 47.1 |
| 6 R | 7 | 0.0 | 0.295 | 9.9 | LOS A | 1.6 | 10.9 | 0.44 | 0.84 | 47.2 |
| Approach | 364 | 0.0 | 0.295 | 9.6 | LOS A | 1.6 | 10.9 | 0.44 | 0.68 | 47.1 |
| North: Marsden Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 33 | 0.0 | 0.164 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.02 | 49.0 |
| 8 T | 285 | 0.0 | 0.164 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 318 | 0.0 | 0.164 | 0.8 | NA | 0.0 | 0.0 | 0.00 | 0.10 | 58.6 |
| All Vehicles | 1372 | 0.0 | 0.396 | 4.9 | NA | 3.3 | 23.3 | 0.35 | 0.33 | 51.0 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

## 14S1091200 PNUR

Future Saturday
Church St/ Market St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Church Street |  |  |  |  |  |  |  |  |  |  |
| L | 4 | 0.0 | 0.005 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.84 | 49.0 |
| 2 T | 5 | 0.0 | 0.005 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 9 | 0.0 | 0.005 | 3.6 | NA | 0.0 | 0.0 | 0.00 | 0.38 | 54.5 |
| North: Church Street |  |  |  |  |  |  |  |  |  |  |
| 8 T | 26 | 0.0 | 0.013 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 9 R | 354 | 0.0 | 0.303 | 8.9 | LOS A | 1.0 | 7.1 | 0.57 | 0.40 | 46.5 |
| Approach | 380 | 0.0 | 0.303 | 8.2 | NA | 1.0 | 7.1 | 0.53 | 0.37 | 47.2 |
| West: Market Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 220 | 0.0 | 0.142 | 8.3 | LOS A | 0.7 | 4.9 | 0.03 | 0.65 | 48.8 |
| 12 R | 5 | 0.0 | 0.142 | 8.5 | LOS A | 0.7 | 4.9 | 0.03 | 0.73 | 48.5 |
| Approach | 225 | 0.0 | 0.142 | 8.3 | LOS A | 0.7 | 4.9 | 0.03 | 0.65 | 48.8 |
| All Vehicles | 615 | 0.0 | 0.303 | 8.2 | NA | 1.0 | 7.1 | 0.34 | 0.47 | 47.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

## 14S1091200 PNUR

Future Saturday
Church St/ Board St/ Seville St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. <br> Satn <br> v/c | Average Delay $\qquad$ sec | Level of Service | 95\% Back Vehicles $\qquad$ | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Church Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 18 | 0.0 | 0.328 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.07 | 49.0 |
| 2 T | 621 | 0.0 | 0.328 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 639 | 0.0 | 0.328 | 0.2 | NA | 0.0 | 0.0 | 0.00 | 0.03 | 59.6 |
| East: Seville Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 25 | 0.0 | 0.113 | 23.6 | LOS B | 0.4 | 2.5 | 0.85 | 0.95 | 36.4 |
| Approach | 25 | 0.0 | 0.113 | 23.6 | LOS B | 0.4 | 2.5 | 0.85 | 0.95 | 36.4 |
| North: Church Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 32 | 0.0 | 0.396 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 1.06 | 49.0 |
| 8 T | 1509 | 0.0 | 0.396 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 1541 | 0.0 | 0.396 | 0.2 | NA | 0.0 | 0.0 | 0.00 | 0.02 | 59.7 |
| West: Board Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 395 | 0.0 | 0.568 | 15.4 | LOS B | 4.2 | 29.2 | 0.70 | 1.06 | 42.1 |
| Approach | 395 | 0.0 | 0.568 | 15.4 | LOS B | 4.2 | 29.2 | 0.70 | 1.06 | 42.1 |
| All Vehicles | 2600 | 0.0 | 0.568 | 2.7 | NA | 4.2 | 29.2 | 0.12 | 0.19 | 55.8 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

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14S1091200 PNUR
Future Saturday
O'Connell St/ Barney St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. <br> Satn <br> v/c | Average Delay $\qquad$ sec | Level of Service | 95\% Back Vehicles $\qquad$ | Queue Distance $\qquad$ m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 94 | 0.0 | 0.334 | 8.8 | LOS A | 2.3 | 16.4 | 0.32 | 0.58 | 48.5 |
| 2 T | 343 | 0.0 | 0.334 | 0.6 | LOS A | 2.3 | 16.4 | 0.32 | 0.00 | 53.4 |
| 3 R | 139 | 0.0 | 0.334 | 9.1 | LOS A | 2.3 | 16.4 | 0.32 | 0.81 | 48.4 |
| Approach | 576 | 0.0 | 0.334 | 4.0 | NA | 2.3 | 16.4 | 0.32 | 0.29 | 51.3 |
| East: Barney Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 505 | 0.0 | 1.051 | 90.6 | LOS F | 61.4 | 429.7 | 1.00 | 1.80 | 17.2 |
| 5 T | 113 | 0.0 | 1.051 | 89.4 | LOS F | 61.4 | 429.7 | 1.00 | 1.84 | 17.2 |
| $6 \quad \mathrm{R}$ | 92 | 0.0 | 1.051 | 90.9 | LOS F | 61.4 | 429.7 | 1.00 | 1.84 | 17.1 |
| Approach | 709 | 0.0 | 1.051 | 90.5 | LOS F | 61.4 | 429.7 | 1.00 | 1.81 | 17.2 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 9 | 0.0 | 0.059 | 10.3 | LOS A | 0.5 | 3.4 | 0.55 | 0.46 | 49.0 |
| 8 T | 102 | 0.0 | 0.059 | 2.2 | LOS A | 0.5 | 3.4 | 0.55 | 0.00 | 50.4 |
| 9 R | 1 | 0.0 | 0.059 | 10.6 | LOS A | 0.5 | 3.4 | 0.55 | 0.93 | 48.9 |
| Approach | 113 | 0.0 | 0.059 | 2.9 | NA | 0.5 | 3.4 | 0.55 | 0.05 | 50.3 |
| West: New Road From Development |  |  |  |  |  |  |  |  |  |  |
| 10 L | 34 | 0.0 | 1.083 | 172.4 | LOS F | 18.4 | 129.0 | 1.00 | 2.74 | 10.4 |
| 11 T | 9 | 0.0 | 1.083 | 171.2 | LOS F | 18.4 | 129.0 | 1.00 | 2.42 | 10.4 |
| 12 R | 136 | 0.0 | 1.083 | 172.7 | LOS F | 18.4 | 129.0 | 1.00 | 2.16 | 10.4 |
| Approach | 179 | 0.0 | 1.083 | 172.6 | LOS F | 18.4 | 129.0 | 1.00 | 2.28 | 10.4 |
| All Vehicles | 1577 | 0.0 | 1.083 | 62.0 | NA | 61.4 | 429.7 | 0.72 | 1.18 | 21.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Future Saturday
O'Connell St/ Dunlop St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street South |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 31 | 0.0 | 0.316 | 14.0 | LOS A | 4.1 | 29.0 | 0.83 | 0.18 | 46.4 |
| 2 | T | 552 | 0.0 | 0.316 | 5.8 | LOSA | 4.1 | 29.0 | 0.83 | 0.00 | 46.7 |
| 3 | R | 13 | 0.0 | 0.316 | 14.3 | LOSA | 4.1 | 29.0 | 0.83 | 1.08 | 46.4 |
| Approac |  | 595 | 0.0 | 0.316 | 6.4 | NA | 4.1 | 29.0 | 0.83 | 0.03 | 46.7 |
| East: Dunlop Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 9 | 0.0 | 0.071 | 17.5 | LOS B | 0.2 | 1.6 | 0.75 | 0.88 | 40.7 |
| 5 | T | 14 | 0.0 | 0.071 | 16.3 | LOS B | 0.2 | 1.6 | 0.75 | 0.89 | 41.2 |
| 6 | R | 1 | 0.0 | 0.071 | 17.8 | LOS B | 0.2 | 1.6 | 0.75 | 0.94 | 40.7 |
| Approac |  | 24 | 0.0 | 0.071 | 16.8 | LOS B | 0.2 | 1.6 | 0.75 | 0.89 | 41.0 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 5 | 0.0 | 0.392 | 12.8 | LOS A | 5.8 | 40.8 | 0.79 | 0.22 | 47.6 |
| 8 | T | 728 | 0.0 | 0.392 | 4.6 | LOS A | 5.8 | 40.8 | 0.79 | 0.00 | 47.3 |
| 9 | R | 16 | 0.0 | 0.392 | 13.1 | LOS A | 5.8 | 40.8 | 0.79 | 1.07 | 47.7 |
| Approac |  | 749 | 0.0 | 0.392 | 4.9 | NA | 5.8 | 40.8 | 0.79 | 0.02 | 47.3 |
| West: Dunlop Street |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 20 | 0.0 | 0.280 | 21.0 | LOS B | 1.0 | 6.9 | 0.79 | 0.93 | 38.1 |
| 11 | T | 34 | 0.0 | 0.280 | 19.7 | LOS B | 1.0 | 6.9 | 0.79 | 0.94 | 38.5 |
| 12 | R | 29 | 0.0 | 0.280 | 21.2 | LOS B | 1.0 | 6.9 | 0.79 | 0.98 | 38.1 |
| Approach |  | 83 | 0.0 | 0.280 | 20.6 | LOS B | 1.0 | 6.9 | 0.79 | 0.95 | 38.2 |
| All Vehicles |  | 1452 | 0.0 | 0.392 | 6.6 | NA | 5.8 | 40.8 | 0.81 | 0.09 | 46.3 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Future Saturday
New St/ Factory St
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: New Street 0 er min |  |  |  |  |  |  |  |  |  |  |
| 2 T | 25 | 0.0 | 0.045 | 0.2 | LOS A | 0.2 | 1.5 | 0.17 | 0.00 | 55.9 |
| 3 R | 55 | 0.0 | 0.045 | 8.7 | LOS A | 0.2 | 1.5 | 0.17 | 0.72 | 48.2 |
| Approach | 80 | 0.0 | 0.045 | 6.0 | NA | 0.2 | 1.5 | 0.17 | 0.49 | 50.4 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 55 | 0.0 | 0.075 | 8.5 | LOS A | 0.3 | 2.1 | 0.14 | 0.61 | 48.3 |
| 6 R | 48 | 0.0 | 0.075 | 8.7 | LOS A | 0.3 | 2.1 | 0.14 | 0.69 | 48.1 |
| Approach | 103 | 0.0 | 0.075 | 8.6 | LOS A | 0.3 | 2.1 | 0.14 | 0.64 | 48.2 |
| North: New Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 59 | 0.0 | 0.040 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.73 | 49.0 |
| 8 T | 17 | 0.0 | 0.040 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| Approach | 76 | 0.0 | 0.040 | 6.4 | NA | 0.0 | 0.0 | 0.00 | 0.57 | 51.0 |
| All Vehicles | 259 | 0.0 | 0.075 | 7.1 | NA | 0.3 | 2.1 | 0.11 | 0.57 | 49.7 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Future Saturday
O'Connell St/ Factory St
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 42 | 0.0 | 0.325 | 17.1 | LOS B | 5.8 | 40.7 | 0.93 | 0.07 | 43.9 |
| 2 | T | 586 | 0.0 | 0.325 | 8.9 | LOS A | 5.8 | 40.7 | 0.93 | 0.00 | 44.6 |
| 3 | R | 1 | 0.0 | 0.325 | 17.3 | LOS B | 5.8 | 40.7 | 0.93 | 1.07 | 44.0 |
| Approac |  | 629 | 0.0 | 0.325 | 9.5 | NA | 5.8 | 40.7 | 0.93 | 0.01 | 44.6 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 41 | 0.0 | 0.739 | 62.8 | LOS E | 3.1 | 21.9 | 0.94 | 1.23 | 22.4 |
| 5 | T | 63 | 0.0 | 0.739 | 62.4 | LOS E | 3.1 | 21.9 | 0.94 | 1.18 | 22.4 |
| 6 | R | 1 | 0.0 | 0.739 | 62.6 | LOS E | 3.1 | 21.9 | 0.94 | 1.18 | 22.4 |
| Approac |  | 105 | 0.0 | 0.739 | 62.6 | LOS E | 3.1 | 21.9 | 0.94 | 1.20 | 22.4 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 165 | 0.0 | 0.400 | 15.0 | LOS B | 8.0 | 55.8 | 0.99 | 0.01 | 45.4 |
| 8 | T | 604 | 0.0 | 0.400 | 6.8 | LOS A | 8.0 | 55.8 | 0.99 | 0.00 | 44.1 |
| 9 | R | 1 | 0.0 | 0.400 | 15.3 | LOS B | 8.0 | 55.8 | 0.99 | 1.09 | 45.5 |
| Approac |  | 771 | 0.0 | 0.400 | 8.6 | NA | 8.0 | 55.8 | 0.99 | 0.00 | 44.4 |
| West: Factory Street |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 11 | 0.0 | 1.308 | 390.3 | LOS F | 23.3 | 163.2 | 1.00 | 2.95 | 5.1 |
| 11 | T | 92 | 0.0 | 1.308 | 389.9 | LOS F | 23.3 | 163.2 | 1.00 | 2.33 | 5.1 |
| 12 | R | 16 | 0.0 | 1.308 | 390.1 | LOS F | 23.3 | 163.2 | 1.00 | 2.35 | 5.1 |
| Approach |  | 118 | 0.0 | 1.308 | 389.9 | LOS F | 23.3 | 163.2 | 1.00 | 2.39 | 5.1 |
| All Vehicles |  | 1623 | 0.0 | 1.308 | 40.1 | NA | 23.3 | 163.2 | 0.97 | 0.25 | 27.5 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

## 14S1091200 PNUR

Future Saturday
O'Connell St/ Board St/ Property Access
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance | Prop. Queued | Effective <br> Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 2 T | 12 | 0.0 | 0.211 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 60.0 |
| 3 R | 380 | 0.0 | 0.211 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.71 | 48.6 |
| Approach | 392 | 0.0 | 0.211 | 8.2 | NA | 0.0 | 0.0 | 0.00 | 0.69 | 48.9 |
| East: Board Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 23 | 0.0 | 0.015 | 8.2 | LOS A | 0.0 | 0.0 | 0.00 | 0.66 | 49.0 |
| 6 R | 4 | 0.0 | 0.015 | 8.4 | LOS A | 0.0 | 0.0 | 0.00 | 0.73 | 48.6 |
| Approach | 27 | 0.0 | 0.015 | 8.2 | NA | 0.0 | 0.0 | 0.00 | 0.67 | 48.9 |
| North: Access Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 13 | 0.0 | 0.013 | 9.3 | LOS A | 0.0 | 0.3 | 0.18 | 0.66 | 47.8 |
| 8 T | 2 | 0.0 | 0.013 | 8.0 | LOS A | 0.0 | 0.3 | 0.18 | 0.50 | 49.0 |
| Approach | 15 | 0.0 | 0.013 | 9.1 | LOS A | 0.0 | 0.3 | 0.18 | 0.64 | 48.0 |
| All Vehicles | 434 | 0.0 | 0.211 | 8.2 | NA | 0.0 | 0.3 | 0.01 | 0.69 | 48.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

14S1091200 PNUR
Future Saturday
New St/ Greenup Drive
Giveway / Yield (Two-Way)

| Mov ID | Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | f Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 40 | 0.0 | 0.102 | 8.5 | LOS A | 0.5 | 3.8 | 0.22 | 0.62 | 48.4 |
| 2 | T | 86 | 0.0 | 0.102 | 0.3 | LOS A | 0.5 | 3.8 | 0.22 | 0.00 | 55.1 |
| 3 | R | 60 | 0.0 | 0.102 | 8.7 | LOS A | 0.5 | 3.8 | 0.22 | 0.77 | 48.3 |
| Approa |  | 186 | 0.0 | 0.102 | 4.8 | NA | 0.5 | 3.8 | 0.22 | 0.38 | 51.3 |
| East: Albert Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 61 | 0.0 | 0.093 | 8.9 | LOS A | 0.4 | 2.5 | 0.20 | 0.62 | 48.1 |
| 5 | T | 42 | 0.0 | 0.093 | 7.6 | LOS A | 0.4 | 2.5 | 0.20 | 0.59 | 49.1 |
| 6 | R | 11 | 0.0 | 0.093 | 9.2 | LOS A | 0.4 | 2.5 | 0.20 | 0.74 | 48.0 |
| Approa |  | 114 | 0.0 | 0.093 | 8.5 | LOS A | 0.4 | 2.5 | 0.20 | 0.62 | 48.5 |
| North: New Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 15 | 0.0 | 0.050 | 8.6 | LOS A | 0.3 | 1.9 | 0.26 | 0.70 | 48.8 |
| 8 | T | 74 | 0.0 | 0.050 | 0.4 | LOS A | 0.3 | 1.9 | 0.26 | 0.00 | 54.9 |
| 9 | R | 6 | 0.0 | 0.050 | 8.8 | LOS A | 0.3 | 1.9 | 0.26 | 0.88 | 48.7 |
| Approa |  | 95 | 0.0 | 0.050 | 2.2 | NA | 0.3 | 1.9 | 0.26 | 0.17 | 53.4 |
| West: Greenup Drive |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 9 | 0.0 | 0.090 | 9.4 | LOS A | 0.3 | 2.3 | 0.32 | 0.61 | 47.7 |
| 11 | T | 61 | 0.0 | 0.090 | 8.1 | LOS A | 0.3 | 2.3 | 0.32 | 0.60 | 48.5 |
| 12 | R | 20 | 0.0 | 0.090 | 9.6 | LOS A | 0.3 | 2.3 | 0.32 | 0.77 | 47.6 |
| Approach |  | 91 | 0.0 | 0.090 | 8.6 | LOS A | 0.3 | 2.3 | 0.32 | 0.64 | 48.2 |
| All Vehicles |  | 485 | 0.0 | 0.102 | 5.8 | NA | 0.5 | 3.8 | 0.24 | 0.44 | 50.4 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

Appendix B

## B. 3 Future Conditions with Intersection Upgrades

14S1091200 PNUR
Post Development Thursday AM
O'Connell St/ Fennell St
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. <br> Satn <br> v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 243 | 0.0 | 0.340 | 7.9 | LOS A | 2.1 | 14.8 | 0.20 | 0.62 | 48.8 |
| 2 | T | 643 | 0.0 | 0.340 | 6.8 | LOS A | 2.1 | 14.8 | 0.21 | 0.52 | 49.7 |
| 3 | R | 39 | 0.0 | 0.340 | 11.4 | LOS A | 2.1 | 14.6 | 0.21 | 0.80 | 46.2 |
| Approac |  | 925 | 0.0 | 0.340 | 7.3 | LOS A | 2.1 | 14.8 | 0.21 | 0.55 | 49.3 |
| East: Fennell Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 9 | 0.0 | 0.055 | 15.0 | LOS B | 0.3 | 2.0 | 0.82 | 0.87 | 42.7 |
| 5 | T | 11 | 0.0 | 0.055 | 14.3 | LOS A | 0.3 | 2.0 | 0.82 | 0.85 | 42.8 |
| 6 | R | 1 | 0.0 | 0.055 | 18.8 | LOS B | 0.3 | 2.0 | 0.82 | 0.91 | 40.6 |
| Approac |  | 21 | 0.0 | 0.055 | 14.8 | LOS B | 0.3 | 2.0 | 0.82 | 0.86 | 42.6 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 27 | 0.0 | 0.657 | 10.4 | LOS A | 6.2 | 43.4 | 0.62 | 0.75 | 47.2 |
| 8 | T | 1332 | 0.0 | 0.657 | 9.5 | LOS A | 6.2 | 43.4 | 0.63 | 0.70 | 47.3 |
| 9 | R | 61 | 0.0 | 0.657 | 14.2 | LOS A | 6.2 | 43.2 | 0.63 | 0.84 | 44.2 |
| Approac |  | 1420 | 0.0 | 0.657 | 9.7 | LOS A | 6.2 | 43.4 | 0.63 | 0.71 | 47.1 |
| West: Fennell Street |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 19 | 0.0 | 0.330 | 10.9 | LOS A | 2.0 | 14.1 | 0.67 | 0.77 | 45.7 |
| 11 | T | 13 | 0.0 | 0.330 | 10.2 | LOS A | 2.0 | 14.1 | 0.67 | 0.74 | 46.0 |
| 12 | R | 226 | 0.0 | 0.330 | 14.7 | LOS B | 2.0 | 14.1 | 0.67 | 0.83 | 43.2 |
| Approac |  | 258 | 0.0 | 0.330 | 14.2 | LOS A | 2.0 | 14.1 | 0.67 | 0.82 | 43.5 |
| All Vehi |  | 2624 | 0.0 | 0.657 | 9.4 | LOS A | 6.2 | 43.4 | 0.48 | 0.67 | 47.4 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 1 October 2014 10:32:15 AM SIDRA INTERSECTION 5.1.13.2093
Project: P:\14S1000-1099\14S1091200 PNUR - Rezoning\ModellingISIDRAI14S1091200sid SIDRA Future
Thursday AM.sip
8000056, GTA CONSULTANTS, ENTERPRISE

14S1091200 PNUR
Post Development Thursday AM
O'Connell St/ Barney St
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| L | 37 | 0.0 | 0.394 | 8.4 | LOS A | 3.3 | 23.3 | 0.43 | 0.60 | 47.8 |
| 2 T | 318 | 0.0 | 0.394 | 7.6 | LOS A | 3.3 | 23.3 | 0.43 | 0.55 | 48.0 |
| 3 R | 121 | 0.0 | 0.394 | 11.7 | LOS A | 3.3 | 23.3 | 0.43 | 0.72 | 45.7 |
| Approach | 476 | 0.0 | 0.394 | 8.7 | LOS A | 3.3 | 23.3 | 0.43 | 0.60 | 47.4 |
| East: Barney Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 852 | 0.0 | 1.065 | 88.6 | LOS F | 66.1 | 462.4 | 1.00 | 2.42 | 17.4 |
| 5 T | 65 | 0.0 | 1.065 | 87.9 | LOS F | 66.1 | 462.4 | 1.00 | 2.42 | 17.4 |
| 6 R | 48 | 0.0 | 1.065 | 91.9 | LOS F | 66.1 | 462.4 | 1.00 | 2.42 | 17.3 |
| Approach | 965 | 0.0 | 1.065 | 88.7 | LOS F | 66.1 | 462.4 | 1.00 | 2.42 | 17.4 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 6 | 0.0 | 0.181 | 10.2 | LOS A | 1.1 | 7.9 | 0.61 | 0.72 | 47.2 |
| 8 T | 147 | 0.0 | 0.181 | 9.4 | LOS A | 1.1 | 7.9 | 0.61 | 0.68 | 47.3 |
| 9 R | 1 | 0.0 | 0.181 | 13.5 | LOS A | 1.1 | 7.9 | 0.61 | 0.81 | 44.7 |
| Approach | 155 | 0.0 | 0.181 | 9.5 | LOS A | 1.1 | 7.9 | 0.61 | 0.68 | 47.2 |
| West: New Road From Development |  |  |  |  |  |  |  |  |  |  |
| 10 L | 55 | 0.0 | 0.381 | 11.9 | LOS A | 2.7 | 19.0 | 0.76 | 0.80 | 44.8 |
| 11 T | 23 | 0.0 | 0.381 | 11.1 | LOS A | 2.7 | 19.0 | 0.76 | 0.78 | 45.1 |
| 12 R | 217 | 0.0 | 0.381 | 15.2 | LOS B | 2.7 | 19.0 | 0.76 | 0.84 | 42.7 |
| Approach | 295 | 0.0 | 0.381 | 14.3 | LOS A | 2.7 | 19.0 | 0.76 | 0.83 | 43.2 |
| All Vehicles | 1891 | 0.0 | 1.065 | 50.5 | LOS D | 66.1 | 462.4 | 0.79 | 1.57 | 25.0 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 1 October 2014 10:16:21 AM SIDRA INTERSECTION 5.1.13.2093
Project: P:\14S1000-1099\14S1091200 PNUR - Rezoning\ModellingISIDRAI14S1091200sid SIDRA Future
Thursday AM.sip
8000056, GTA CONSULTANTS, ENTERPRISE

## 14S1091200 PNUR

Post Development Thursday AM
O'Connell St/ Barney St
Signals - Fixed Time Cycle Time $=70$ seconds (Practical Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. <br> Satn <br> v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 37 | 0.0 | 0.512 | 27.5 | LOS B | 9.7 | 67.7 | 0.84 | 0.88 | 36.0 |
| 2 | T | 318 | 0.0 | 0.512 | 19.3 | LOS B | 9.7 | 67.7 | 0.84 | 0.72 | 36.9 |
| 3 | R | 121 | 0.0 | 0.338 | 30.2 | LOS C | 3.4 | 23.6 | 0.89 | 0.78 | 32.7 |
| Approac |  | 476 | 0.0 | 0.512 | 22.7 | LOS B | 9.7 | 67.7 | 0.85 | 0.75 | 35.7 |
| East: Barney Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 852 | 0.0 | 0.713 | 17.1 | LOS B | 19.2 | 134.4 | 0.71 | 0.84 | 40.8 |
| 5 | T | 65 | 0.0 | 0.247 | 11.4 | LOS A | 2.2 | 15.5 | 0.60 | 0.48 | 42.6 |
| 6 | R | 48 | 0.0 | 0.247 | 19.8 | LOS B | 2.2 | 15.5 | 0.60 | 0.85 | 39.8 |
| Approac |  | 965 | 0.0 | 0.713 | 16.8 | LOS B | 19.2 | 134.4 | 0.70 | 0.82 | 40.9 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 6 | 0.0 | 0.436 | 36.4 | LOS C | 4.9 | 34.1 | 0.93 | 0.82 | 31.6 |
| 8 | T | 147 | 0.0 | 0.436 | 28.2 | LOS B | 4.9 | 34.1 | 0.93 | 0.75 | 32.0 |
| 9 | R | 1 | 0.0 | 0.436 | 36.5 | LOS C | 4.9 | 34.1 | 0.93 | 0.82 | 31.6 |
| Approac |  | 155 | 0.0 | 0.436 | 28.6 | LOS C | 4.9 | 34.1 | 0.93 | 0.75 | 32.0 |
| West: New Road From Development |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 55 | 0.0 | 0.088 | 19.1 | LOS B | 1.5 | 10.2 | 0.58 | 0.77 | 39.8 |
| 11 | T | 23 | 0.0 | 0.088 | 10.9 | LOS A | 1.5 | 10.2 | 0.58 | 0.46 | 42.6 |
| 12 | R | 217 | 0.0 | 0.839 | 45.0 | LOS D | 9.1 | 63.9 | 0.98 | 1.03 | 26.7 |
| Approach |  | 295 | 0.0 | 0.839 | 37.5 | LOS C | 9.1 | 63.9 | 0.88 | 0.94 | 29.4 |
| All Vehicles |  | 1891 | 0.0 | 0.839 | 22.5 | LOS B | 19.2 | 134.4 | 0.78 | 0.81 | 36.5 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | ---: | :---: | ---: | ---: | ---: | ---: | ---: |
| Mov ID | Description | Demand <br> Flow <br> ped/h | Average <br> Delay <br> sec | Level of <br> Service | Average Back of Queue <br> Pedestrian <br> ped | Prop. <br> Distance <br> Queued | Effective <br> Stop Rate <br> per ped |  |
| P1 | Across S approach | 53 | 13.8 | LOS B | 0.1 | 0.1 | 0.63 | 0.63 |
| P3 | Across E approach | 53 | 29.3 | LOS C | 0.1 | 0.1 | 0.91 | 0.91 |
| P5 | Across N approach | 53 | 12.6 | LOS B | 0.1 | 0.1 | 0.60 | 0.60 |
| P7 | Across W approach | 53 | 29.3 | LOS C | 0.1 | 0.1 | 0.91 | 0.91 |
| All Pedestrians | 212 | 21.2 | LOS C |  |  | 0.76 | 0.76 |  |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

14S1091200 PNUR
Post Development Thursday AM
O'Connell St/ Dunlop St
Roundabout


Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 1 October 2014 10:20:37 AM SIDRA INTERSECTION 5.1.13.2093
Project: P:\14S1000-1099\14S1091200 PNUR - Rezoning\ModellingISIDRAI14S1091200sid SIDRA Future
Thursday AM.sip
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14S1091200 PNUR
Post Development Thursday AM
O'Connell St/ Factory St
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. <br> Satn <br> v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 29 | 0.0 | 0.357 | 7.7 | LOS A | 2.8 | 19.7 | 0.19 | 0.61 | 48.9 |
| 2 | T | 487 | 0.0 | 0.357 | 6.9 | LOS A | 2.8 | 19.7 | 0.19 | 0.53 | 49.5 |
| 3 | R | 1 | 0.0 | 0.357 | 11.0 | LOS A | 2.8 | 19.7 | 0.19 | 0.80 | 46.3 |
| Approac |  | 518 | 0.0 | 0.357 | 7.0 | LOS A | 2.8 | 19.7 | 0.19 | 0.53 | 49.5 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 42 | 0.0 | 0.204 | 16.5 | LOS B | 1.2 | 8.6 | 0.79 | 0.88 | 41.3 |
| 5 | T | 31 | 0.0 | 0.204 | 15.8 | LOS B | 1.2 | 8.6 | 0.79 | 0.86 | 41.5 |
| 6 | R | 1 | 0.0 | 0.204 | 19.8 | LOS B | 1.2 | 8.6 | 0.79 | 0.92 | 39.6 |
| Approac |  | 74 | 0.0 | 0.204 | 16.3 | LOS B | 1.2 | 8.6 | 0.79 | 0.87 | 41.4 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 262 | 0.0 | 1.022 | 51.1 | LOS D | 65.1 | 455.6 | 1.00 | 1.37 | 25.0 |
| 8 | T | 923 | 0.0 | 1.022 | 50.3 | LOS D | 65.1 | 455.6 | 1.00 | 1.37 | 25.1 |
| 9 | R | 1 | 0.0 | 1.022 | 54.4 | LOS D | 65.1 | 455.6 | 1.00 | 1.37 | 24.6 |
| Approac |  | 1186 | 0.0 | 1.022 | 50.5 | LOS D | 65.1 | 455.6 | 1.00 | 1.37 | 25.1 |
| West: Factory Street |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 14 | 0.0 | 0.229 | 11.6 | LOS A | 1.6 | 11.5 | 0.69 | 0.73 | 45.8 |
| 11 | T | 149 | 0.0 | 0.229 | 10.8 | LOS A | 1.6 | 11.5 | 0.69 | 0.70 | 46.1 |
| 12 | R | 25 | 0.0 | 0.229 | 14.9 | LOS B | 1.6 | 11.5 | 0.69 | 0.80 | 43.5 |
| Approach |  | 188 | 0.0 | 0.229 | 11.4 | LOS A | 1.6 | 11.5 | 0.69 | 0.72 | 45.7 |
| All Vehi |  | 1966 | 0.0 | 1.022 | 34.0 | LOS C | 65.1 | 455.6 | 0.75 | 1.07 | 30.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 1 October 2014 10:24:06 AM SIDRA INTERSECTION 5.1.13.2093
Project: P:\14S1000-1099\14S1091200 PNUR - Rezoning\ModellingISIDRAI14S1091200sid SIDRA Future
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## 14S1091200 PNUR

Post Development Thursday AM
O'Connell St/ Factory St
Signals - Fixed Time Cycle Time $=80$ seconds (Practical Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 29 | 0.0 | 0.373 | 13.0 | LOS A | 7.8 | 54.6 | 0.42 | 0.98 | 45.8 |
| 2 T | 487 | 0.0 | 0.373 | 4.8 | LOS A | 7.8 | 54.6 | 0.42 | 0.37 | 51.0 |
| Approach | 517 | 0.0 | 0.373 | 5.3 | LOS A | 7.8 | 54.6 | 0.42 | 0.41 | 50.7 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 42 | 0.0 | 0.279 | 42.8 | LOS D | 2.6 | 18.5 | 0.94 | 0.77 | 28.1 |
| 5 T | 31 | 0.0 | 0.279 | 34.7 | LOS C | 2.6 | 18.5 | 0.94 | 0.72 | 28.4 |
| Approach | 73 | 0.0 | 0.279 | 39.4 | LOS C | 2.6 | 18.5 | 0.94 | 0.75 | 28.2 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 262 | 0.0 | 0.863 | 23.0 | LOS B | 39.9 | 279.6 | 0.81 | 0.99 | 38.5 |
| 8 T | 923 | 0.0 | 0.863 | 14.8 | LOS B | 39.9 | 279.6 | 0.81 | 0.82 | 39.8 |
| Approach | 1185 | 0.0 | 0.863 | 16.6 | LOS B | 39.9 | 279.6 | 0.81 | 0.86 | 39.5 |
| West: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 14 | 0.0 | 0.771 | 48.8 | LOS D | 7.9 | 55.2 | 1.00 | 0.92 | 26.6 |
| 11 T | 149 | 0.0 | 0.771 | 40.6 | LOS C | 7.9 | 55.2 | 1.00 | 0.92 | 26.7 |
| 12 R | 25 | 0.0 | 0.771 | 49.1 | LOS D | 7.9 | 55.2 | 1.00 | 0.92 | 26.6 |
| Approach | 188 | 0.0 | 0.771 | 42.3 | LOS C | 7.9 | 55.2 | 1.00 | 0.92 | 26.7 |
| All Vehicles | 1963 | 0.0 | 0.863 | 16.9 | LOS B | 39.9 | 279.6 | 0.73 | 0.74 | 39.4 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Mov ID | Description | Demand <br> Flow <br> ped/h | Average <br> Delay <br> sec | Level of <br> Service | Average Back of Queue <br> Pedestrian <br> ped | Prop. <br> Distance <br> Queued | Effective <br> Stop Rate <br> per ped |  |
| P1 | Across S approach | 53 | 34.2 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 |
| P3 | Across E approach | 53 | 4.9 | LOS A | 0.0 | 0.0 | 0.35 | 0.35 |
| P5 | Across N approach | 53 | 34.2 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 |
| P7 | Across W approach | 53 | 4.9 | LOS A | 0.0 | 0.0 | 0.35 | 0.35 |
| All Pedestrians | 212 | 19.6 | LOS B |  |  | 0.64 | 0.64 |  |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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14S1091200 PNUR
Post Development Thursday PM
O'Connell St/ Fennell St
Roundabout


Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

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14S1091200 PNUR
Post Development Thursday PM
O'Connell St/ Barney St
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| L | 148 | 0.0 | 0.988 | 44.2 | LOS D | 49.7 | 348.2 | 1.00 | 1.31 | 27.0 |
| 2 T | 471 | 0.0 | 0.988 | 43.5 | LOS D | 49.7 | 348.2 | 1.00 | 1.31 | 27.1 |
| 3 R | 402 | 0.0 | 0.988 | 47.5 | LOS D | 49.7 | 348.2 | 1.00 | 1.31 | 26.5 |
| Approach | 1021 | 0.0 | 0.988 | 45.2 | LOS D | 49.7 | 348.2 | 1.00 | 1.31 | 26.8 |
| East: Barney Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 409 | 0.0 | 0.561 | 8.7 | LOS A | 5.1 | 35.6 | 0.38 | 0.61 | 47.9 |
| 5 T | 191 | 0.0 | 0.561 | 7.9 | LOS A | 5.1 | 35.6 | 0.38 | 0.55 | 48.2 |
| 6 R | 99 | 0.0 | 0.561 | 12.0 | LOSA | 5.1 | 35.6 | 0.38 | 0.73 | 45.6 |
| Approach | 699 | 0.0 | 0.561 | 9.0 | LOS A | 5.1 | 35.6 | 0.38 | 0.61 | 47.7 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 16 | 0.0 | 0.103 | 10.9 | LOS A | 0.6 | 4.5 | 0.69 | 0.74 | 46.4 |
| 8 T | 58 | 0.0 | 0.103 | 10.2 | LOS A | 0.6 | 4.5 | 0.69 | 0.70 | 46.7 |
| 9 R | 1 | 0.0 | 0.103 | 14.3 | LOS A | 0.6 | 4.5 | 0.69 | 0.81 | 44.0 |
| Approach | 75 | 0.0 | 0.103 | 10.4 | LOS A | 0.6 | 4.5 | 0.69 | 0.71 | 46.6 |
| West: New Road From Development |  |  |  |  |  |  |  |  |  |  |
| 10 L | 14 | 0.0 | 0.258 | 17.7 | LOS B | 1.9 | 13.6 | 0.99 | 0.95 | 40.1 |
| 11 T | 24 | 0.0 | 0.258 | 16.9 | LOS B | 1.9 | 13.6 | 0.99 | 0.95 | 40.2 |
| 12 R | 54 | 0.0 | 0.258 | 21.0 | LOS B | 1.9 | 13.6 | 0.99 | 0.95 | 38.6 |
| Approach | 92 | 0.0 | 0.258 | 19.4 | LOS B | 1.9 | 13.6 | 0.99 | 0.95 | 39.2 |
| All Vehicles | 1886 | 0.0 | 0.988 | 29.1 | LOS C | 49.7 | 348.2 | 0.76 | 1.01 | 33.3 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 1 October 2014 10:58:17 AM SIDRA INTERSECTION 5.1.13.2093
Project: P:\14S1000-1099\14S1091200 PNUR - Rezoning\ModellingISIDRAI14S1091200sid SIDRA Future
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## 14S1091200 PNUR

Post Development Thursday PM
O'Connell St/ Barney St
Signals - Fixed Time Cycle Time $=70$ seconds (Practical Cycle Time)


Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back Pedestrian ped | f Queue Distance m | Prop. Queued | Effective Stop Rate per ped |
| P1 | Across S approach | 53 | 29.3 | LOS C | 0.1 | 0.1 | 0.91 | 0.91 |
| P3 | Across E approach | 53 | 29.3 | LOS C | 0.1 | 0.1 | 0.91 | 0.91 |
| P5 | Across N approach | 53 | 29.3 | LOS C | 0.1 | 0.1 | 0.91 | 0.91 |
| P7 | Across W approach | 53 | 29.3 | LOS C | 0.1 | 0.1 | 0.91 | 0.91 |
| All Pedestrians |  | 212 | 29.3 | LOS C |  |  | 0.91 | 0.91 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

14S1091200 PNUR
Post Development Thursday PM
O'Connell St/ Dunlop St
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street South |  |  |  |  |  |  |  |  |  |  |
| 1 L | 55 | 0.0 | 0.719 | 8.3 | LOS A | 10.3 | 72.0 | 0.41 | 0.57 | 48.0 |
| 2 T | 964 | 0.0 | 0.719 | 7.5 | LOS A | 10.3 | 72.0 | 0.41 | 0.51 | 48.3 |
| 3 R | 20 | 0.0 | 0.719 | 11.6 | LOS A | 10.3 | 72.0 | 0.41 | 0.70 | 46.0 |
| Approach | 1039 | 0.0 | 0.719 | 7.6 | LOS A | 10.3 | 72.0 | 0.41 | 0.52 | 48.2 |
| East: Dunlop Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 9 | 0.0 | 0.054 | 11.6 | LOS A | 0.3 | 2.2 | 0.68 | 0.72 | 45.7 |
| 5 T | 26 | 0.0 | 0.054 | 10.9 | LOS A | 0.3 | 2.2 | 0.68 | 0.69 | 46.0 |
| 6 R | 2 | 0.0 | 0.054 | 14.9 | LOS B | 0.3 | 2.2 | 0.68 | 0.79 | 43.4 |
| Approach | 38 | 0.0 | 0.054 | 11.3 | LOS A | 0.3 | 2.2 | 0.68 | 0.70 | 45.7 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 4 | 0.0 | 0.418 | 8.3 | LOS A | 3.7 | 26.1 | 0.42 | 0.60 | 48.0 |
| 8 T | 497 | 0.0 | 0.418 | 7.5 | LOS A | 3.7 | 26.1 | 0.42 | 0.54 | 48.2 |
| 9 R | 23 | 0.0 | 0.418 | 11.6 | LOS A | 3.7 | 26.1 | 0.42 | 0.74 | 46.0 |
| Approach | 524 | 0.0 | 0.418 | 7.7 | LOS A | 3.7 | 26.1 | 0.42 | 0.55 | 48.1 |
| West: Dunlop Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 65 | 0.0 | 0.302 | 19.3 | LOS B | 2.5 | 17.3 | 0.98 | 0.90 | 39.0 |
| 11 T | 16 | 0.0 | 0.302 | 18.5 | LOS B | 2.5 | 17.3 | 0.98 | 0.90 | 39.1 |
| 12 R | 58 | 0.0 | 0.302 | 22.6 | LOS B | 2.5 | 17.3 | 0.98 | 0.91 | 37.5 |
| Approach | 139 | 0.0 | 0.302 | 20.6 | LOS B | 2.5 | 17.3 | 0.98 | 0.90 | 38.4 |
| All Vehicles | 1740 | 0.0 | 0.719 | 8.8 | LOS A | 10.3 | 72.0 | 0.47 | 0.56 | 47.2 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 1 October 2014 11:01:54 AM SIDRA INTERSECTION 5.1.13.2093
Project: P:\14S1000-1099\14S1091200 PNUR - Rezoning\ModellingISIDRAI14S1091200sid SIDRA Future
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14S1091200 PNUR
Post Development Thursday PM
O'Connell St/ Factory St
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn V/C | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street 0.0 |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 56 | 0.0 | 0.816 | 9.5 | LOS A | 14.8 | 103.5 | 0.68 | 0.58 | 47.0 |
| 2 | T | 1011 | 0.0 | 0.816 | 8.7 | LOS A | 14.8 | 103.5 | 0.68 | 0.55 | 46.9 |
| 3 | R | 1 | 0.0 | 0.816 | 12.8 | LOS A | 14.8 | 103.5 | 0.68 | 0.66 | 45.4 |
| Approac |  | 1067 | 0.0 | 0.816 | 8.7 | LOS A | 14.8 | 103.5 | 0.68 | 0.55 | 46.9 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 28 | 0.0 | 0.171 | 11.2 | LOS A | 0.9 | 6.1 | 0.53 | 0.74 | 46.0 |
| 5 | T | 102 | 0.0 | 0.171 | 10.5 | LOS A | 0.9 | 6.1 | 0.53 | 0.69 | 46.5 |
| 6 | R | 1 | 0.0 | 0.171 | 14.5 | LOS B | 0.9 | 6.1 | 0.53 | 0.85 | 43.6 |
| Approac |  | 132 | 0.0 | 0.171 | 10.7 | LOS A | 0.9 | 6.1 | 0.53 | 0.71 | 46.4 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 87 | 0.0 | 0.413 | 7.9 | LOS A | 4.0 | 27.7 | 0.32 | 0.58 | 48.4 |
| 8 | T | 480 | 0.0 | 0.413 | 7.1 | LOS A | 4.0 | 27.7 | 0.32 | 0.52 | 48.8 |
| 9 | R | 1 | 0.0 | 0.413 | 11.2 | LOS A | 4.0 | 27.7 | 0.32 | 0.74 | 46.1 |
| Approac |  | 568 | 0.0 | 0.413 | 7.3 | LOS A | 4.0 | 27.7 | 0.32 | 0.53 | 48.7 |
| West: Factory Street |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 38 | 0.0 | 0.228 | 19.2 | LOS B | 1.9 | 13.0 | 1.00 | 0.90 | 39.4 |
| 11 | T | 44 | 0.0 | 0.228 | 18.4 | LOS B | 1.9 | 13.0 | 1.00 | 0.90 | 39.5 |
| 12 | R | 6 | 0.0 | 0.228 | 22.5 | LOS B | 1.9 | 13.0 | 1.00 | 0.90 | 37.9 |
| Approac |  | 88 | 0.0 | 0.228 | 19.1 | LOS B | 1.9 | 13.0 | 1.00 | 0.90 | 39.3 |
| All Vehi |  | 1856 | 0.0 | 0.816 | 8.9 | LOS A | 14.8 | 103.5 | 0.57 | 0.57 | 47.0 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 1 October 2014 11:04:34 AM SIDRA INTERSECTION 5.1.13.2093
Project: P:\14S1000-1099\14S1091200 PNUR - Rezoning\ModellingISIDRAI14S1091200sid SIDRA Future
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## 14S1091200 PNUR

Post Development Thursday PM
O'Connell St/ Factory St
Signals - Fixed Time Cycle Time $=60$ seconds (Practical Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 56 | 0.0 | 0.889 | 29.9 | LOS C | 35.7 | 250.2 | 0.92 | 1.11 | 34.9 |
| 2 T | 1011 | 0.0 | 0.889 | 21.7 | LOS B | 35.7 | 250.2 | 0.92 | 1.03 | 35.4 |
| Approach | 1066 | 0.0 | 0.889 | 22.2 | LOS B | 35.7 | 250.2 | 0.92 | 1.03 | 35.4 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 28 | 0.0 | 0.369 | 32.3 | LOS C | 3.5 | 24.5 | 0.92 | 0.81 | 33.2 |
| 5 T | 102 | 0.0 | 0.369 | 24.2 | LOS B | 3.5 | 24.5 | 0.92 | 0.73 | 33.7 |
| Approach | 131 | 0.0 | 0.369 | 25.9 | LOS B | 3.5 | 24.5 | 0.92 | 0.75 | 33.6 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 87 | 0.0 | 0.475 | 14.9 | LOS B | 9.0 | 63.2 | 0.58 | 0.93 | 44.4 |
| 8 T | 480 | 0.0 | 0.475 | 6.7 | LOS A | 9.0 | 63.2 | 0.58 | 0.52 | 47.8 |
| Approach | 567 | 0.0 | 0.475 | 8.0 | LOS A | 9.0 | 63.2 | 0.58 | 0.58 | 47.3 |
| West: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 38 | 0.0 | 0.268 | 31.9 | LOS C | 2.3 | 16.3 | 0.90 | 0.78 | 32.8 |
| 11 T | 44 | 0.0 | 0.268 | 23.7 | LOS B | 2.3 | 16.3 | 0.90 | 0.70 | 33.4 |
| 12 R | 6 | 0.0 | 0.268 | 32.1 | LOS C | 2.3 | 16.3 | 0.90 | 0.79 | 32.8 |
| Approach | 88 | 0.0 | 0.268 | 27.8 | LOS B | 2.3 | 16.3 | 0.90 | 0.74 | 33.1 |
| All Vehicles | 1853 | 0.0 | 0.889 | 18.4 | LOS B | 35.7 | 250.2 | 0.82 | 0.86 | 38.0 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back Pedestrian ped | f Queue <br> Distance <br> m | Prop. Queued | Effective Stop Rate per ped |
| P1 | Across S approach | 53 | 24.3 | LOS C | 0.1 | 0.1 | 0.90 | 0.90 |
| P3 | Across E approach | 53 | 6.5 | LOS A | 0.0 | 0.0 | 0.47 | 0.47 |
| P5 | Across N approach | 53 | 24.3 | LOS C | 0.1 | 0.1 | 0.90 | 0.90 |
| P7 | Across W approach | 53 | 6.5 | LOS A | 0.0 | 0.0 | 0.47 | 0.47 |
| All Ped | estrians | 212 | 15.4 | LOS B |  |  | 0.68 | 0.68 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Processed: Wednesday, 1 October 2014 11:05:33 AM SIDRA INTERSECTION 5.1.13.2093

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Project: P:\14S1000-1099\14S1091200 PNUR - Rezoning\ModellingISIDRAI14S1091200sid_SIDRA Future
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8000056, GTA CONSULTANTS, ENTERPRISE

14S1091200 PNUR
Future Saturday
O'Connell St/ Fennell St
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn <br> v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 188 | 0.0 | 0.299 | 8.1 | LOS A | 1.7 | 11.9 | 0.12 | 0.66 | 48.8 |
| 2 T | 648 | 0.0 | 0.299 | 6.9 | LOS A | 1.7 | 11.9 | 0.12 | 0.53 | 49.9 |
| 3 R | 11 | 0.0 | 0.299 | 11.0 | LOS A | 1.7 | 11.8 | 0.12 | 0.84 | 46.3 |
| Approach | 847 | 0.0 | 0.299 | 7.2 | LOS A | 1.7 | 11.9 | 0.12 | 0.57 | 49.6 |
| East: Fennell Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 13 | 0.0 | 0.042 | 12.6 | LOS A | 0.2 | 1.4 | 0.69 | 0.79 | 44.6 |
| 5 T | 7 | 0.0 | 0.042 | 11.8 | LOS A | 0.2 | 1.4 | 0.69 | 0.76 | 44.9 |
| 6 R | 2 | 0.0 | 0.042 | 15.9 | LOS B | 0.2 | 1.4 | 0.69 | 0.86 | 42.4 |
| Approach | 22 | 0.0 | 0.042 | 12.6 | LOS A | 0.2 | 1.4 | 0.69 | 0.79 | 44.5 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 9 | 0.0 | 0.408 | 9.3 | LOS A | 2.5 | 17.7 | 0.43 | 0.71 | 47.9 |
| 8 T | 868 | 0.0 | 0.408 | 8.1 | LOS A | 2.5 | 17.7 | 0.43 | 0.61 | 48.2 |
| 9 R | 23 | 0.0 | 0.408 | 12.2 | LOS A | 2.5 | 17.7 | 0.43 | 0.80 | 45.7 |
| Approach | 901 | 0.0 | 0.408 | 8.2 | LOS A | 2.5 | 17.7 | 0.43 | 0.62 | 48.1 |
| West: Fennell Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 27 | 0.0 | 0.285 | 11.2 | LOS A | 1.6 | 11.4 | 0.63 | 0.77 | 45.5 |
| 11 T | 16 | 0.0 | 0.285 | 10.5 | LOS A | 1.6 | 11.4 | 0.63 | 0.74 | 45.8 |
| 12 R | 172 | 0.0 | 0.285 | 14.5 | LOS B | 1.6 | 11.4 | 0.63 | 0.83 | 43.2 |
| Approach | 215 | 0.0 | 0.285 | 13.8 | LOS A | 1.6 | 11.4 | 0.63 | 0.81 | 43.6 |
| All Vehicles | 1985 | 0.0 | 0.408 | 8.4 | LOS A | 2.5 | 17.7 | 0.32 | 0.62 | 48.1 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 1 October 2014 11:44:31 AM SIDRA INTERSECTION 5.1.13.2093
Project: P:\14S1000-1099\14S1091200 PNUR - Rezoning\ModellingISIDRAI14S1091200sid_SIDRA Future
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14S1091200 PNUR
Future Saturday
O'Connell St/ Barney St
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 94 | 0.0 | 0.533 | 9.6 | LOS A | 5.0 | 35.1 | 0.63 | 0.66 | 46.9 |
| 2 | T | 343 | 0.0 | 0.533 | 8.9 | LOS A | 5.0 | 35.1 | 0.63 | 0.62 | 46.9 |
| 3 | R | 139 | 0.0 | 0.533 | 12.9 | LOS A | 5.0 | 35.1 | 0.63 | 0.73 | 45.0 |
| Approac |  | 576 | 0.0 | 0.533 | 10.0 | LOS A | 5.0 | 35.1 | 0.63 | 0.65 | 46.4 |
| East: Barney Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 505 | 0.0 | 0.683 | 11.3 | LOS A | 7.2 | 50.2 | 0.58 | 0.72 | 45.7 |
| 5 | T | 113 | 0.0 | 0.683 | 10.5 | LOS A | 7.2 | 50.2 | 0.58 | 0.68 | 46.1 |
| 6 | R | 92 | 0.0 | 0.683 | 14.6 | LOS B | 7.2 | 50.2 | 0.58 | 0.80 | 43.4 |
| Approac |  | 709 | 0.0 | 0.683 | 11.6 | LOS A | 7.2 | 50.2 | 0.58 | 0.73 | 45.5 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 9 | 0.0 | 0.121 | 9.5 | LOS A | 0.7 | 5.0 | 0.52 | 0.67 | 47.6 |
| 8 | T | 102 | 0.0 | 0.121 | 8.7 | LOS A | 0.7 | 5.0 | 0.52 | 0.62 | 47.7 |
| 9 | R | 1 | 0.0 | 0.121 | 12.8 | LOS A | 0.7 | 5.0 | 0.52 | 0.78 | 45.2 |
| Approac |  | 113 | 0.0 | 0.121 | 8.8 | LOS A | 0.7 | 5.0 | 0.52 | 0.63 | 47.7 |
| West: New Road From Development |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 34 | 0.0 | 0.258 | 12.3 | LOS A | 1.8 | 12.3 | 0.77 | 0.80 | 44.4 |
| 11 | T | 9 | 0.0 | 0.258 | 11.5 | LOS A | 1.8 | 12.3 | 0.77 | 0.78 | 44.7 |
| 12 | R | 136 | 0.0 | 0.258 | 15.6 | LOS B | 1.8 | 12.3 | 0.77 | 0.84 | 42.3 |
| Approac |  | 179 | 0.0 | 0.258 | 14.8 | LOS B | 1.8 | 12.3 | 0.77 | 0.83 | 42.8 |
| All Vehicles |  | 1577 | 0.0 | 0.683 | 11.2 | LOS A | 7.2 | 50.2 | 0.62 | 0.70 | 45.6 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 1 October 2014 11:34:13 AM SIDRA INTERSECTION 5.1.13.2093
Project: P:\14S1000-1099\14S1091200 PNUR - Rezoning\ModellingISIDRAI14S1091200sid SIDRA Future
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## 14S1091200 PNUR

Future Saturday
O'Connell St/ Barney St
Signals - Fixed Time Cycle Time $=60$ seconds (Practical Cycle Time)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. <br> Satn <br> v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 94 | 0.0 | 0.485 | 20.2 | LOS B | 8.9 | 62.4 | 0.74 | 0.89 | 40.3 |
| 2 | T | 343 | 0.0 | 0.485 | 12.0 | LOS A | 8.9 | 62.4 | 0.74 | 0.64 | 42.2 |
| 3 | R | 139 | 0.0 | 0.249 | 20.8 | LOS B | 2.7 | 18.9 | 0.77 | 0.77 | 38.0 |
| Approac |  | 576 | 0.0 | 0.485 | 15.5 | LOS B | 8.9 | 62.4 | 0.74 | 0.71 | 40.8 |
| East: Barney Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 505 | 0.0 | 0.466 | 15.9 | LOS B | 8.5 | 59.6 | 0.61 | 0.80 | 41.7 |
| 5 | T | 113 | 0.0 | 0.478 | 16.9 | LOS B | 4.7 | 32.9 | 0.81 | 0.67 | 37.6 |
| 6 | R | 92 | 0.0 | 0.478 | 25.3 | LOS B | 4.7 | 32.9 | 0.81 | 0.84 | 36.4 |
| Approac |  | 709 | 0.0 | 0.478 | 17.3 | LOS B | 8.5 | 59.6 | 0.67 | 0.78 | 40.2 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 9 | 0.0 | 0.273 | 30.0 | LOS C | 2.8 | 19.9 | 0.87 | 0.82 | 34.6 |
| 8 | T | 102 | 0.0 | 0.273 | 21.8 | LOS B | 2.8 | 19.9 | 0.87 | 0.69 | 35.4 |
| 9 | R | 1 | 0.0 | 0.273 | 30.1 | LOS C | 2.8 | 19.9 | 0.87 | 0.83 | 34.6 |
| Approac |  | 113 | 0.0 | 0.273 | 22.6 | LOS B | 2.8 | 19.9 | 0.87 | 0.70 | 35.3 |
| West: New Road From Development |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 34 | 0.0 | 0.069 | 23.0 | LOS B | 0.9 | 6.1 | 0.71 | 0.75 | 37.1 |
| 11 | T | 9 | 0.0 | 0.069 | 14.8 | LOS B | 0.9 | 6.1 | 0.71 | 0.53 | 38.8 |
| 12 | R | 136 | 0.0 | 0.331 | 22.3 | LOS B | 2.9 | 20.1 | 0.73 | 0.79 | 37.1 |
| Approach |  | 179 | 0.0 | 0.331 | 22.0 | LOS B | 2.9 | 20.1 | 0.73 | 0.77 | 37.2 |
| All Vehicles |  | 1577 | 0.0 | 0.485 | 17.5 | LOS B | 8.9 | 62.4 | 0.72 | 0.75 | 39.7 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | ---: | :---: | ---: | ---: | ---: | ---: | ---: |
| Mov ID | Description | Demand <br> Flow <br> ped/h | Average <br> Delay <br> sec | Level of <br> Service | Average Back of Queue <br> Pedestrian <br> ped | Prop. <br> Distance <br> Queued | Effective <br> Stop Rate <br> per ped |  |
| P1 | Across S approach | 53 | 18.4 | LOS B | 0.1 | 0.1 | 0.78 | 0.78 |
| P3 | Across E approach | 53 | 24.3 | LOS C | 0.1 | 0.1 | 0.90 | 0.90 |
| P5 | Across N approach | 53 | 16.9 | LOS B | 0.1 | 0.1 | 0.75 | 0.75 |
| P7 | Across W approach | 53 | 24.3 | LOS C | 0.1 | 0.1 | 0.90 | 0.90 |
| All Pedestrians | 212 | 21.0 | LOS C |  |  | 0.83 | 0.83 |  |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

14S1091200 PNUR
Future Saturday
O'Connell St/ Dunlop St
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | f Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street South |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 31 | 0.0 | 0.403 | 7.7 | LOS A | 3.1 | 22.0 | 0.18 | 0.61 | 48.9 |
| 2 | T | 552 | 0.0 | 0.403 | 6.9 | LOS A | 3.1 | 22.0 | 0.18 | 0.53 | 49.6 |
| 3 | R | 13 | 0.0 | 0.403 | 11.0 | LOSA | 3.1 | 22.0 | 0.18 | 0.80 | 46.3 |
| Approa |  | 595 | 0.0 | 0.403 | 7.1 | LOSA | 3.1 | 22.0 | 0.18 | 0.54 | 49.4 |
| East: Dunlop Street |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 9 | 0.0 | 0.043 | 13.6 | LOS A | 0.3 | 1.8 | 0.77 | 0.75 | 43.8 |
| 5 | T | 14 | 0.0 | 0.043 | 12.9 | LOS A | 0.3 | 1.8 | 0.77 | 0.73 | 44.0 |
| 6 | R | 1 | 0.0 | 0.043 | 17.0 | LOS B | 0.3 | 1.8 | 0.77 | 0.80 | 41.8 |
| Approa |  | 24 | 0.0 | 0.043 | 13.3 | LOS A | 0.3 | 1.8 | 0.77 | 0.74 | 43.8 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 5 | 0.0 | 0.560 | 8.3 | LOS A | 6.0 | 42.2 | 0.44 | 0.59 | 47.9 |
| 8 | T | 728 | 0.0 | 0.560 | 7.5 | LOS A | 6.0 | 42.2 | 0.44 | 0.53 | 48.1 |
| 9 | R | 16 | 0.0 | 0.560 | 11.6 | LOS A | 6.0 | 42.2 | 0.44 | 0.72 | 46.0 |
| Approa |  | 749 | 0.0 | 0.560 | 7.6 | LOS A | 6.0 | 42.2 | 0.44 | 0.54 | 48.1 |
| West: Dunlop Street |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 20 | 0.0 | 0.109 | 11.8 | LOS A | 0.7 | 4.9 | 0.68 | 0.72 | 45.2 |
| 11 | T | 34 | 0.0 | 0.109 | 11.1 | LOS A | 0.7 | 4.9 | 0.68 | 0.69 | 45.5 |
| 12 | R | 29 | 0.0 | 0.109 | 15.1 | LOS B | 0.7 | 4.9 | 0.68 | 0.78 | 43.0 |
| Approach |  | 83 | 0.0 | 0.109 | 12.7 | LOS A | 0.7 | 4.9 | 0.68 | 0.73 | 44.5 |
| All Vehi |  | 1452 | 0.0 | 0.560 | 7.8 | LOS A | 6.0 | 42.2 | 0.35 | 0.55 | 48.3 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 1 October 2014 11:39:18 AM SIDRA INTERSECTION 5.1.13.2093
Project: P:\14S1000-1099\14S1091200 PNUR - RezoningIModellingISIDRAI14S1091200sid SIDRA Future
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## 14S1091200 PNUR

Future Saturday
O'Connell St/ Factory St
Signals - Fixed Time Cycle Time $=65$ seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \text { v/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back o Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
|  |  |  |  |  |  |  |  |  |  |  |
| 1 L | 42 | 0.0 | 0.500 | 14.6 | LOS B | 10.4 | 72.9 | 0.56 | 0.96 | 44.7 |
| 2 T | 586 | 0.0 | 0.500 | 6.5 | LOS A | 10.4 | 72.9 | 0.56 | 0.50 | 48.5 |
| Approach | 628 | 0.0 | 0.500 | 7.0 | LOS A | 10.4 | 72.9 | 0.56 | 0.53 | 48.2 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 41 | 0.0 | 0.322 | 34.8 | LOS C | 3.0 | 21.2 | 0.92 | 0.79 | 31.6 |
| 5 T | 63 | 0.0 | 0.322 | 26.7 | LOS B | 3.0 | 21.2 | 0.92 | 0.72 | 32.1 |
| Approach | 104 | 0.0 | 0.322 | 29.9 | LOS C | 3.0 | 21.2 | 0.92 | 0.75 | 31.9 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 165 | 0.0 | 0.617 | 15.5 | LOS B | 14.4 | 100.6 | 0.64 | 0.92 | 43.9 |
| 8 T | 604 | 0.0 | 0.617 | 7.3 | LOS A | 14.4 | 100.6 | 0.64 | 0.58 | 46.8 |
| Approach | 769 | 0.0 | 0.617 | 9.0 | LOS A | 14.4 | 100.6 | 0.64 | 0.65 | 46.2 |
| West: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 11 | 0.0 | 0.395 | 35.4 | LOS C | 3.5 | 24.5 | 0.93 | 0.80 | 31.7 |
| 11 T | 92 | 0.0 | 0.395 | 27.2 | LOS B | 3.5 | 24.5 | 0.93 | 0.74 | 32.1 |
| 12 R | 16 | 0.0 | 0.395 | 35.6 | LOS C | 3.5 | 24.5 | 0.93 | 0.81 | 31.7 |
| Approach | 118 | 0.0 | 0.395 | 29.1 | LOS C | 3.5 | 24.5 | 0.93 | 0.75 | 32.0 |
| All Vehicles | 1620 | 0.0 | 0.617 | 11.1 | LOS A | 14.4 | 100.6 | 0.65 | 0.62 | 44.2 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back Pedestrian ped | Queue <br> Distance m | Prop. Queued | Effective Stop Rate per ped |
| P1 | Across S approach | 53 | 26.8 | LOS C | 0.1 | 0.1 | 0.91 | 0.91 |
| P3 | Across E approach | 53 | 6.0 | LOS A | 0.0 | 0.0 | 0.43 | 0.43 |
| P5 | Across N approach | 53 | 26.8 | LOS C | 0.1 | 0.1 | 0.91 | 0.91 |
| P7 | Across W approach | 53 | 6.0 | LOS A | 0.0 | 0.0 | 0.43 | 0.43 |
| All Pede | estrians | 212 | 16.4 | LOS B |  |  | 0.67 | 0.67 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

14S1091200 PNUR
Future Saturday
O'Connell St/ Factory St
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 1 L | 42 | 0.0 | 0.464 | 8.1 | LOS A | 4.1 | 28.8 | 0.30 | 0.60 | 48.5 |
| 2 T | 586 | 0.0 | 0.464 | 7.3 | LOS A | 4.1 | 28.8 | 0.30 | 0.53 | 48.9 |
| 3 R | 1 | 0.0 | 0.464 | 11.4 | LOS A | 4.1 | 28.8 | 0.30 | 0.76 | 46.1 |
| Approach | 629 | 0.0 | 0.464 | 7.4 | LOS A | 4.1 | 28.8 | 0.30 | 0.53 | 48.9 |
| East: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 41 | 0.0 | 0.163 | 12.4 | LOS A | 0.9 | 6.1 | 0.60 | 0.78 | 44.9 |
| 5 T | 63 | 0.0 | 0.163 | 11.7 | LOS A | 0.9 | 6.1 | 0.60 | 0.74 | 45.2 |
| 6 R | 1 | 0.0 | 0.163 | 15.7 | LOS B | 0.9 | 6.1 | 0.60 | 0.86 | 42.6 |
| Approach | 105 | 0.0 | 0.163 | 12.0 | LOS A | 0.9 | 6.1 | 0.60 | 0.75 | 45.1 |
| North: O'Connell Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 165 | 0.0 | 0.614 | 8.8 | LOS A | 7.0 | 49.2 | 0.58 | 0.60 | 47.3 |
| 8 T | 604 | 0.0 | 0.614 | 8.0 | LOS A | 7.0 | 49.2 | 0.58 | 0.56 | 47.3 |
| 9 R | 1 | 0.0 | 0.614 | 12.1 | LOS A | 7.0 | 49.2 | 0.58 | 0.70 | 45.7 |
| Approach | 771 | 0.0 | 0.614 | 8.2 | LOS A | 7.0 | 49.2 | 0.58 | 0.57 | 47.3 |
| West: Factory Street |  |  |  |  |  |  |  |  |  |  |
| 10 L | 11 | 0.0 | 0.160 | 12.3 | LOS A | 1.1 | 7.9 | 0.73 | 0.74 | 45.1 |
| 11 T | 92 | 0.0 | 0.160 | 11.5 | LOS A | 1.1 | 7.9 | 0.73 | 0.71 | 45.3 |
| 12 R | 16 | 0.0 | 0.160 | 15.6 | LOS B | 1.1 | 7.9 | 0.73 | 0.80 | 42.9 |
| Approach | 118 | 0.0 | 0.160 | 12.2 | LOS A | 1.1 | 7.9 | 0.73 | 0.73 | 44.9 |
| All Vehicles | 1623 | 0.0 | 0.614 | 8.4 | LOS A | 7.0 | 49.2 | 0.48 | 0.58 | 47.6 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

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## Appendix C

## Post Development Intersection Tuming Movement Diagrams








## Appendix D

## Appendix D

Linsig Modelling Process

Appendix D

## D. 1 Modelling Process

The LinSig models were built with the aims of representing the existing traffic conditions and forming the basis for the future development and optionstesting. These models were calibrated using RMS provided and site-measured data to ensure that any proposed scenario can be fully compared to a reliable baseline. On-site observationswere also undertaken in order to get better understanding of the network operation in the study area.

## D.1.1 Software

The LinSig models were built using LinSig version 3.2.
LinSig is a computer software package for the assessment and design of traffic signal intersections either individually or as a network of multiple intersections. It is generally used to construct a model of the intersection or network which can then be used to assess different designs and methods of operation. It can also be used to optimise traffic signal timings and offsets for individual intersections or at network level on the basis of traffic delay or Practical Reserve Capacity (PRC).

LinSig is best suited to the a ssessment of smaller networks; where the modelled intersections are operate in the same SCATS sub-system with similar cycle timing. For a larger comidor it can be split into separate LinSig models to remain appropriate for use. The coridor splitting is normally aligned with SCATS sub-systems and therefore does not compromise the evaluation procedure.

## D.1.2 Model Extents

The modelled intersections are shown in Figure D- 1.
Existing intersection operations in PNUR was modelled using a combination of LinSig 3.2 and SIDRA Intersection 5.1. The breakup of the existing study area intersections was selected using existing SC ATS linking data as well as consideration of the traffic streams and implic ation of the traffic queue on the road comidors. Other signalised/ prionity controlled intersections which not covered in the LinSig models were then modelled with SIDRA intersection and assessed individually.

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Appendix D

Figure D- 1: Model Coverage


Figure D- 2: Model Screenshot-Part 1. Panamatta North Modelling - Church Street North


Figure D- 3: Model Screenshot - Part 2. Panramatta North - Pennant Hills Road


Figure D- 4: Model Screenshot-Part 3. Parramatta North - Victoria Road


Appendix D

## D. 2 Modelling Development

## D.2.1 Base Assumptions

For the purpose of the LinSig analysis the following key assumptions have been made:

- Given that traffic flow in LinSig are represented in passenger car units (pcu), the following conversion was adopted forthe existing traffic volumes:
- Car=1pcu
- Bus=2pcu
- Heavy vehicle $=2 p c u$.
- Standard LinSig saturation flow values of 1,800 were generally adopted for through and tuming lanes respec tively.
- Lane lengths have been based on the existing intersection layout with short la nes used to represent how road space is currently used.
- Phase sequence arrangements, durations and cycle time for the existing models were input based on SCATS IDM and offset information provided by RMS, as well as a site inspection of the study coridor.
- Phase inter-green (i.e. combined red and amber) times of 6 sec onds have been applied to all intersections, except atJ a mes Rules Drive, Windsor Road Interchange.
- A start lag of 4 seconds has been applied to left-tuming vehic le movements that run simultaneously with pedestrian movements to represent the delay to vehiclescaused by pedestrians.
- De-sliverqueue thresholds were adopted to ensure that LinSig reported the realistic queue length results.


## D.2.2 Data

The models were developed and calibrated using the following data provided by RMS and AusTraffic:

- Tuming movement count data for all signalised intersectionsidentified in Figure D-1and selected priority controlled intersections. Count data was provided for a typical Thursday from 7:00am to 9:00am, 4:00pm to 6:00pm and for a typic al Saturday from 12:00pm to $2: 00 \mathrm{pm}$.
- Queue length data at all tuming movement count locations for the same time periods.
- SCATSIDM data.
- SCATS linking and offset information.

In addition, site visits were undertaken to observe the intersection performance and general traffic behaviour within the study area and ensuring that the models have been coded to accurately represent operating conditions.

Appendix D

## D.2.3 Temporal Coverage

After a review of the data for each intersection, the following peak hour were determined for the study a rea and applied to all models in the study area:

- Thursday AM Peak: 7:45am to 8:45am
- Thursday PM Peak: 4:30pm to 5:30pm
- Saturday Peak: 12:00pm to 1:00pm


## D.2.4 Modelling Scenarios

Three different scenarios were modelled in this assessment; existing condition, future scenario underexisting configurations and future scenario with conceptual improvement upgrades.

The existing condition wasmodelled with LinSig delay based traffic assignment method and calibrated to represent the on-site operating conditions. The existing condition model is then used as the base model for future model and options testing.

The future scenario (under existing configuration) was modelled based on the base model with future development trips and general traffic growth added on the existing network. The phasing of model was optimised to represent the variation on phase time due to the increase of traffic volume in the network.

Future scenario with conceptual improvement upgrades scenario was modelled based on future scenario and tested with conceptual upgrades. The purpose of this scenario isto provide conceptual improvements in orderto accommodate the potential future traffic in the study area.

## D. 3 Modelling Results

Network performance of the intersections in the study area is provided in Appendix E.
The results in Appendix E also summa rise the observed and modelled queue length and comparison with different scenarios.

## Appendix E

Linsig Modelling Results

Table 1. LinSig results Summary - Existing Condition

| Intersections |  | Approach | Existing Thursday AM Peak |  |  |  | Existing Thursday PM Peak |  |  |  | Existing Saturday Peak |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Level of Service | Average Delay (sec) | Modelled Queue (m) | Observed Max Queue (m) | Level of Service | Average Delay (sec) | Modelled Queue (m) | Observed Max Queue (m) | Level of Service | Average Delay (sec) | Modelled Queue (m) | Observed Max Queue (m) |
|  | 1. Windsor Road and Cumberland Highway |  | North - Windsor Road <br> East - James Rules Drive <br> South - Windsor Road <br> West - Cumberland Highway | F | 73 | $\begin{gathered} 155 \\ 129 \\ 93 \\ 93 \end{gathered}$ | $\begin{gathered} 210 \\ 102 \\ 66 \\ 198 \\ \hline \end{gathered}$ | F | 119 | $\begin{aligned} & 164 \\ & 311 \\ & 196 \\ & 144 \end{aligned}$ | $\begin{aligned} & 162 \\ & 114+ \\ & 108 \\ & 126 \\ & \hline \end{aligned}$ | F | 83 | $\begin{gathered} 128 \\ 94 \\ 115 \\ 178 \\ \hline \end{gathered}$ | $\begin{gathered} \hline 270 \\ 108 \\ 60 \\ 126 \\ \hline \end{gathered}$ |
|  | 22. Church Street and The Junction Access | North - Windsor Road <br> South - Windsor Road <br> West - The Junction Access | A | 10 | $\begin{aligned} & 98 \\ & 31 \\ & 37 \\ & \hline \end{aligned}$ | $\begin{aligned} & 24 \\ & 72 \\ & 30 \\ & \hline \end{aligned}$ | B | 19 | $\begin{gathered} 74 \\ 127 \\ 41 \\ \hline \end{gathered}$ | $\begin{aligned} & 66 \\ & 90 \\ & 48 \\ & \hline \end{aligned}$ | B | 19 | $\begin{gathered} 72 \\ 120 \\ 31 \\ \hline \end{gathered}$ | $\begin{aligned} & 60 \\ & 78 \\ & 36 \\ & \hline \end{aligned}$ |
|  | 2. Church Street and North Rocks Road | North - Windsor Road East - North Rocks Road South - Church Street | D | 50 | $\begin{gathered} 141 \\ 167 \\ 46 \\ \hline \end{gathered}$ | $\begin{gathered} 102 \\ 246 \\ 60 \\ \hline \end{gathered}$ | B | 26 | $\begin{aligned} & 53 \\ & 94 \\ & 57 \\ & \hline \end{aligned}$ | $\begin{gathered} 90 \\ 96 \\ 180 \\ \hline \end{gathered}$ | C | 33 | $\begin{aligned} & 89 \\ & 109 \\ & 37 \\ & \hline \end{aligned}$ | $\begin{aligned} & 96 \\ & 84 \\ & 48 \\ & \hline \end{aligned}$ |
|  | 4. Church Street, Barney Street | North - Church Street East - Barney Street South - Church Street West - Barney Street | C | 40 | $\begin{gathered} \hline 107 \\ 36 \\ 53 \\ 10 \\ \hline \end{gathered}$ | $\begin{aligned} & \hline 102 \\ & 78 \\ & 72 \\ & 18 \\ & \hline \end{aligned}$ | D | 47 | $\begin{gathered} 94 \\ 71 \\ 104 \\ 81 \\ \hline \end{gathered}$ | $\begin{aligned} & \hline 60 \\ & 72 \\ & 102 \\ & 78 \\ & \hline \end{aligned}$ | C | 33 | $\begin{aligned} & 55 \\ & 41 \\ & 69 \\ & 23 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 78 \\ & 36 \\ & 78 \\ & 36 \\ & \hline \end{aligned}$ |
|  | 9. Church Street and Factory Street | North - Church Street East - Factory Street South - Church Street West-Factory Street | B | 16 | $\begin{gathered} \hline 115 \\ 6 \\ 17 \\ 6 \\ \hline \end{gathered}$ | $\begin{aligned} & 108 \\ & 24 \\ & 54 \\ & 12 \\ & \hline \end{aligned}$ | A | 13 | $\begin{gathered} 53 \\ 6 \\ 86 \\ 6 \\ \hline \end{gathered}$ | $\begin{aligned} & \hline 108 \\ & 24 \\ & 84 \\ & 18 \\ & \hline \end{aligned}$ | A | 14 | $\begin{gathered} \hline 69 \\ 6 \\ 35 \\ 6 \\ \hline \end{gathered}$ | $\begin{aligned} & \hline 54 \\ & 18 \\ & 90 \\ & 24 \\ & \hline \end{aligned}$ |
|  | 11. Church Street, Albert Street and Pennant Hills Road | North - Church Street NorthEast - Pennant Hills Road South - Church Street West - Albert Street | C | 40 | $\begin{aligned} & \hline 127 \\ & 72 \\ & 81 \\ & 23 \\ & \hline \end{aligned}$ | $\begin{aligned} & 12 \\ & 78 \\ & 36 \\ & 66 \\ & 60 \end{aligned}$ | B | 29 | $\begin{aligned} & 98 \\ & 52 \\ & 201 \\ & 23 \\ & \hline \end{aligned}$ | $\begin{aligned} & 72 \\ & 70 \\ & 78 \\ & 42 \end{aligned}$ | B | 26 | $\begin{aligned} & 86 \\ & 43 \\ & 37 \\ & 13 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 54 \\ & 36 \\ & 66 \\ & 24 \\ & 24 \end{aligned}$ |
|  | 14. Church Street and Grose Street | North - Church Street East - Grose Street South - Church Street West - Grose Street | C | 35 | $\begin{array}{r} 127 \\ 37 \\ 48 \\ 46 \\ \hline \end{array}$ | $\begin{aligned} & \hline 144 \\ & 66 \\ & 36 \\ & 42 \\ & \hline \end{aligned}$ | C | 30 | $\begin{gathered} 40 \\ 23 \\ 138 \\ 37 \\ \hline \end{gathered}$ | $\begin{gathered} \hline 114 \\ 48 \\ 66 \\ 30 \\ \hline \end{gathered}$ | B | 24 | $\begin{aligned} & 58 \\ & 23 \\ & 69 \\ & 23 \\ & \hline \end{aligned}$ | $\begin{aligned} & 150 \\ & 60 \\ & 66 \\ & 30 \\ & \hline \end{aligned}$ |
|  | 10. O'Connell Street and Albert Street | North - O'Connell Street <br> East - Albert Street <br> South - O'Connell Street <br> West - Albert Street | B | 19 | $\begin{gathered} \hline 48 \\ 49 \\ 17 \\ 2 \\ \hline \end{gathered}$ | $\begin{aligned} & \hline 84 \\ & 54 \\ & 30 \\ & 6 \\ & \hline \end{aligned}$ | A | 14 | $\begin{aligned} & 22 \\ & 35 \\ & 26 \\ & 2 \\ & \hline \end{aligned}$ | $\begin{aligned} & 30 \\ & 48 \\ & 54 \\ & 18 \\ & \hline \end{aligned}$ | B | 17 | $\begin{gathered} 25 \\ 33 \\ 12 \\ 2 \\ \hline \end{gathered}$ | $\begin{gathered} \hline 60 \\ 48 \\ 30 \\ 6 \\ \hline \end{gathered}$ |
|  | 13. O'Connell Street and Grose Street | North - O'Connell Street East - Grose Street South - O'Connell Street West - Grose Street | B | 21 | $\begin{aligned} & 58 \\ & 20 \\ & 40 \\ & 6 \\ & \hline \end{aligned}$ | $\begin{aligned} & 60 \\ & 24 \\ & 48 \\ & 6 \\ & \hline \end{aligned}$ | B | 21 | $\begin{aligned} & \hline 32 \\ & 22 \\ & 69 \\ & 12 \\ & \hline \end{aligned}$ | $\begin{aligned} & 54 \\ & 30 \\ & 60 \\ & 24 \\ & \hline \end{aligned}$ | B | 17 | $\begin{aligned} & 31 \\ & 20 \\ & 37 \\ & 12 \\ & \hline \end{aligned}$ | $\begin{aligned} & 48 \\ & 24 \\ & 36 \\ & 66 \\ & \hline \end{aligned}$ |
|  | 15. O'Connell Street and Victoria Road | North - O'Connell Street <br> East - Victoria Road <br> South - O'Connell Street <br> West - Stadium Carpark Access | C | 31 | $\begin{gathered} \hline 15 \\ 75 \\ 138 \\ 3 \\ \hline \end{gathered}$ | $\begin{gathered} 90 \\ 78 \\ 162 \\ 18 \\ \hline \end{gathered}$ | C | 30 | $\begin{gathered} \hline 81 \\ 129 \\ 104 \\ 6 \\ \hline \end{gathered}$ | $\begin{aligned} & 78 \\ & 78 \\ & 108 \\ & 12 \\ & \hline \end{aligned}$ | C | 35 | $\begin{aligned} & \hline 98 \\ & 81 \\ & 95 \\ & 3 \\ & \hline \end{aligned}$ | $\begin{aligned} & 72 \\ & 84 \\ & 42 \\ & 6 \\ & \hline \end{aligned}$ |
|  | 18. Victoria Road and Marsden Street | $\begin{aligned} & \hline \text { North - Villiers Street } \\ & \text { East - Victoria Road } \\ & \text { South - Marsden Street } \\ & \text { West - Victoria Road } \\ & \hline \end{aligned}$ | C | 37 | $\begin{aligned} & 20 \\ & 26 \\ & 32 \\ & 75 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 36 \\ & 90 \\ & 54 \\ & 30 \\ & \hline \end{aligned}$ | D | 51 | $\begin{gathered} \hline 17 \\ 115 \\ 58 \\ 81 \\ \hline \end{gathered}$ | $\begin{gathered} \hline 30 \\ 108 \\ 90 \\ 42 \\ \hline \end{gathered}$ | C | 39 | $\begin{aligned} & 13 \\ & 71 \\ & 46 \\ & 58 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 54 \\ & 90 \\ & 78 \\ & 48 \end{aligned}$ |
|  | 16. Church Street and Victoria Road | North - Church Street East - Victoria Road South - Church Street West - Victoria Road | C | 32 | $\begin{aligned} & 86 \\ & 95 \\ & 35 \\ & 46 \\ & \hline \end{aligned}$ | $\begin{aligned} & 54 \\ & 66 \\ & 42 \\ & 66 \\ & \hline \end{aligned}$ | D | 49 | $\begin{aligned} & \hline 86 \\ & 109 \\ & 40 \\ & 98 \\ & \hline \end{aligned}$ | $\begin{aligned} & 72 \\ & 60 \\ & 54 \\ & 78 \\ & \hline \end{aligned}$ | E | 62 | $\begin{aligned} & 78 \\ & 37 \\ & 23 \\ & 63 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 66 \\ & 54 \\ & 30 \\ & 72 \\ & \hline \end{aligned}$ |
|  | 17. Victoria Road and Wilde Avenue | East - Victoria Road <br> South - Wilde Avenue <br> West - Victoria Road | C | 40 | $\begin{gathered} \hline 104 \\ 46 \\ 121 \\ \hline \end{gathered}$ | $\begin{aligned} & \hline 96 \\ & 84 \\ & 72 \end{aligned}$ | C | 34 | $\begin{aligned} & 81 \\ & 86 \\ & 40 \\ & \hline \end{aligned}$ | $\begin{aligned} & 90 \\ & 90 \\ & 94 \\ & \hline \end{aligned}$ | B | 25 | $\begin{aligned} & \hline 52 \\ & 36 \\ & 18 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 60 \\ & 54 \\ & 48 \\ & \hline \end{aligned}$ |

Table 2. LinSig results Summary - Future Scenario

| Intersections |  | Approach | Future Scenario - Thursday AM Peak |  |  | Future Scenario - Thursday PM Peak |  |  | Future Scenario - Saturday Peak |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Level of Service | Average Delay (sec) | Modelled Queue (m) | Level of Service | Average Delay (sec) | Modelled Queue (m) | Level of Service | Average Delay (sec) | Modelled Queue (m) |
|  | 1. Windsor Road and Cumberland Highway |  | North - Windsor Road East - James Rules Drive South - Windsor Road West - Cumberland Highway | F | 109 | $\begin{aligned} & 272 \\ & 191 \\ & 185 \\ & 198 \\ & \hline \end{aligned}$ | F | 150 | $\begin{aligned} & 386 \\ & 171 \\ & 270 \\ & 308 \\ & \hline \end{aligned}$ | F | 106 | $\begin{aligned} & 432 \\ & 153 \\ & 129 \\ & 116 \\ & \hline \end{aligned}$ |
| 㐫 | 22. Church Street and The Junction Access | North - Windsor Road <br> South - Windsor Road <br> West - The Junction Access | A | 10 | $\begin{gathered} 119 \\ 59 \\ 31 \\ \hline \end{gathered}$ | A | 13 | $\begin{gathered} 17 \\ 126 \\ 40 \\ \hline \end{gathered}$ | A | 15 | $\begin{aligned} & 40 \\ & 90 \\ & 29 \\ & \hline \end{aligned}$ |
| 를 | 2. Church Street and North Rocks Road | North - Windsor Road East - North Rocks Road South - Church Street | D | 47 | $\begin{gathered} 189 \\ 127 \\ 85 \\ \hline \end{gathered}$ | B | 26 | $\begin{aligned} & 90 \\ & 71 \\ & 82 \\ & \hline \end{aligned}$ | C | 29 | $\begin{aligned} & 93 \\ & 71 \\ & 46 \\ & \hline \end{aligned}$ |
| $\underset{\underline{2}}{\underset{\sim}{2}}$ | 4. Church Street, Barney Street | $\begin{aligned} & \text { North - Church Street } \\ & \text { East - Barney Sreet } \\ & \text { South - Church Street } \\ & \text { West - Barney Street } \end{aligned}$ | C | 39 | $\begin{gathered} 137 \\ 36 \\ 98 \\ 12 \end{gathered}$ | F | 109 | $\begin{aligned} & \hline 142 \\ & 101 \\ & 447 \\ & 74 \\ & \hline \end{aligned}$ | C | 40 | $\begin{aligned} & \hline 97 \\ & 49 \\ & 86 \\ & 20 \\ & \hline \end{aligned}$ |
|  | 9. Church Street and Factory Street | North - Church Street East - Factory Street South - Church Street West - Factory Street | F | 139 | $\begin{gathered} 167 \\ 6 \\ 29 \\ 437 \\ \hline \end{gathered}$ | B | 21 | $\begin{aligned} & \hline 68 \\ & 6 \\ & 49 \\ & 16 \\ & \hline \end{aligned}$ | B | 25 | $\begin{aligned} & \hline 91 \\ & 6 \\ & 33 \\ & 40 \\ & \hline \end{aligned}$ |
|  | 11. Church Street, Albert Street and Pennant Hills Road | North - Church Street NorthEast - Pennant Hills Road South - Church Street West - Albert Street | F | 148 | $\begin{aligned} & 788 \\ & \hline 725 \\ & 82 \\ & 32 \\ & \hline \end{aligned}$ | F | 173 | $\begin{gathered} \hline 71 \\ \hline 569 \\ 564 \\ 26 \\ \hline \end{gathered}$ | C | 35 | $\begin{aligned} & \hline 98 \\ & 93 \\ & 51 \\ & 19 \\ & \hline \end{aligned}$ |
|  | 14. Church Street and Grose Street | North - Church Street East - Grose Street South - Church Street West-Grose Street | D | 54 | $\begin{aligned} & \hline 273 \\ & 46 \\ & 58 \\ & 72 \\ & \hline \end{aligned}$ | E | 71 | $\begin{gathered} 68 \\ 95 \\ 267 \\ 74 \\ \hline \end{gathered}$ | B | 28 | $\begin{aligned} & 86 \\ & 26 \\ & 87 \\ & 25 \\ & \hline \end{aligned}$ |
|  | 10. O'Connell Street and Albert Street | North - O'Connell Street <br> East - Albert Street South - O'Connell Street <br> West - Albert Street | B | 28 | $\begin{aligned} & 75 \\ & 42 \\ & 36 \\ & 43 \\ & \hline \end{aligned}$ | B | 24 | $\begin{aligned} & 29 \\ & 56 \\ & 44 \\ & 12 \\ & \hline \end{aligned}$ | B | 22 | $\begin{aligned} & \hline 38 \\ & 30 \\ & 25 \\ & 23 \\ & \hline \end{aligned}$ |
|  | 13. O'Connell Street and Grose Street | North - O'Connell Street East - Grose Street South - O'Connell Street West - Grose Street | B | 19 | $\begin{aligned} & 44 \\ & 35 \\ & 33 \\ & 12 \\ & \hline \end{aligned}$ | B | 21 | $\begin{aligned} & \hline 35 \\ & 32 \\ & 76 \\ & 26 \\ & \hline \end{aligned}$ | A | 13 | $\begin{aligned} & 32 \\ & 21 \\ & 41 \\ & 9 \\ & \hline \end{aligned}$ |
|  | 15. O'Connell Street and Victoria Road | North - O'Connell Street <br> East - Victoria Road <br> South - O'Connell Street <br> West - Stadium Carpark Access | F | 72 | $\begin{aligned} & \hline 204 \\ & 117 \\ & 231 \\ & 14 \end{aligned}$ | C | 32 | $\begin{aligned} & 97 \\ & 98 \\ & 134 \\ & 40 \\ & \hline \end{aligned}$ | B | 26 | $\begin{aligned} & \hline 93 \\ & 86 \\ & 71 \\ & 6 \\ & \hline \end{aligned}$ |
|  | 18. Victoria Road and Marsden Street | North - Villiers Street East - Victoria Road South - Marsden Street West - Victoria Road | C | 34 | $\begin{aligned} & 25 \\ & 41 \\ & 35 \\ & 87 \\ & \hline \end{aligned}$ | C | 41 | $\begin{aligned} & \hline 18 \\ & 40 \\ & 63 \\ & 86 \\ & \hline \end{aligned}$ | C | 34 | $\begin{aligned} & 14 \\ & 35 \\ & 45 \\ & 67 \\ & \hline \end{aligned}$ |
|  | 16. Church Street and Victoria Road | North - Church Street <br> East - Victoria Road <br> South - Church Street <br> West - Victoria Road | D | 47 | $\begin{gathered} 120 \\ 169 \\ 30 \\ 111 \end{gathered}$ | E | 69 | $\begin{aligned} & \hline 197 \\ & 193 \\ & 50 \\ & 151 \end{aligned}$ | C | 33 | $\begin{gathered} 66 \\ 129 \\ 24 \\ 66 \\ \hline \end{gathered}$ |
|  | 17. Victoria Road and Wilde Avenue | East - Victoria Road South - Wilde Avenue West - Victoria Road | C | 32 | $\begin{gathered} 99 \\ 57 \\ 135 \\ \hline \end{gathered}$ | C | 37 | $\begin{gathered} 119 \\ 94 \\ 38 \\ \hline \end{gathered}$ | B | 27 | $\begin{aligned} & 79 \\ & 39 \\ & 26 \\ & \hline \end{aligned}$ |

Table 3．LinSig results Summary－Future Scenario with conceptual upgrades

| Intersections |  | Approach | Future upgrades－Thursday AM Peak |  |  | Future upgrades－Thursday PM Peak |  |  | Future upgrades－Saturday Peak |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Level of Service | Average Delay（sec） | Modelled Queue（m） | Level of Service | Average Delay（sec） | Modelled Queue（m） | Level of Service | Average Delay（sec） | Modelled Queue（m） |
|  | 1．Windsor Road and Cumberland Highway |  | North－Windsor Road <br> East－James Rules Drive <br> South－Windsor Road <br> West－Cumberland Highway | F | 114 | $\begin{aligned} & 265 \\ & 196 \\ & 148 \\ & 204 \\ & \hline \end{aligned}$ | F | 169 | $\begin{aligned} & 380 \\ & 198 \\ & 240 \\ & 384 \\ & \hline \end{aligned}$ | F | 106 | $\begin{aligned} & 371 \\ & 158 \\ & 235 \\ & 130 \end{aligned}$ |
|  | 22．Church Street and The Junction Access | North－Windsor Road <br> South－Windsor Road <br> West－The Junction Access | A | 9 | $\begin{gathered} 109 \\ 51 \\ 31 \end{gathered}$ | A | 12 | $\begin{aligned} & 26 \\ & 69 \\ & 40 \\ & \hline \end{aligned}$ | A | 14 | $\begin{aligned} & 66 \\ & 63 \\ & 30 \\ & \hline \end{aligned}$ |
|  | 2．Church Street and North Rocks Road | North－Windsor Road East－North Rocks Road South－Church Street | D | 48 | $\begin{aligned} & \begin{array}{l} 190 \\ 143 \\ 78 \\ \hline \end{array} ⿳ ⿱ ㇒ ⿲ 丶 丶 ㇒ 冖 子 力 \end{aligned}$ | B | 26 | $\begin{array}{r} 93 \\ 68 \\ 137 \\ \hline \end{array}$ | B | 28 | $\begin{gathered} 133 \\ 81 \\ 89 \\ \hline \end{gathered}$ |
|  | 3．Church Street，Board Street and Seville Street | South－Church Street <br> West－Board Street | A | 9 | $\begin{aligned} & 10 \\ & \hline 65 \\ & \hline \end{aligned}$ | B | 17 | $\begin{aligned} & \hline 110 \\ & 95 \\ & \hline \end{aligned}$ | A | 9 | $\begin{aligned} & 41 \\ & 70 \\ & \hline \end{aligned}$ |
|  | 4．Church Street，Barney Street | North－Church Street East－Barney Street South－Church Street West－Barney Street | C | 35 | $\begin{aligned} & \hline 99 \\ & 33 \\ & 86 \\ & 12 \\ & \hline \end{aligned}$ | D | 49 | $\begin{gathered} 109 \\ 78 \\ 154 \\ 86 \\ \hline \end{gathered}$ | C | 39 | $\begin{aligned} & \hline 132 \\ & 38 \\ & 105 \\ & 20 \end{aligned}$ |
|  | 9．Church Street and Factory Street | North－Church Street East－Factory Street South－Church Street West－Factory Street | C | 35 | $\begin{aligned} & \hline 69 \\ & 6 \\ & 40 \\ & 69 \\ & \hline \end{aligned}$ | B | 21 | $\begin{aligned} & \hline 83 \\ & 6 \\ & 47 \\ & 20 \\ & \hline \end{aligned}$ | B | 25 | $\begin{gathered} \hline 91 \\ 6 \\ 33 \\ 40 \\ \hline \end{gathered}$ |
|  | 11．Church Street，Albert Street and Pennant Hills Road | North－Church Street NorthEast－Pennant Hills Road South－Church Street West－Albert Street | C | 32 | $\begin{aligned} & \hline 49 \\ & 86 \\ & 86 \\ & 29 \\ & \hline \end{aligned}$ | C | 37 | $\begin{aligned} & 132 \\ & 128 \\ & 138 \\ & 23 \\ & \hline \end{aligned}$ | C | 35 | $\begin{gathered} \hline 101 \\ 91 \\ 53 \\ 18 \\ \hline \end{gathered}$ |
|  | 14．Church Street and Grose Street | North－Church Street East－Grose Street South－Church Street West－Grose Street | B | 26 | $\begin{aligned} & 75 \\ & 39 \\ & 63 \\ & 55 \\ & \hline \end{aligned}$ | B | 26 | $\begin{aligned} & \hline 51 \\ & 24 \\ & 69 \\ & 36 \\ & \hline \end{aligned}$ | B | 28 | $\begin{aligned} & 92 \\ & 24 \\ & 88 \\ & 25 \\ & \hline \end{aligned}$ |
|  | 10．O＇Connell Street and Albert Street | North－O＇Connell Street East－Albert Street South－O＇Connell Street West－Albert Street | C | 29 | $\begin{aligned} & 75 \\ & 40 \\ & 35 \\ & 43 \end{aligned}$ | B | 24 | $\begin{aligned} & \hline 29 \\ & 58 \\ & 38 \\ & 12 \\ & \hline \end{aligned}$ | B | 22 | $\begin{aligned} & \hline 37 \\ & 30 \\ & 24 \\ & 23 \\ & \hline \end{aligned}$ |
|  | 13．O＇Connell Street and Grose Street | North－O＇Connell Street East－Grose Street South－O＇Connell Street West－Grose Street | B | 18 | $\begin{aligned} & 42 \\ & 37 \\ & 35 \\ & 12 \\ & \hline \end{aligned}$ | B | 21 | $\begin{aligned} & \hline 35 \\ & 32 \\ & 81 \\ & 26 \\ & \hline \end{aligned}$ | A | 13 | $\begin{aligned} & \hline 31 \\ & 21 \\ & 44 \\ & 9 \\ & \hline \end{aligned}$ |
|  | 15．O＇Connell Street and Victoria Road | North－O＇Connell Street <br> East－Victoria Road <br> South－O＇Connell Street <br> West－Stadium Carpark Access | D | 44 | $\begin{aligned} & \hline 167 \\ & 98 \\ & 122 \\ & 92 \\ & \hline \end{aligned}$ | C | 34 | $\begin{aligned} & \hline 98 \\ & 81 \\ & 155 \\ & 53 \\ & \hline \end{aligned}$ | B | 26 | $\begin{aligned} & 90 \\ & 86 \\ & 58 \\ & 35 \\ & \hline \end{aligned}$ |
|  | 18．Victoria Road and Marsden Street | North－Villiers Street East－Victoria Road South－Marsden Street West－Victoria Road | C | 36 | $\begin{gathered} \hline 25 \\ 47 \\ 35 \\ 112 \\ \hline \end{gathered}$ | C | 42 | $\begin{aligned} & \hline 19 \\ & 48 \\ & 63 \\ & 96 \\ & \hline \end{aligned}$ | C | 35 | $\begin{aligned} & 14 \\ & 40 \\ & 46 \\ & 71 \\ & \hline \end{aligned}$ |
|  | 16．Church Street and Victoria Road | $\begin{aligned} & \text { North - Church Street } \\ & \text { East Victoria Road } \\ & \text { South - Church Street } \\ & \text { West - Victoria Road } \\ & \hline \end{aligned}$ | D | 44 | $\begin{gathered} 110 \\ 9 \\ 69 \\ 112 \end{gathered}$ | D | 50 | $\begin{gathered} \hline 98 \\ 14 \\ 97 \\ 97 \\ \hline 109 \\ \hline \end{gathered}$ | C | 35 | $\begin{aligned} & 73 \\ & 9 \\ & 50 \\ & 50 \\ & \hline \end{aligned}$ |
|  | 17．Victoria Road and Wilde Avenue | East－Victoria Road South－Wilde Avenue West－Victoria Road | C | 32 | $\begin{array}{r} 96 \\ 60 \\ 131 \\ \hline \end{array}$ | C | 37 | $\begin{aligned} & 117 \\ & 91 \\ & 39 \\ & \hline \end{aligned}$ | B | 24 | $\begin{aligned} & 67 \\ & 40 \\ & 18 \\ & \hline \end{aligned}$ |

GTA Basic Results Summary GTA Basic Results Summary

User and Project Details

| Project: | 14S1091200 PNUR - Rezoning |
| :--- | :--- |
| Title: | Parramatta North Modelling - North |
| File name: | 141008Ing_Parramatta North modelling_North_EX-AM.Isg3x |
| Company: | GTA Consultants Sydney |
| Address: | Lv6, 15 Help Street CHATSWOOD NSW 2067 |

## Scenario 1: 'Ex-AM' (FG1: 'Existing AM', Plan 1: 'Existing - AM') <br> Network Layout Diagram



GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - North | - | - | - | 107.4\% | - | - |
| J1: Windsor Rd/James Rules Dr | - | - | - | 107.4\% | - | - |
| 1/1 | Windsor Rd - N Left | U | 1416 | 71.9\% | 5.3 | 16.6 |
| 1/3+1/2 | Windsor Rd - N Ahead | U | 596 | 90.0: 90.0\% | 64.5 | 24.4 |
| 1/4+1/5 | Windsor Rd - N Ahead Right | U | 874 | 92.7 : 92.7\% | 66.9 | 26.8 |
| 2/2+2/1 | James Rule Dr (off ramp) - E Right Left | U | 347 | 100.5 : 100.5\% | 153.7 | 22.4 |
| 2/3 | James Rule Dr (off ramp) - E Right | U | 193 | 94.0\% | 138.3 | 11.6 |
| 3/1+3/2 | Church St - S Ahead Left | U | 534 | 32.9 : 33.0\% | 2.0 | 0.6 |
| 3/3 | Church St - S Ahead | U | 376 | 87.5\% | 60.3 | 16.2 |
| 3/4+3/5 | Church St - S Ahead Right | U | 118 | 77.2: 77.2\% | 126.0 | 5.8 |
| 4/2+4/1 | Briens Rd (off ramp) - W Left Right | U | 304 | 107.0: 107.0\% | 234.5 | 23.7 |
| 4/3 | Briens Rd (off ramp) - W Right | U | 303 | 107.4\% | 254.1 | 27.4 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| Ped Link: P5 | P6 | - | 0 | 0.0\% | - | - |
| Ped Link: P6 | P7 | - | 0 | 0.0\% | - | - |
| Ped Link: P7 | P8 | - | 0 | 0.0\% | - | - |
| Ped Link: P8 | P5 | - | 0 | 0.0\% | - | - |
| J2: Windsor Rd/The Junction | - | - | - | 81.9\% | - | - |
| 1/1 | Windsor Rd ( N ) Ahead | U | 18 | 1.2\% | 1.3 | 0.0 |
| 1/2 | Windsor Rd (N) Ahead | 0 | 1073 | 72.3\% | 7.8 | 17.1 |
| 1/3+1/4 | Windsor Rd ( N ) Ahead Right | U | 714 | 46.6 : 47.0\% | 2.9 | 1.6 |
| 2/2+2/1 | Windsor Rd - S Ahead Left | U+O | 570 | 42.2 : 42.2\% | 5.7 | 5.4 |
| 2/3 | Windsor Rd - S Ahead | U | 370 | 27.8\% | 5.1 | 2.3 |
| 2/4 | Windsor Rd - S Ahead | U | 118 | 8.9\% | 4.8 | 0.7 |
| 3/2+3/1 | The Junction Access - W Left Right | U | 138 | 81.9 : 81.9\% | 110.9 | 6.4 |


| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| J3: Church St/North Rocks Rd | - | - | - | 101.6\% | - | - |
| 1/1 | Windsor Rd - N Left | U | 444 | 38.2\% | 6.3 | 5.4 |
| 1/2 | Windsor Rd-N Ahead | U | 18 | 2.2\% | 5.0 | 0.2 |
| 1/3 | Windsor Rd - N Ahead | U | 712 | 86.8\% | 35.3 | 24.6 |
| 1/4 | Windsor Rd - N Ahead | U | 723 | 87.8\% | 37.4 | 23.8 |
| 2/2+2/1 | North Rocks Rd (E) Right Left | U | 565 | 101.6 : 101.6\% | 130.7 | 29.1 |
| 2/3 | North Rocks Rd (E) Right | U | 255 | 97.6\% | 147.2 | 16.0 |
| 3/1 | Church St - S Ahead | U | 15 | 1.2\% | 9.8 | 0.2 |
| 3/2 | Church St - S Ahead | U | 356 | 27.3\% | 5.1 | 3.5 |
| 3/3 | Church St - S Ahead | U | 360 | 27.6\% | 5.2 | 3.8 |
| 3/5+3/4 | Church St - S Ahead Right | U | 250 | 74.4 : 0.0\% | 51.1 | 8.0 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J4: Church St/Board St/Seville St | - | - | - | 77.5\% | - | - |
| 1/1 | Church St - N Left Ahead | U | 41 | 2.3\% | 1.0 | 0.0 |
| 1/2 | Church St - N Ahead | U | 759 | 41.6\% | 1.7 | 0.4 |
| 1/3 | Church St - N Ahead | U | 996 | 54.4\% | 2.7 | 4.5 |
| 2/1 | Seville St - E Left | O | 3 | 0.9\% | 9.6 | 0.0 |
| 3/1 | Church St - S Ahead Left | U | 29 | 1.6\% | 1.0 | 0.0 |
| 3/2 | Church St - S Ahead | U | 345 | 19.2\% | 1.2 | 0.1 |
| 3/3 | Church St - S Ahead | U | 345 | 19.2\% | 1.2 | 0.1 |
| 4/1 | Board St - W Left | 0 | 274 | 77.5\% | 26.4 | 5.9 |
| J5: Church St/Barney St | - | - | - | 92.8\% | - | - |
| 1/2+1/1 | Church St - N Left Ahead | U | 97 | 7.5 : 7.4\% | 11.2 | 1.3 |
| 1/3 | Church St - N Ahead | U | 893 | 65.5\% | 16.9 | 16.1 |
| 1/4 | Church St - N Right | O | 788 | 92.8\% | 40.1 | 18.6 |
| 2/1+2/2 | Barney St - E Right Left Ahead | U | 114 | 82.9 : 82.9\% | 123.5 | 5.5 |

## GTA Basic Results Summary

| 2/3 | Barney St - E Right |  | U |  | 113 | 84.1 |  | 131.2 |  | 6.3 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $3 / 2+3 / 1$ | Church St - S Ahead Left |  | U |  | 22 | 4.4 : | 4\% | 40.2 |  | 0.4 |
| $3 / 3+3 / 4$ | Church St - S Ahead |  | U |  | 441 | 55.3 : 5 | .7\% | 46.4 |  | 9.3 |
| 4/2+4/1 | Barney St - W Left Ahead |  | U |  | 119 | 45.9 : 4 | .9\% | 40.0 |  | 1.8 |
| Ped Link: P1 | P1 |  | - |  | 0 | 0.0 |  | - |  | - |
| Ped Link: P2 | P2 |  | - |  | 0 | 0.0 |  | - |  | - |
| Ped Link: P3 | P3 |  | - |  | 0 | 0.0 |  | - |  | - |
| C1 - TCS704 - Windsor Rd/Briens Rd C2 - TCS 3704 - Windsor Rd/The Junction Access C3 - TCS464 - North Rocks Rd/Church St C4 - TCS1085-Church St/Barney St | PRC for Signalled Lanes (\%): <br> PRC for Signalled Lanes (\%): <br> PRC for Signalled Lanes (\%): <br> PRC for Signalled Lanes (\%): <br> PRC Over All Lanes (\%): | $\begin{array}{r} -19.3 \\ 9.9 \\ -12.9 \\ -3.1 \\ -19.3 \end{array}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes ( pcuHr ): Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes ( pcuHr ): Total Delay Over All Lanes(pcuHr): |  |  |  | $\begin{array}{r} 103.15 \\ 8.69 \\ 50.54 \\ 28.36 \\ 194.11 \end{array}$ | Cycle Time (s): Cycle Time (s): Cycle Time (s): Cycle Time (s): |  |  |

GTA Basic Results Summary GTA Basic Results Summary

User and Project Details

| Project: | 14S1091200 PNUR - Rezoning |
| :--- | :--- |
| Title: | Parramatta North Modelling - North |
| File name: | 141008Ing_Parramatta North modelling_North_EX_PM_SAT.Isg3x |
| Company: | GTA Consultants Sydney |
| Address: | Lv6, 15 Help Street CHATSWOOD NSW 2067 |

## Scenario 1: 'Ex-PM' (FG1: 'Existing PM', Plan 2: 'Existing - PM') Network Layout Diagram



GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - North | - | - | - | 124.5\% | - | - |
| J1: Windsor Rd/James Rules Dr | - | - | - | 124.5\% | - | - |
| 1/1 | Windsor Rd - N Left | 0 | 817 | 50.2\% | 3.3 | 5.7 |
| 1/3+1/2 | Windsor Rd - N Ahead | U | 719 | 68.9 : 68.9\% | 37.9 | 13.8 |
| 1/4+1/5 | Windsor Rd - N Ahead Right | U | 184 | 0.0 : 124.5\% | 488.3 | 28.6 |
| 2/2+2/1 | James Rule Dr (off ramp) - E Right Left | U | 390 | 119.7: 119.7\% | 404.2 | 53.9 |
| 2/3 | James Rule Dr (off ramp) - E Right | U | 380 | 117.9\% | 381.8 | 48.1 |
| 3/1+3/2 | Church St - S Ahead Left | O+U | 768 | 46.7 : 46.7\% | 2.8 | 8.5 |
| 3/3 | Church St - S Ahead | U | 730 | 95.9\% | 72.1 | 34.1 |
| $3 / 4+3 / 5$ | Church St - S Ahead Right | U | 704 | 88.5 : 88.5\% | 41.7 | 9.5 |
| 4/1+4/2 | Briens Rd (off ramp) - W Left Right | U | 501 | 80.1: 109.1\% | 149.5 | 23.5 |
| 4/3 | Briens Rd (off ramp) - W Right | U | 274 | 107.4\% | 257.8 | 25.1 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| Ped Link: P5 | P6 | - | 0 | 0.0\% | - | - |
| Ped Link: P6 | P7 | - | 0 | 0.0\% | - | - |
| Ped Link: P7 | P8 | - | 0 | 0.0\% | - | - |
| Ped Link: P8 | P4 | - | 0 | 0.0\% | - | - |
| J2: Windsor Rd/The Junction | - | - | - | 73.1\% | - | - |
| 1/1 | Windsor Rd ( N ) Ahead | U | 707 | 49.0\% | 4.7 | 12.9 |
| 1/2 | Windsor Rd (N) Ahead | U | 326 | 22.2\% | 1.9 | 0.3 |
| 1/3+1/4 | Windsor Rd ( N ) Ahead Right | U | 235 | 30.8 : 32.5\% | 17.7 | 2.5 |
| 2/2+2/1 | Windsor Rd - S Ahead Left | U+O | 855 | 73.1 : 73.1\% | 21.3 | 22.1 |
| 2/3 | Windsor Rd - S Ahead | U | 635 | 55.6\% | 9.1 | 11.8 |
| 2/4 | Windsor Rd - S Ahead | U | 704 | 61.7\% | 28.8 | 20.9 |
| 3/2+3/1 | The Junction Access - W Left Right | U | 301 | 71.8:71.8\% | 62.8 | 7.1 |

GTA Basic Results Summary

| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| J3: Church St/North Rocks Rd | - | - | - | 94.3\% | - | - |
| 1/1 | Windsor Rd - N Left | U | 462 | 40.5\% | 5.5 | 3.9 |
| 1/2 | Windsor Rd-N Ahead | U | 295 | 37.9\% | 12.1 | 1.7 |
| 1/3 | Windsor Rd - N Ahead | U | 351 | 43.4\% | 32.4 | 8.0 |
| 1/4 | Windsor Rd - N Ahead | U | 261 | 31.7\% | 37.1 | 9.2 |
| $2 / 2+2 / 1$ | North Rocks Rd (E) Right Left | U | 398 | 86.7 : 86.7\% | 65.0 | 12.2 |
| 2/3 | North Rocks Rd (E) Right | U | 304 | 94.3\% | 116.6 | 16.4 |
| 3/1 | Church St - S Ahead | U | 17 | 1.3\% | 8.8 | 0.2 |
| 3/2 | Church St - S Ahead | U | 784 | 59.6\% | 9.4 | 9.9 |
| 3/3 | Church St - S Ahead | U | 613 | 46.6\% | 7.0 | 5.5 |
| 3/5+3/4 | Church St - S Ahead Right | U | 688 | 70.7 : 70.7\% | 21.7 | 8.5 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J4: Church St/Board St/Seville St | - | - | - | 91.0\% | - | - |
| 1/1 | Church St - N Left Ahead | U | 339 | 18.8\% | 1.2 | 0.1 |
| 1/2 | Church St - N Ahead | U | 329 | 17.7\% | 1.2 | 0.1 |
| 1/3 | Church St - N Ahead | U | 433 | 23.2\% | 1.3 | 0.3 |
| 2/1 | Seville St - E Left | O | 9 | 1.7\% | 3.5 | 0.0 |
| 3/1 | Church St - S Ahead Left | U | 24 | 1.3\% | 1.0 | 0.0 |
| 3/2 | Church St - S Ahead | U | 848 | 47.1\% | 1.9 | 0.4 |
| 3/3 | Church St - S Ahead | U | 851 | 47.3\% | 1.9 | 0.4 |
| 4/1 | Board St - W Left | 0 | 384 | 91.0\% | 46.7 | 16.6 |
| J5: Church St/Barney St | - | - | - | 91.0\% | - | - |
| 1/2+1/1 | Church St - N Left Ahead | U | 116 | 10.6:10.3\% | 9.0 | 0.8 |
| 1/3 | Church St - N Ahead | U | 582 | 49.7\% | 10.1 | 5.3 |
| 1/4 | Church St - N Right | 0 | 400 | 85.7\% | 53.4 | 16.3 |
| 2/1+2/2 | Barney St - E Right Left Ahead | U | 239 | 91.0: 91.0\% | 112.2 | 12.4 |

## GTA Basic Results Summary

| 2/3 | Barney St - E Right |  | U |  | 254 | 85.9 |  | 92.9 |  | 11.9 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $3 / 2+3 / 1$ | Church St - S Ahead Left |  | U |  | 20 | 2.6 : | 6\% | 25.2 |  | 0.4 |
| $3 / 3+3 / 4$ | Church St - S Ahead |  | U |  | 879 | 71.3 : 7 | 1.2\% | 36.8 |  | 17.9 |
| 4/2+4/1 | Barney St - W Left Ahead |  | U |  | 399 | 75.5 : 7 | 5.5\% | 58.9 |  | 13.9 |
| Ped Link: P1 | P1 |  | - |  | 0 | 0.0 |  | - |  | - |
| Ped Link: P2 | P2 |  | - |  | 0 | 0.0 |  | - |  | - |
| Ped Link: P3 | P3 |  | - |  | 0 | 0.0 |  | - |  | - |
| C1 - TCS704 - Windsor Rd/Briens Rd C2 - TCS 3704 - Windsor Rd/The Junction Access C3 - TCS464 - North Rocks Rd/Church St C4 - TCS1085-Church St/Barney St | PRC for Signalled Lanes (\%): <br> PRC for Signalled Lanes (\%): <br> PRC for Signalled Lanes (\%): <br> PRC for Signalled Lanes (\%): <br> PRC Over All Lanes (\%): | $\begin{array}{r} -38.4 \\ 23.1 \\ -4.8 \\ -1.2 \\ -38.4 \\ \hline \end{array}$ |  | Total <br> Total <br> Total <br> Total | Delay for Signalled Lanes Delay for Signalled Lanes Delay for Signalled Lanes Delay for Signalled Lanes Total Delay Over All Lanes | (pcuHr): <br> (pcuHr): <br> (pcuHr): <br> (pcuHr): <br> (pcuHr): | $\begin{array}{r} 181.18 \\ 19.70 \\ 31.72 \\ 37.32 \\ 276.19 \\ \hline \end{array}$ | Cycle Time (s): <br> Cycle Time (s): <br> Cycle Time (s): <br> Cycle Time (s): | $\begin{aligned} & 134 \\ & 134 \\ & 134 \\ & 134 \end{aligned}$ |  |

GTA Basic Results Summary
Scenario 2: 'Ex-SAT' (FG2: 'Existing SAT', Plan 3: 'Existing - SAT') Network Layout Diagram


GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - North | - | - | - | 110.1\% | - | - |
| J1: Windsor Rd/James Rules Dr | - | - | - | 110.1\% | - | - |
| 1/1 | Windsor Rd - N Left | 0 | 954 | 59.1\% | 4.1 | 8.1 |
| 1/3+1/2 | Windsor Rd - N Ahead | U | 525 | 71.5 : 71.5\% | 52.9 | 11.4 |
| 1/4+1/5 | Windsor Rd-N Ahead Right | U | 482 | 55.4 : 110.1\% | 151.3 | 22.3 |
| $2 / 2+2 / 1$ | James Rule Dr (off ramp) - E Right Left | U | 430 | 100.6 : 100.6\% | 140.8 | 26.3 |
| 2/3 | James Rule Dr (off ramp) - E Right | U | 408 | 98.2\% | 124.0 | 23.0 |
| 3/1+3/2 | Church St - S Ahead Left | O+U | 706 | 43.9 : 43.9\% | 3.2 | 2.3 |
| 3/3 | Church St - S Ahead | U | 437 | 91.6\% | 74.3 | 19.7 |
| $3 / 4+3 / 5$ | Church St - S Ahead Right | U | 466 | 80.6 : 80.6\% | 57.2 | 7.1 |
| 4/1+4/2 | Briens Rd (off ramp) - W Left Right | U | 574 | 58.5 : 109.8\% | 142.4 | 27.2 |
| 4/3 | Briens Rd (off ramp) - W Right | U | 356 | 107.1\% | 239.3 | 30.8 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| Ped Link: P5 | P6 | - | 0 | 0.0\% | - | - |
| Ped Link: P6 | P7 | - | 0 | 0.0\% | - | - |
| Ped Link: P7 | P8 | - | 0 | 0.0\% | - | - |
| Ped Link: P8 | P4 | - | 0 | 0.0\% | - | - |
| J2: Windsor Rd/The Junction | - - | - | - | 69.8\% | - | - |
| 1/1 | Windsor Rd (N) Ahead | U | 656 | 44.4\% | 5.0 | 12.6 |
| 1/2 | Windsor Rd (N) Ahead | U | 544 | 36.3\% | 2.3 | 0.6 |
| 1/3+1/4 | Windsor Rd (N) Ahead Right | U | 352 | 69.1 : 69.8\% | 35.1 | 6.2 |
| 2/2+2/1 | Windsor Rd-S Ahead Left | U+O | 755 | 64.8 : 64.8\% | 21.3 | 20.8 |
| 2/3 | Windsor Rd-S Ahead | U | 356 | 31.4\% | 7.3 | 6.1 |
| 2/4 | Windsor Rd-S Ahead | U | 466 | 41.0\% | 24.3 | 10.7 |
| $3 / 2+3 / 1$ | The Junction Access - W Left Right | U | 279 | 66.5 : 66.5\% | 58.4 | 5.4 |

GTA Basic Results Summary

| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| J3: Church St/North Rocks Rd | - | - | - | 97.1\% | - | - |
| 1/1 | Windsor Rd - N Left | U | 471 | 40.0\% | 8.8 | 6.5 |
| 1/2 | Windsor Rd-N Ahead | U | 218 | 27.1\% | 3.7 | 0.3 |
| 1/3 | Windsor Rd - N Ahead | U | 553 | 66.2\% | 42.4 | 15.5 |
| 1/4 | Windsor Rd - N Ahead | U | 291 | 35.2\% | 36.9 | 9.4 |
| $2 / 2+2 / 1$ | North Rocks Rd (E) Right Left | U | 452 | 97.1:97.1\% | 101.3 | 19.1 |
| 2/3 | North Rocks Rd (E) Right | U | 252 | 75.8\% | 71.9 | 10.1 |
| 3/1 | Church St - S Ahead | U | 7 | 0.5\% | 6.6 | 0.1 |
| 3/2 | Church St - S Ahead | U | 559 | 42.9\% | 5.4 | 5.6 |
| 3/3 | Church St - S Ahead | U | 356 | 27.4\% | 5.8 | 4.4 |
| 3/5+3/4 | Church St - S Ahead Right | U | 421 | 62.3 : 62.3\% | 22.7 | 6.4 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J4: Church St/Board St/Seville St | - | - | - | 55.4\% | - | - |
| 1/1 | Church St - N Left Ahead | U | 386 | 21.2\% | 1.3 | 0.1 |
| 1/2 | Church St - N Ahead | U | 397 | 21.4\% | 1.3 | 0.1 |
| 1/3 | Church St - N Ahead | U | 493 | 26.7\% | 1.5 | 0.6 |
| 2/1 | Seville St - E Left | O | 24 | 4.8\% | 3.8 | 0.0 |
| 3/1 | Church St - S Ahead Left | U | 25 | 1.4\% | 1.0 | 0.0 |
| 3/2 | Church St - S Ahead | U | 509 | 28.3\% | 1.4 | 0.2 |
| 3/3 | Church St - S Ahead | U | 508 | 28.2\% | 1.4 | 0.2 |
| 4/1 | Board St - W Left | O | 318 | 55.4\% | 7.8 | 3.1 |
| J5: Church St/Barney St | - | - | - | 77.8\% | - | - |
| 1/2+1/1 | Church St - N Left Ahead | U | 109 | 9.6 : 9.3\% | 12.1 | 1.2 |
| 1/3 | Church St - N Ahead | U | 661 | 54.2\% | 14.2 | 8.2 |
| 1/4 | Church St - N Right | 0 | 499 | 77.8\% | 35.4 | 9.6 |
| 2/1+2/2 | Barney St - E Right Left Ahead | U | 184 | 73.0 : 73.0\% | 76.7 | 7.2 |

## GTA Basic Results Summary



GTA Basic Results Summary GTA Basic Results Summary

User and Project Details

| Project: | 14S1091200 PNUR - Rezoning |
| :--- | :--- |
| Title: | Parramatta North Modelling - PHR |
| Location: | Pennant Hills Rd |
| File name: | 140909Ing_Parramatta North modelling_PHR_EX.Isg3x |

Scenario 1: 'EX-AM' (FG1: 'Existing AM', Plan 1: 'Network Control Plan 1')

## Network Layout Diagram



GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - PHR | - | - | - | 95.7\% | - | - |
| J1: Church St/Factory St | - | - | - | 71.5\% | - | - |
| 1/2+1/1 | Church St - N Left Ahead | U | 46 | 3.7 : 3.7\% | 8.1 | 0.4 |
| 1/3+1/4 | Church St - N Ahead Right | U | 895 | 71.5 : 71.5\% | 17.7 | 19.8 |
| $2 / 2+2 / 1$ | Factory St -E Left Ahead | U | 37 | 15.7 : 15.7\% | 55.0 | 0.8 |
| 2/3 | Factory St -E Right Ahead | 0 | 19 | 10.6\% | 60.2 | 0.6 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 36 | 2.9 : 2.9\% | 10.7 | 0.5 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U+O | 439 | 35.0 : 35.0\% | 6.9 | 2.9 |
| $4 / 2+4 / 1$ | Factory St - W Left Ahead | U | 21 | 6.3 : 6.3\% | 48.3 | 0.5 |
| 4/3 | Factory St - W Ahead Right | 0 | 1 | 0.6\% | 60.5 | 0.0 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J2: Church St/Pennant Hills Rd | - | - | - | 95.7\% | - | - |
| 1/2+1/1 | Church St - N U-Turn Ahead | U+O | 40 | 4.4 : 0.0\% | 16.9 | 0.6 |
| 1/4+1/3 | Church St - N U-Turn Ahead | U+O | 892 | 95.7 : 95.7\% | 48.3 | 21.5 |
| 2/1 | Pennant Hills Rd-NE Left | U | 443 | 76.3\% | 48.2 | 11.7 |
| 2/2 | Pennant Hills Rd - NE Right | U | 374 | 69.6\% | 49.4 | 12.5 |
| 3/1 | Albert St - E Left | 0 | 37 | 17.2\% | 10.2 | 0.1 |
| 4/2+4/1 | Church St - S Ahead Left | U | 33 | 3.0 : 3.0\% | 11.6 | 0.4 |
| 4/3+4/4 | Church St - S Ahead Right | U | 585 | 47.9 : 74.3\% | 21.9 | 14.1 |
| 5/2+5/1 | Albert St - W Left Left2 | U | 122 | 22.6 : 22.6\% | 37.0 | 3.2 |
| 5/3 | Albert St - W Left | U | 118 | 22.0\% | 37.0 | 3.2 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |

GTA Basic Results Summary

| J3: Church St/Grose St | - | - | - | 94.1\% | - |  | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1/2+1/1 | Church St - N Left Ahead | U | 93 | 8.2 : 8.2\% | 9.1 |  | 0.6 |
| 1/3+1/4 | Church St - N Ahead Right | $\mathrm{U}+\mathrm{O}$ | 1251 | 94.1: 94.1\% | 29.1 |  | 21.9 |
| $2 / 2+2 / 1$ | Grose St - E Left Ahead | U | 188 | 58.4 : 58.4\% | 59.3 |  | 6.5 |
| 2/3 | Grose St - E Right Ahead | 0 | 71 | 80.0\% | 145.2 |  | 4.0 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 72 | 6.3 : 6.3\% | 10.6 |  | 0.6 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | $\mathrm{U}+\mathrm{O}$ | 512 | 44.1: 44.1\% | 15.2 |  | 8.3 |
| $4 / 2+4 / 1$ | Grose St - W Left Ahead | U | 209 | 64.8: 64.8\% | 62.0 |  | 7.5 |
| 4/3 | Grose St - W Ahead Right | 0 | 58 | 41.0\% | 70.4 |  | 2.2 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P2 | 2 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P3 | 3 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P4 | 4 | - | 0 | 0.0\% | - |  | - |
| C1 - Church St/Factory St C2 - Church St/Pennant Hills Rd C3 - Church St/Grose St | PRC for Signalled Lanes (\%): <br> PRC for Signalled Lanes (\%): <br> PRC for Signalled Lanes (\%): <br> PRC Over All Lanes (\%): | $\begin{array}{r} 25.9 \\ -6.3 \\ -4.5 \\ -6.3 \end{array}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  | $\begin{array}{r} 6.64 \\ 29.35 \\ 23.43 \\ 59.52 \end{array}$ | $\begin{array}{ll} \text { Cycle Time (s): } & 124 \\ \text { Cycle Time (s): } & 124 \\ \text { Cycle Time (s): } & 124 \end{array}$ |  |

GTA Basic Results Summary
Scenario 2: 'EX-PM' (FG2: 'Existing PM', Plan 1: 'Network Control Plan 1')

## Network Layout Diagram



GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - PHR | - | - | - | 84.9\% | - | - |
| J1: Church St/Factory St | - | - | - | 66.7\% | - | - |
| 1/2+1/1 | Church St - N Left Ahead | U | 43 | 3.5 : 3.5\% | 7.8 | 0.3 |
| 1/3+1/4 | Church St - N Ahead Right | U | 622 | 50.3: 50.3\% | 13.0 | 9.2 |
| $2 / 2+2 / 1$ | Factory St -E Left Ahead | U | 32 | 13.6:13.6\% | 48.0 | 0.5 |
| 2/3 | Factory St -E Right Ahead | 0 | 22 | 14.4\% | 56.7 | 0.7 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 36 | 3.0 : 3.0\% | 4.4 | 0.2 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U+O | 822 | 66.7 : 66.7\% | 9.1 | 15.1 |
| 4/2+4/1 | Factory St - W Left Ahead | U | 39 | 13.9: 13.9\% | 45.3 | 0.8 |
| 4/3 | Factory St - W Ahead Right | 0 | 2 | 1.3\% | 55.1 | 0.1 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J2: Church St/Pennant Hills Rd | - | - | - | 83.2\% | - | - |
| 1/2+1/1 | Church St - N U-Turn Ahead | U+O | 32 | 4.1: 0.0\% | 15.6 | 0.6 |
| 1/4+1/3 | Church St - N U-Turn Ahead | U+O | 619 | 72.2 : 72.2\% | 35.5 | 16.8 |
| 2/1 | Pennant Hills Rd-NE Left | U | 267 | 41.4\% | 23.9 | 5.4 |
| 2/2 | Pennant Hills Rd - NE Right | U | 291 | 65.9\% | 47.8 | 8.6 |
| 3/1 | Albert St - E Left | 0 | 25 | 7.8\% | 6.1 | 0.0 |
| 4/2+4/1 | Church St - S Ahead Left | U | 45 | 3.9 : 3.9\% | 8.2 | 0.4 |
| 4/3+4/4 | Church St - S Ahead Right | U | 1096 | 83.2 : 83.2\% | 20.0 | 35.5 |
| 5/2+5/1 | Albert St - W Left Left2 | U | 159 | 35.6 : 35.6\% | 39.3 | 4.0 |
| 5/3 | Albert St - W Left | U | 153 | 34.7\% | 39.2 | 4.0 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |

GTA Basic Results Summary

| J3: Church St/Grose St | - | - | - | 84.9\% | - |  | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1/2+1/1 | Church St - N Left Ahead | U | 86 | 8.7 : 8.7\% | 10.0 |  | 0.6 |
| 1/3+1/4 | Church St - N Ahead Right | U+O | 765 | 67.3: 67.3\% | 15.1 |  | 6.9 |
| $2 / 2+2 / 1$ | Grose St - E Left Ahead | U | 157 | 39.4 : 39.4\% | 41.4 |  | 4.0 |
| 2/3 | Grose St - E Right Ahead | 0 | 106 | 73.0\% | 91.5 |  | 4.2 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 71 | 6.9 : 6.9\% | 12.6 |  | 0.7 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U+O | 944 | 84.9: 84.9\% | 29.4 |  | 24.1 |
| $4 / 2+4 / 1$ | Grose St - W Left Ahead | U | 239 | 58.4:58.4\% | 44.6 |  | 6.4 |
| 4/3 | Grose St - W Ahead Right | 0 | 77 | 30.0\% | 47.0 |  | 2.2 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P2 | 2 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P3 | 3 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P4 | 4 | - | 0 | 0.0\% | - |  | - |
| C1 - Church St/Factory St C2 - Church St/Pennant Hills Rd C3 - Church St/Grose St | PRC for Signalled Lanes (\%): PRC for Signalled Lanes (\%): PRC for Signalled Lanes (\%): PRC Over All Lanes (\%): | $\begin{array}{r} 34.8 \\ 8.2 \\ 5.9 \\ 5.9 \end{array}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  | $\begin{array}{r} 5.74 \\ 21.48 \\ 19.88 \\ 47.15 \end{array}$ | Cycle Time (s): 106 <br> Cycle Time (s): 106 <br> Cycle Time (s): 106 |  |

GTA Basic Results Summary
Scenario 3: 'EX-SAT' (FG3: 'Existing SAT', Plan 1: 'Network Control Plan 1')
Network Layout Diagram


GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - PHR | - | - | - | 84.6\% | - | - |
| J1: Church St/Factory St | - | - | - | 57.9\% | - | - |
| 1/2+1/1 | Church St - N Left Ahead | U | 36 | 3.0:3.0\% | 8.0 | 0.2 |
| 1/3+1/4 | Church St - N Ahead Right | U | 725 | 57.9 : 57.9\% | 13.5 | 11.9 |
| $2 / 2+2 / 1$ | Factory St -E Left Ahead | U | 48 | 21.0:21.0\% | 51.1 | 0.8 |
| 2/3 | Factory St -E Right Ahead | 0 | 23 | 16.8\% | 60.4 | 0.7 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 15 | 1.2 : 1.2\% | 10.0 | 0.2 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U+O | 644 | 51.6 : 51.6\% | 9.0 | 5.7 |
| 4/2+4/1 | Factory St - W Left Ahead | U | 42 | 13.3:13.3\% | 45.7 | 1.0 |
| 4/3 | Factory St - W Ahead Right | 0 | 5 | 3.6\% | 58.3 | 0.2 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J2: Church St/Pennant Hills Rd | - | - | - | 84.6\% | - | - |
| 1/2+1/1 | Church St - N U-Turn Ahead | U+O | 18 | 2.2 : 0.0\% | 19.0 | 0.3 |
| 1/4+1/3 | Church St - N U-Turn Ahead | U+O | 737 | 84.6 : 84.6\% | 28.1 | 15.0 |
| 2/1 | Pennant Hills Rd-NE Left | U | 295 | 46.2\% | 26.0 | 6.3 |
| 2/2 | Pennant Hills Rd - NE Right | U | 253 | 59.5\% | 47.7 | 7.5 |
| 3/1 | Albert St - E Left | 0 | 29 | 9.9\% | 6.8 | 0.1 |
| 4/2+4/1 | Church St - S Ahead Left | U | 23 | 1.9 : 1.9\% | 7.0 | 0.1 |
| 4/3+4/4 | Church St - S Ahead Right | U | 817 | 62.3 : 62.3\% | 16.6 | 6.5 |
| 5/2+5/1 | Albert St - W Left Left2 | U | 95 | 21.9 : 21.9\% | 39.3 | 2.2 |
| 5/3 | Albert St - W Left | U | 84 | 19.7\% | 38.9 | 2.2 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |

GTA Basic Results Summary

| J3: Church St/Grose St | - | - | - | 75.7\% | - |  | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1/2+1/1 | Church St - N Left Ahead | U | 67 | 6.4 : 6.4\% | 10.7 |  | 0.4 |
| 1/3+1/4 | Church St - N Ahead Right | $\mathrm{U}+\mathrm{O}$ | 908 | 75.7 : 75.7\% | 15.5 |  | 10.3 |
| $2 / 2+2 / 1$ | Grose St - E Left Ahead | U | 107 | 32.0 : 32.0\% | 45.4 |  | 2.8 |
| 2/3 | Grose St - E Right Ahead | 0 | 104 | 64.5\% | 78.9 |  | 3.9 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 48 | 4.5 : 4.5\% | 11.1 |  | 0.3 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | $\mathrm{U}+\mathrm{O}$ | 682 | 61.4: 61.4\% | 17.2 |  | 12.3 |
| $4 / 2+4 / 1$ | Grose St - W Left Ahead | U | 156 | 44.2 : 44.2\% | 45.9 |  | 3.9 |
| 4/3 | Grose St - W Ahead Right | 0 | 60 | 26.5\% | 51.5 |  | 1.8 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P2 | 2 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P3 | 3 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P4 | 4 | - | 0 | 0.0\% | - |  | - |
| C1 - Church St/Factory St C2 - Church St/Pennant Hills Rd C3 - Church St/Grose St | $\begin{aligned} & \text { PRC for Signalled Lanes (\%): } \\ & \text { PRC for Signalled Lanes (\%): } \\ & \text { PRC for Signalled Lanes (\%): } \\ & \text { PRC Over All Lanes (\%): } \end{aligned}$ | $\begin{array}{r} 55.3 \\ 6.4 \\ 18.8 \\ 6.4 \end{array}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  | $\begin{array}{r} 6.13 \\ 17.08 \\ 13.99 \\ 37.25 \end{array}$ | Cycle Time (s): 110 <br> Cycle Time (s): 110 <br> Cycle Time (s): 110 |  |

GTA Basic Results Summary GTA Basic Results Summary

## User and Project Details

| Project: | 14S1091200 PNUR - Rezoning |
| :--- | :--- |
| Title: | Parramatta North Modelling - Victoria Road |
| Location: | North Parramatta - Victoria Road |
| File name: | 140910Ing_Parramatta North modelling_South_EX-AM_PM_SAT.Isg3x |

Scenario 1: 'Ex-AM' (FG1: 'Existing AM', Plan 1: 'Existing AM') Network Layout Diagram


GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - Victoria Road | - | - | - | 93.3\% | - | - |
| J1: O'Connell St/Albert St | - | - | - | 74.9\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 346 | 31.3:31.3\% | 13.3 | 5.6 |
| 1/3 | O'Connell St - N Ahead | U | 466 | 42.2\% | 14.8 | 8.4 |
| 2/1 | Albert St - E Left | U | 272 | 74.9\% | 58.8 | 8.6 |
| 2/2 | Albert St - E Right Ahead | 0 | 21 | 7.7\% | 49.9 | 0.6 |
| 3/2+3/1 | O'Connell St - S Ahead Left | U | 137 | 30.7:30.7\% | 6.4 | 2.8 |
| 3/3+3/4 | O'Connell St - S Ahead Right | U+O | 447 | 41.7:41.7\% | 6.2 | 1.4 |
| 4/2+4/1 | Albert St - W Left Ahead Right | O+U | 12 | 5.9:5.9\% | 53.1 | 0.3 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| J2: O'Connell St/Grose St | - | - | - | 64.9\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 456 | 48.5 : 48.5\% | 16.2 | 7.4 |
| 1/3+1/4 | O'Connell St - N Ahead Right | U+O | 621 | 64.9:64.9\% | 18.6 | 10.4 |
| 2/2+2/1 | Grose St - E Left Ahead | U | 141 | 0.0 : 22.2\% | 30.7 | 3.4 |
| 2/3 | Grose St - E Right Ahead | 0 | 55 | 18.2\% | 47.6 | 1.6 |
| 3/2+3/1 | O'Connell St - S Ahead Left | U | 159 | 16.9 : 16.9\% | 14.5 | 2.2 |
| $3 / 3+3 / 4$ | O'Connell St - S Ahead Right | U+O | 645 | 57.4:59.0\% | 22.4 | 6.8 |
| 4/2+4/1 | Grose St - W Left Ahead | U | 9 | 2.4 : $2.4 \%$ | 39.1 | 0.2 |
| 4/3 | Grose St - W Ahead Right | 0 | 18 | 9.2\% | 49.6 | 0.5 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J3: O'Connell St/Victoria Rd | - | - | - | 93.0\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 565 | 72.9: 72.9\% | 35.9 | 18.5 |
| 1/3+1/4 | O'Connell St - N Ahead Right | U+O | 622 | 76.3:76.3\% | 23.2 | 20.1 |


| 2/1 | Victoria Rd-E Left | U | 437 | 51.6\% | 13.8 | 13.2 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2/2 | Victoria Rd - E Right Ahead | U | 109 | 51.5\% | 56.6 | 2.4 |
| $3 / 2+3 / 1$ | O'Connell St - S Ahead Left | U | 778 | 55.2 : 55.2\% | 7.8 | 10.3 |
| $3 / 4+3 / 3$ | O'Connell St - S Ahead Right | U | 577 | 93.0 : 0.0\% | 73.7 | 23.8 |
| 4/2+4/1 | Victoria Rd-W Left Ahead | U | 14 | 6.3 : 6.3\% | 55.3 | 0.3 |
| 4/3 | Victoria Rd - W Ahead | U | 6 | 2.8\% | 55.5 | 0.2 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J4: Marist Pl/Victoria Rd | - | - | - | 57.9\% | - | - |
| 1/2+1/1 | Villiers St - N Left Ahead | U | 136 | 36.6 : 36.6\% | 48.4 | 3.4 |
| 1/3 | Villiers St - N Ahead Right | U | 102 | 29.3\% | 48.4 | 3.1 |
| 2/2+2/1 | Victoria Rd - E Left Ahead | U | 468 | 53.5 : 53.5\% | 14.3 | 3.8 |
| 2/3 | Victoria Rd - E Right Ahead | O | 180 | 38.7\% | 22.2 | 4.5 |
| $3 / 2+3 / 1$ | Marist PI - S Ahead Left | U | 175 | 54.0:54.0\% | 56.3 | 5.6 |
| 3/3 | Marist PI - S Ahead Right | U | 166 | 52.3\% | 56.2 | 5.5 |
| 4/2+4/1 | Victoria Rd - W Left Ahead | U | 399 | 46.0 : 46.0\% | 42.6 | 13.1 |
| 4/3 | Victoria Rd - W Ahead Right | O | 252 | 57.9\% | 42.0 | 8.1 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J5: Church St/Victoria Rd | - | - | - | 85.7\% | - | - |
| 1/2+1/1 | Church St - N Left | U | 632 | 40.4 : 40.4\% | 15.6 | 6.1 |
| 1/3+1/4 | Church St - N Ahead Right | U | 428 | 85.7 : 85.7\% | 69.2 | 13.6 |
| $2 / 2+2 / 1$ | Victoria Rd - E Left Ahead | U | 430 | 47.1: 47.1\% | 4.7 | 4.5 |
| 2/3+2/4 | Victoria Rd - E Right Ahead | U | 534 | 57.8:57.8\% | 25.6 | 16.6 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 51 | 11.9: 11.9\% | 40.7 | 1.3 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U | 238 | 73.7 : 73.7\% | 66.2 | 6.0 |



GTA Basic Results Summary
Scenario 2: 'Ex-PM' (FG2: 'Existing PM', Plan 2: 'Existing PM') Network Layout Diagram


GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - Victoria Road | - | - | - | 87.8\% | - | - |
| J1: O'Connell St/Albert St | - | - | - | 56.3\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 221 | 20.2 : $20.2 \%$ | 12.6 | 3.3 |
| 1/3 | O'Connell St - N Ahead | U | 241 | 22.1\% | 12.8 | 3.8 |
| 2/1 | Albert St - E Left | U | 213 | 56.3\% | 46.5 | 6.1 |
| 2/2 | Albert St - E Right Ahead | 0 | 46 | 16.7\% | 50.6 | 1.4 |
| 3/2+3/1 | O'Connell St - S Ahead Left | U | 214 | 47.7: $47.7 \%$ | 9.5 | 4.6 |
| 3/3+3/4 | O'Connell St - S Ahead Right | U+O | 760 | 46.1:46.1\% | 5.1 | 3.1 |
| 4/2+4/1 | Albert St - W Left Ahead Right | $\mathrm{O}+\mathrm{U}$ | 12 | 3.9 : 3.9\% | 48.6 | 0.3 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| J2: O'Connell St/Grose St | - | - | - | 76.7\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 318 | 33.3:33.3\% | 14.9 | 5.6 |
| 1/3+1/4 | O'Connell St - N Ahead Right | U+O | 339 | 34.7 : 34.7\% | 14.8 | 5.2 |
| 2/2+2/1 | Grose St - E Left Ahead | U | 157 | 25.2 : $25.2 \%$ | 32.6 | 3.8 |
| 2/3 | Grose St - E Right Ahead | 0 | 72 | 23.1\% | 45.7 | 2.1 |
| 3/2+3/1 | O'Connell St - S Ahead Left | U | 257 | 26.8: $26.8 \%$ | 15.0 | 3.9 |
| $3 / 3+3 / 4$ | O'Connell St - S Ahead Right | U+O | 783 | 76.7:76.7\% | 22.2 | 11.9 |
| 4/2+4/1 | Grose St - W Left Ahead | U | 72 | 14.8: 14.8\% | 34.3 | 1.0 |
| 4/3 | Grose St - W Ahead Right | 0 | 63 | 25.4\% | 48.1 | 1.8 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J3: O'Connell St/Victoria Rd | - | - | - | 79.1\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 390 | 53.4:53.4\% | 33.2 | 12.2 |
| 1/3+1/4 | O'Connell St - N Ahead Right | U+O | 432 | 59.5 : 0.0\% | 32.9 | 13.7 |


| 2/1 | Victoria Rd-E Left | U | 661 | 66.7\% | 20.8 | 22.5 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2/2 | Victoria Rd - E Right Ahead | U | 124 | 35.6\% | 62.1 | 3.2 |
| $3 / 2+3 / 1$ | O'Connell St - S Ahead Left | U | 894 | 70.3: 70.3\% | 15.0 | 18.3 |
| $3 / 4+3 / 3$ | O'Connell St - S Ahead Right | U | 359 | 79.1 : 0.0\% | 59.9 | 12.9 |
| 4/2+4/1 | Victoria Rd - W Left Ahead | U | 59 | 15.6: 15.6\% | 45.1 | 0.9 |
| 4/3 | Victoria Rd - W Ahead | U | 4 | 1.1\% | 44.4 | 0.1 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J4: Marist PI/Victoria Rd | - | - | - | 83.1\% | - | - |
| 1/2+1/1 | Villiers St - N Left Ahead | U | 110 | 35.9 : 35.9\% | 53.0 | 2.9 |
| 1/3 | Villiers St - N Ahead Right | U | 85 | 29.6\% | 53.0 | 2.7 |
| 2/2+2/1 | Victoria Rd - E Left Ahead | U | 687 | 83.1: 83.1\% | 55.8 | 19.9 |
| 2/3 | Victoria Rd - E Right Ahead | 0 | 205 | 51.1\% | 49.0 | 5.4 |
| $3 / 2+3 / 1$ | Marist PI - S Ahead Left | U | 293 | 67.7 : 67.7\% | 53.7 | 9.4 |
| 3/3 | Marist PI - S Ahead Right | U | 281 | 66.3\% | 53.7 | 9.3 |
| 4/2+4/1 | Victoria Rd - W Left Ahead | U | 418 | 51.0:51.0\% | 40.8 | 13.5 |
| 4/3 | Victoria Rd - W Ahead Right | 0 | 21 | 34.7\% | 73.2 | 0.4 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J5: Church St/Victoria Rd | - | - | - | 87.8\% | - | - |
| 1/2+1/1 | Church St - N Left | U | 352 | 23.9: 23.9\% | 14.3 | 2.5 |
| 1/3+1/4 | Church St - N Ahead Right | U | 414 | 76.5 : 82.4\% | 61.9 | 8.9 |
| 2/2+2/1 | Victoria Rd - E Left Ahead | U | 632 | 68.2 : 68.2\% | 11.5 | 18.7 |
| 2/3+2/4 | Victoria Rd - E Right Ahead | U | 732 | 81.8 : 81.8\% | 46.8 | 10.8 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 64 | 16.7 : 16.7\% | 44.2 | 1.6 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U | 314 | 77.1: 77.1\% | 64.6 | 7.2 |



GTA Basic Results Summary
Scenario 3: 'Ex-SAT' (FG3: 'Existing SAT', Plan 3: 'Existing SAT') Network Layout Diagram


GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - Victoria Road | - | - | - | 94.2\% | - | - |
| J1: O'Connell St/Albert St | - | - | - | 62.1\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 305 | 26.2 : $26.2 \%$ | 11.0 | 4.4 |
| 1/3 | O'Connell St - N Ahead | U | 226 | 19.4\% | 10.4 | 3.1 |
| 2/1 | Albert St - E Left | U | 188 | 62.1\% | 56.7 | 5.8 |
| 2/2 | Albert St - E Right Ahead | 0 | 37 | 14.7\% | 52.8 | 1.1 |
| 3/2+3/1 | O'Connell St - S Ahead Left | U | 58 | 12.0: 12.0\% | 6.6 | 0.2 |
| 3/3+3/4 | O'Connell St - S Ahead Right | U+O | 479 | 29.7 : 29.7\% | 4.7 | 2.0 |
| 4/2+4/1 | Albert St - W Left Ahead Right | O+U | 18 | 6.4:6.4\% | 51.1 | 0.4 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| J2: O'Connell St/Grose St | - | - | - | 67.0\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 412 | 35.3:35.3\% | 9.3 | 5.4 |
| 1/3+1/4 | O'Connell St - N Ahead Right | U+O | 301 | 25.0: $25.0 \%$ | 7.9 | 3.0 |
| 2/2+2/1 | Grose St - E Left Ahead | U | 121 | 45.9: 45.9\% | 59.6 | 3.5 |
| 2/3 | Grose St - E Right Ahead | $\bigcirc$ | 31 | 51.2\% | 115.5 | 1.5 |
| 3/2+3/1 | O'Connell St - S Ahead Left | U | 120 | 10.6: 10.6\% | 9.1 | 0.7 |
| 3/3+3/4 | O'Connell St - S Ahead Right | U+O | 523 | 42.5: 42.5\% | 9.5 | 6.4 |
| 4/2+4/1 | Grose St - W Left Ahead | U | 41 | 34.8:34.8\% | 69.0 | 0.9 |
| 4/3 | Grose St - W Ahead Right | $\bigcirc$ | 36 | 67.0\% | 151.1 | 2.1 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J3: O'Connell St/Victoria Rd | - | - | - | 94.2\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 421 | 46.3: 46.3\% | 18.7 | 11.3 |
| 1/3+1/4 | O'Connell St - N Ahead Right | U+O | 373 | 40.5: $40.5 \%$ | 25.1 | 17.4 |


| 2/1 | Victoria Rd-E Left | U | 413 | 58.1\% | 33.6 | 14.2 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2/2 | Victoria Rd - E Right Ahead | U | 77 | 29.9\% | 65.4 | 2.4 |
| $3 / 2+3 / 1$ | O'Connell St - S Ahead Left | U | 552 | 40.5 : 40.5\% | 7.3 | 6.6 |
| $3 / 4+3 / 3$ | O'Connell St - S Ahead Right | U | 342 | 94.2 : 0.0\% | 103.4 | 16.5 |
| 4/2+4/1 | Victoria Rd-W Left Ahead | U | 20 | 7.6 : 7.6\% | 51.6 | 0.5 |
| 4/3 | Victoria Rd - W Ahead | U | 8 | 3.1\% | 51.4 | 0.2 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J4: Marist Pl/Victoria Rd | - | - | - | 60.8\% | - | - |
| 1/2+1/1 | Villiers St - N Left Ahead | U | 87 | 31.6 : 31.6\% | 54.6 | 2.2 |
| 1/3 | Villiers St - N Ahead Right | U | 63 | 24.5\% | 54.6 | 2.0 |
| 2/2+2/1 | Victoria Rd - E Left Ahead | U | 534 | 60.8 : 60.8\% | 30.7 | 12.7 |
| 2/3 | Victoria Rd - E Right Ahead | O | 109 | 21.9\% | 29.0 | 2.3 |
| $3 / 2+3 / 1$ | Marist PI - S Ahead Left | U | 249 | 60.1: 60.1\% | 51.7 | 7.9 |
| 3/3 | Marist PI - S Ahead Right | U | 244 | 59.7\% | 52.0 | 7.9 |
| 4/2+4/1 | Victoria Rd - W Left Ahead | U | 295 | 34.1: 34.1\% | 29.7 | 9.9 |
| 4/3 | Victoria Rd - W Ahead Right | O | 87 | 31.7\% | 41.1 | 2.9 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J5: Church St/Victoria Rd | - | - | - | 90.3\% | - | - |
| 1/2+1/1 | Church St - N Left | U | 323 | 23.5 : 23.5\% | 16.0 | 2.6 |
| 1/3+1/4 | Church St - N Ahead Right | U | 472 | 89.1: 90.3\% | 75.8 | 13.6 |
| 2/2+2/1 | Victoria Rd - E Left Ahead | U | 447 | 48.0 : 48.0\% | 7.3 | 6.5 |
| $2 / 3+2 / 4$ | Victoria Rd - E Right Ahead | U | 434 | 58.2 : 58.2\% | 40.6 | 6.1 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 42 | 11.0: 11.0\% | 43.9 | 0.7 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U | 227 | 34.8: 49.8\% | 49.5 | 3.8 |



GTA Basic Results Summary GTA Basic Results Summary

User and Project Details

| Project: | 14S1091200 PNUR - Rezoning |
| :--- | :--- |
| Title: | Parramatta North Modelling - North |
| File name: | 141008Ing_Parramatta North modelling_North_FUT-AM.Isg3x |
| Company: | GTA Consultants Sydney |
| Address: | Lv6, 15 Help Street CHATSWOOD NSW 2067 |

GTA Basic Results Summary
Scenario 2: 'Fut-AM OPT' (FG1: 'Future AM + Dev (Sensitivity)', Plan 1: 'Existing - AM') Network Layout Diagram


GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - North | - - | - | - | 110.2\% | - | - |
| J1: Windsor Rd/James Rules Dr | - | - | - | 109.8\% | - | - |
| 1/1 | Windsor Rd - N Left | U | 1418 | 72.0\% | 5.3 | 16.6 |
| 1/3+1/2 | Windsor Rd - N Ahead | U | 668 | 97.2 : 97.2\% | 89.3 | 32.9 |
| 1/4+1/5 | Windsor Rd-N Ahead Right | U | 950 | 100.2 : 108.5\% | 134.5 | 47.3 |
| $2 / 2+2 / 1$ | James Rule Dr (off ramp) - E Right Left | U | 345 | 108.3 : 108.3\% | 254.2 | 33.2 |
| 2/3 | James Rule Dr (off ramp) - E Right | U | 195 | 103.3\% | 221.7 | 16.2 |
| 3/1+3/2 | Church St - S Ahead Left | U | 669 | 40.6 : 41.4\% | 2.4 | 0.9 |
| 3/3 | Church St - S Ahead | U | 537 | 100.7\% | 124.9 | 32.1 |
| $3 / 4+3 / 5$ | Church St - S Ahead Right | U | 112 | 0.0 : 102.3\% | 263.4 | 10.1 |
| 4/2+4/1 | Briens Rd (off ramp) - W Left Right | U | 339 | 108.7 : 108.7\% | 251.9 | 28.3 |
| 4/3 | Briens Rd (off ramp) - W Right | U | 354 | 109.8\% | 278.8 | 34.5 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| Ped Link: P5 | P6 | - | 0 | 0.0\% | - | - |
| Ped Link: P6 | P7 | - | 0 | 0.0\% | - | - |
| Ped Link: P7 | P8 | - | 0 | 0.0\% | - | - |
| Ped Link: P8 | P5 | - | 0 | 0.0\% | - | - |
| J2: Windsor Rd/The Junction | - - | - | - | 81.3\% | - | - |
| 1/1 | Windsor Rd (N) Ahead | U | 18 | 1.2\% | 1.3 | 0.0 |
| 1/2 | Windsor Rd (N) Ahead | 0 | 1197 | 81.3\% | 10.2 | 20.7 |
| 1/3+1/4 | Windsor Rd (N) Ahead Right | U | 823 | 54.1 : 55.1\% | 3.2 | 1.4 |
| 2/2+2/1 | Windsor Rd-S Ahead Left | U+O | 706 | 52.1 : 52.2\% | 8.3 | 10.3 |
| 2/3 | Windsor Rd-S Ahead | U | 530 | 39.5\% | 6.4 | 7.6 |
| 2/4 | Windsor Rd-S Ahead | U | 112 | 8.4\% | 4.2 | 0.9 |
| $3 / 2+3 / 1$ | The Junction Access - W Left Right | U | 138 | 68.7 : 68.7\% | 84.6 | 5.4 |

GTA Basic Results Summary

| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| J3: Church St/North Rocks Rd | - | - | - | 99.0\% | - | - |
| 1/1 | Windsor Rd - N Left | U | 447 | 35.2\% | 6.0 | 3.0 |
| 1/2 | Windsor Rd-N Ahead | U | 18 | 2.1\% | 4.6 | 0.1 |
| 1/3 | Windsor Rd - N Ahead | U | 829 | 94.0\% | 48.1 | 32.9 |
| 1/4 | Windsor Rd - N Ahead | U | 835 | 94.1\% | 52.8 | 32.6 |
| $2 / 2+2 / 1$ | North Rocks Rd (E) Right Left | U | 565 | 95.3: 95.3\% | 81.5 | 22.1 |
| 2/3 | North Rocks Rd (E) Right | U | 256 | 87.5\% | 93.6 | 12.2 |
| 3/1 | Church St - S Ahead | U | 32 | 2.5\% | 6.5 | 0.3 |
| 3/2 | Church St - S Ahead | U | 470 | 36.2\% | 5.7 | 3.2 |
| 3/3 | Church St - S Ahead | U | 516 | 39.8\% | 5.6 | 3.4 |
| 3/5+3/4 | Church St - S Ahead Right | U | 245 | 99.0: 0.0\% | 147.1 | 14.7 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J4: Church St/Board St/Seville St | - | - | - | 110.2\% | - | - |
| 1/1 | Church St - N Left Ahead | U | 41 | 2.2\% | 1.0 | 0.0 |
| 1/2 | Church St - N Ahead | U | 986 | 53.7\% | 2.2 | 0.6 |
| 1/3 | Church St - N Ahead | U | 996 | 53.7\% | 2.9 | 1.6 |
| 2/1 | Seville St - E Left | O | 3 | 1.0\% | 14.3 | 0.0 |
| 3/1 | Church St - S Ahead Left | U | 46 | 2.6\% | 1.0 | 0.0 |
| 3/2 | Church St - S Ahead | U | 442 | 24.6\% | 1.3 | 0.2 |
| 3/3 | Church St - S Ahead | U | 434 | 24.1\% | 1.3 | 0.2 |
| 4/1 | Board St - W Left | O | 353 | 110.2\% | 253.9 | 59.7 |
| J5: Church St/Barney St | - | - | - | 91.7\% | - | - |
| 1/2+1/1 | Church St - N Left Ahead | U | 96 | 7.4 : 7.2\% | 6.9 | 0.6 |
| 1/3 | Church St - N Ahead | U | 1067 | 77.5\% | 12.8 | 11.7 |
| 1/4 | Church St - N Right | 0 | 842 | 91.7\% | 32.6 | 23.8 |
| 2/1+2/2 | Barney St - E Right Left Ahead | U | 114 | 82.9 : 82.9\% | 123.5 | 5.5 |

## GTA Basic Results Summary



GTA Basic Results Summary GTA Basic Results Summary

User and Project Details

| Project: | 14S1091200 PNUR - Rezoning |
| :--- | :--- |
| Title: | Parramatta North Modelling - North |
| File name: | 141008Ing_Parramatta North modelling_North_FUT-PMSAT.Isg3x |
| Company: | GTA Consultants Sydney |
| Address: | Lv6, 15 Help Street CHATSWOOD NSW 2067 |

## Scenario 3: 'Future PM OPT' (FG1: 'Future PM + Dev (Sensitivity)', Plan 2: 'Existing - PM') Network Layout Diagram



GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - North | - | - | - | 229.5\% | - | - |
| J1: Windsor Rd/James Rules Dr | - | - | - | 229.5\% | - | - |
| 1/1 | Windsor Rd - N Left | 0 | 818 | 50.3\% | 3.3 | 5.7 |
| 1/3+1/2 | Windsor Rd - N Ahead | U | 474 | 68.6 : 68.6\% | 43.2 | 15.3 |
| 1/4+1/5 | Windsor Rd - N Ahead Right | U | 606 | 62.7 : 229.5\% | 402.7 | 67.2 |
| 2/2+2/1 | James Rule Dr (off ramp) - E Right Left | U | 391 | 103.0: 103.0\% | 178.3 | 29.7 |
| 2/3 | James Rule Dr (off ramp) - E Right | U | 381 | 101.3\% | 159.6 | 25.4 |
| 3/1+3/2 | Church St - S Ahead Left | O+U | 857 | 50.5 : 52.2\% | 2.7 | 8.1 |
| 3/3 | Church St - S Ahead | U | 790 | 99.7\% | 102.0 | 40.6 |
| $3 / 4+3 / 5$ | Church St - S Ahead Right | U | 801 | 101.8 : 96.8\% | 113.5 | 46.9 |
| 4/1+4/2 | Briens Rd (off ramp) - W Left Right | U | 561 | 89.3 : 123.8\% | 293.0 | 48.2 |
| 4/3 | Briens Rd (off ramp) - W Right | U | 353 | 125.1\% | 481.7 | 53.6 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| Ped Link: P5 | P6 | - | 0 | 0.0\% | - | - |
| Ped Link: P6 | P7 | - | 0 | 0.0\% | - | - |
| Ped Link: P7 | P8 | - | 0 | 0.0\% | - | - |
| Ped Link: P8 | P4 | - | 0 | 0.0\% | - | - |
| J2: Windsor Rd/The Junction | - | - | - | 72.2\% | - | - |
| 1/1 | Windsor Rd (N) Ahead | U | 442 | 28.9\% | 2.5 | 0.8 |
| 1/2 | Windsor Rd (N) Ahead | U | 626 | 41.1\% | 2.7 | 1.0 |
| 1/3+1/4 | Windsor Rd (N) Ahead Right | U | 514 | 61.5 : 63.0\% | 12.3 | 2.9 |
| 2/2+2/1 | Windsor Rd-S Ahead Left | U+O | 941 | 72.2 :71.9\% | 12.3 | 10.4 |
| 2/3 | Windsor Rd - S Ahead | U | 694 | 52.4\% | 7.5 | 5.3 |
| 2/4 | Windsor Rd - S Ahead | U | 801 | 63.7\% | 14.6 | 21.9 |
| 3/2+3/1 | The Junction Access - W Left Right | U | 300 | 70.5 : 70.5\% | 64.5 | 6.9 |


| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| J3: Church St/North Rocks Rd | - | - | - | 79.4\% | - | - |
| 1/1 | Windsor Rd - N Left | U | 465 | 37.3\% | 6.0 | 5.8 |
| 1/2 | Windsor Rd-N Ahead | U | 16 | 2.1\% | 19.0 | 0.2 |
| 1/3 | Windsor Rd - N Ahead | U | 638 | 78.7\% | 33.6 | 12.3 |
| 1/4 | Windsor Rd - N Ahead | U | 560 | 68.7\% | 37.7 | 15.6 |
| 2/2+2/1 | North Rocks Rd (E) Right Left | U | 391 | 72.7 : 72.7\% | 46.0 | 9.6 |
| 2/3 | North Rocks Rd (E) Right | U | 311 | 77.2\% | 67.7 | 12.4 |
| 3/1 | Church St - S Ahead | U | 17 | 1.4\% | 10.7 | 0.2 |
| 3/2 | Church St - S Ahead | U | 871 | 67.4\% | 12.6 | 14.3 |
| 3/3 | Church St - S Ahead | U | 609 | 45.6\% | 6.9 | 7.0 |
| 3/5+3/4 | Church St - S Ahead Right | U | 837 | 77.7 : 79.4\% | 25.0 | 11.0 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J4: Church St/Board St/Seville St | - | - | - | 102.0\% | - | - |
| 1/1 | Church St - N Left Ahead | U | 360 | 18.7\% | 1.2 | 0.1 |
| 1/2 | Church St - N Ahead | U | 364 | 19.0\% | 1.2 | 0.1 |
| 1/3 | Church St - N Ahead | U | 679 | 35.3\% | 1.5 | 0.3 |
| 2/1 | Seville St - E Left | O | 9 | 1.9\% | 3.9 | 0.0 |
| 3/1 | Church St - S Ahead Left | U | 54 | 3.0\% | 1.0 | 0.0 |
| 3/2 | Church St - S Ahead | U | 939 | 49.8\% | 2.0 | 0.5 |
| 3/3 | Church St - S Ahead | U | 940 | 49.0\% | 2.0 | 0.5 |
| 4/1 | Board St - W Left | 0 | 406 | 102.0\% | 126.9 | 44.8 |
| J5: Church St/Barney St | - | - | - | 126.6\% | - | - |
| 1/2+1/1 | Church St - N Left Ahead | U | 118 | 10.5 : 9.9\% | 8.9 | 0.6 |
| 1/3 | Church St - N Ahead | U | 735 | 58.9\% | 10.1 | 5.7 |
| 1/4 | Church St - N Right | O | 546 | 95.1\% | 67.7 | 24.7 |
| 2/1+2/2 | Barney St - E Right Left Ahead | U | 185 | 64.4 : 64.4\% | 67.6 | 7.0 |

GTA Basic Results Summary


GTA Basic Results Summary
Scenario 4: 'Future SAT OPT' (FG2: 'Future SAT + Dev (Sensitivity)', Plan 3: 'Existing - SAT')

## Network Layout Diagram



GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - North | - | - | - | 176.2\% | - | - |
| J1: Windsor Rd/James Rules Dr | - | - | - | 176.2\% | - | - |
| 1/1 | Windsor Rd - N Left | 0 | 954 | 59.1\% | 4.1 | 8.1 |
| 1/3+1/2 | Windsor Rd - N Ahead | U | 421 | 93.2 : 93.2\% | 91.7 | 19.7 |
| 1/4+1/5 | Windsor Rd - N Ahead Right | U | 693 | 101.3: 176.2\% | 389.1 | 75.1 |
| 2/2+2/1 | James Rule Dr (off ramp) - E Right Left | U | 431 | 100.9 : 100.9\% | 143.0 | 26.6 |
| 2/3 | James Rule Dr (off ramp) - E Right | U | 408 | 98.2\% | 124.0 | 23.0 |
| 3/1+3/2 | Church St - S Ahead Left | O+U | 773 | 47.6 : 47.6\% | 2.5 | 0.7 |
| 3/3 | Church St - S Ahead | U | 467 | 94.9\% | 100.0 | 22.5 |
| $3 / 4+3 / 5$ | Church St - S Ahead Right | U | 544 | 94.6: 86.0\% | 70.0 | 21.8 |
| 4/1+4/2 | Briens Rd (off ramp) - W Left Right | U | 614 | 60.9 : 102.8\% | 78.3 | 19.3 |
| 4/3 | Briens Rd (off ramp) - W Right | U | 385 | 95.9\% | 110.8 | 20.2 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| Ped Link: P5 | P6 | - | 0 | 0.0\% | - | - |
| Ped Link: P6 | P7 | - | 0 | 0.0\% | - | - |
| Ped Link: P7 | P8 | - | 0 | 0.0\% | - | - |
| Ped Link: P8 | P4 | - | 0 | 0.0\% | - | - |
| J2: Windsor Rd/The Junction | - | - | - | 76.1\% | - | - |
| 1/1 | Windsor Rd ( N ) Ahead | U | 452 | 31.3\% | 2.7 | 1.3 |
| 1/2 | Windsor Rd (N) Ahead | U | 680 | 47.3\% | 3.4 | 2.2 |
| 1/3+1/4 | Windsor Rd ( N ) Ahead Right | U | 597 | 76.1 : 76.1\% | 19.5 | 6.9 |
| 2/2+2/1 | Windsor Rd - S Ahead Left | U+O | 823 | 70.0 : 70.0\% | 14.4 | 15.7 |
| 2/3 | Windsor Rd - S Ahead | U | 384 | 33.4\% | 5.6 | 1.5 |
| 2/4 | Windsor Rd - S Ahead | U | 544 | 47.3\% | 18.6 | 14.1 |
| 3/2+3/1 | The Junction Access - W Left Right | U | 278 | 61.1 : 61.1\% | 55.7 | 5.1 |

GTA Basic Results Summary

| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| J3: Church St/North Rocks Rd | - | - | - | 86.5\% | - | - |
| 1/1 | Windsor Rd - N Left | U | 473 | 38.8\% | 7.4 | 6.9 |
| 1/2 | Windsor Rd-N Ahead | U | 8 | 1.0\% | 13.8 | 0.1 |
| 1/3 | Windsor Rd - N Ahead | U | 688 | 86.5\% | 41.0 | 16.1 |
| 1/4 | Windsor Rd - N Ahead | U | 538 | 67.6\% | 41.7 | 14.8 |
| $2 / 2+2 / 1$ | North Rocks Rd (E) Right Left | U | 448 | 81.5 : 81.5\% | 52.5 | 12.3 |
| 2/3 | North Rocks Rd (E) Right | U | 259 | 62.4\% | 56.3 | 9.2 |
| 3/1 | Church St - S Ahead | U | 7 | 0.6\% | 7.5 | 0.1 |
| $3 / 2$ | Church St - S Ahead | U | 631 | 51.8\% | 6.6 | 6.6 |
| 3/3 | Church St - S Ahead | U | 380 | 31.2\% | 4.9 | 3.3 |
| 3/5+3/4 | Church St - S Ahead Right | U | 495 | 78.3 : 78.3\% | 28.8 | 8.0 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J4: Church St/Board St/Seville St | - | - | - | 67.6\% | - | - |
| 1/1 | Church St - N Left Ahead | U | 359 | 19.8\% | 1.2 | 0.1 |
| 1/2 | Church St - N Ahead | U | 365 | 20.2\% | 1.3 | 0.1 |
| 1/3 | Church St - N Ahead | U | 723 | 39.9\% | 1.7 | 0.6 |
| 2/1 | Seville St - E Left | O | 24 | 5.3\% | 4.2 | 0.0 |
| 3/1 | Church St - S Ahead Left | U | 29 | 1.6\% | 1.0 | 0.0 |
| 3/2 | Church St - S Ahead | U | 565 | 31.4\% | 1.5 | 0.2 |
| 3/3 | Church St - S Ahead | U | 565 | 31.4\% | 1.5 | 0.2 |
| 4/1 | Board St - W Left | O | 371 | 67.6\% | 12.5 | 6.1 |
| J5: Church St/Barney St | - | - | - | 92.1\% | - | - |
| 1/2+1/1 | Church St - N Left Ahead | U | 108 | 8.9 : 8.9\% | 10.3 | 1.2 |
| 1/3 | Church St - N Ahead | U | 745 | 58.7\% | 13.4 | 9.3 |
| 1/4 | Church St - N Right | 0 | 587 | 87.0\% | 45.5 | 16.9 |
| 2/1+2/2 | Barney St - E Right Left Ahead | U | 112 | 55.0 : 55.0\% | 70.6 | 3.6 |

## GTA Basic Results Summary



GTA Basic Results Summary GTA Basic Results Summary

User and Project Details

| Project: | 14S1091200 PNUR - Rezoning |
| :--- | :--- |
| Title: | Parramatta North Modelling - PHR |
| File name: | 141001Ing_Parramatta North modelling_PHR_FUT.Isg3x |
| Company: | GTA Consultants Sydney |
| Address: | Lv6, 15 Help Street CHATSWOOD NSW 2067 |

Scenario 4: 'FUT-AM OPT' (FG1: 'Future AM + Dev (Sensitivity)', Plan 1: 'Network Control Plan 1')

## Network Layout Diagram



GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - PHR | - | - | - | 167.5\% | - | - |
| J1: Church St/Factory St | - | - | - | 167.5\% | - | - |
| 1/2+1/1 | Church St - N Left Ahead | U | 45 | 3.6 : 3.6\% | 8.1 | 0.4 |
| 1/3+1/4 | Church St - N Ahead Right | U | 1068 | 84.7 : 84.7\% | 25.0 | 29.1 |
| 2/2+2/1 | Factory St -E Left Ahead | U | 39 | 15.6 : 15.6\% | 53.9 | 0.8 |
| 2/3 | Factory St -E Right Ahead | O | 16 | 14.5\% | 66.9 | 0.6 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 40 | 3.2 : 3.2\% | 10.1 | 0.5 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U+O | 530 | 42.2 : 42.2\% | 8.9 | 4.7 |
| 4/2+4/1 | Factory St - W Left Ahead | U | 120 | 35.9 : 35.9\% | 52.6 | 3.9 |
| 4/3 | Factory St - W Ahead Right | 0 | 288 | 167.5\% | 891.0 | 76.1 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J2: Church St/Pennant Hills Rd | - | - | - | 115.2\% | - | - |
| $1 / 2+1 / 1$ | Church St - N U-Turn Ahead | U+O | 40 | 4.1 : 0.0\% | 14.0 | 0.6 |
| 1/4+1/3 | Church St - N U-Turn Ahead | U+O | 1329 | 115.2 : 115.1\% | 288.4 | 137.1 |
| 2/1 | Pennant Hills Rd-NE Ahead | U | 493 | 97.0\% | 100.3 | 21.7 |
| 2/2 | Pennant Hills Rd-NE Right | U | 454 | 94.8\% | 93.0 | 21.4 |
| 3/1 | Albert St - E Left | O | 37 | 19.0\% | 11.4 | 0.1 |
| 4/2+4/1 | Church St - S Ahead Left | U | 33 | 2.9 : 2.9\% | 10.1 | 0.4 |
| 4/3+4/4 | Church St - S Ahead Right | U | 700 | 55.1: 97.5\% | 22.1 | 14.3 |
| 5/2+5/1 | Albert St - W Left Left2 | U | 189 | 39.3 : 39.3\% | 43.4 | 5.6 |
| 5/3 | Albert St - W Left | U | 184 | 38.4\% | 43.3 | 5.5 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |

GTA Basic Results Summary

| J3: Church St/Grose St | - | - | - | 101.6\% | - |  | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1/2+1/1 | Church St - N Left Ahead | U | 88 | 7.4 : 5.9\% | 7.1 |  | 0.5 |
| 1/3+1/4 | Church St - N Ahead Right | U+O | 1573 | 97.8: 100.0\% | 46.8 |  | 47.4 |
| $2 / 2+2 / 1$ | Grose St - E Left Ahead | U | 199 | 75.3 : 75.3\% | 76.4 |  | 7.9 |
| 2/3 | Grose St - E Right Ahead | 0 | 59 | 101.6\% | 316.9 |  | 5.9 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 91 | 7.7 : 7.7\% | 9.3 |  | 0.6 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U+O | 626 | 51.4 : 51.4\% | 14.5 |  | 10.0 |
| $4 / 2+4 / 1$ | Grose St - W Left Ahead | U | 247 | 92.5 : 92.5\% | 113.5 |  | 12.5 |
| 4/3 | Grose St - W Ahead Right | 0 | 48 | 73.5\% | 150.1 |  | 2.7 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P2 | 2 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P3 | 3 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P4 | 4 | - | 0 | 0.0\% | - |  | - |
| C1 - Church St/Factory St C2 - Church St/Pennant Hills Rd C3 - Church St/Grose St | PRC for Signalled Lanes (\%): <br> PRC for Signalled Lanes (\%): <br> PRC for Signalled Lanes (\%): <br> PRC Over All Lanes (\%): | $\begin{aligned} & -86.1 \\ & -28.0 \\ & -12.9 \\ & -86.1 \end{aligned}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  | $\begin{array}{r} 82.84 \\ 131.67 \\ 39.71 \\ 254.34 \end{array}$ | $\begin{array}{ll} \text { Cycle Time (s): } & 124 \\ \text { Cycle Time (s): } & 124 \\ \text { Cycle Time (s): } & 124 \end{array}$ |  |

GTA Basic Results Summary
Scenario 5: 'FUT-PM OPT' (FG2: 'Future PM + Dev (Sensitivity)', Plan 1: 'Network Control Plan 1')

## Network Layout Diagram



GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - PHR | - | - | - | 133.0\% | - | - |
| J1: Church St/Factory St | - | - | - | 86.7\% | - | - |
| 1/2+1/1 | Church St - N Left Ahead | U | 42 | 3.6 : 3.6\% | 8.6 | 0.3 |
| 1/3+1/4 | Church St - N Ahead Right | U | 776 | 61.5 : 86.7\% | 19.5 | 11.9 |
| 2/2+2/1 | Factory St -E Left Ahead | U | 34 | 14.4 : 14.4\% | 47.2 | 0.5 |
| 2/3 | Factory St -E Right Ahead | O | 20 | 16.4\% | 60.4 | 0.6 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 50 | 4.3 : 3.9\% | 9.7 | 0.4 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U+O | 1083 | 82.5 : 83.9\% | 15.6 | 8.5 |
| 4/2+4/1 | Factory St - W Left Ahead | U | 79 | 22.6 : 22.6\% | 43.4 | 1.9 |
| 4/3 | Factory St - W Ahead Right | 0 | 80 | 53.1\% | 70.7 | 2.7 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J2: Church St/Pennant Hills Rd | - | - | - | 133.0\% | - | - |
| 1/2+1/1 | Church St - N U-Turn Ahead | U+O | 32 | 3.6 : 0.0\% | 21.2 | 0.6 |
| 1/4+1/3 | Church St - N U-Turn Ahead | U+O | 770 | 78.5 : 78.5\% | 23.4 | 12.3 |
| 2/1 | Pennant Hills Rd - NE Ahead | U | 315 | 58.0\% | 33.8 | 7.2 |
| 2/2 | Pennant Hills Rd - NE Right | U | 587 | 133.0\% | 545.8 | 98.8 |
| 3/1 | Albert St - E Left | O | 25 | 8.7\% | 6.9 | 0.1 |
| 4/2+4/1 | Church St - S Ahead Left | U | 45 | 3.9 : 3.9\% | 6.2 | 0.2 |
| 4/3+4/4 | Church St - S Ahead Right | U | 1412 | 107.4 : 108.0\% | 170.9 | 98.0 |
| 5/2+5/1 | Albert St - W Left Left2 | U | 175 | 39.3 : 39.3\% | 40.0 | 4.5 |
| 5/3 | Albert St - W Left | U | 168 | 38.1\% | 39.9 | 4.4 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |

GTA Basic Results Summary

| J3: Church St/Grose St | - | - | - | 129.7\% | - |  | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1/2+1/1 | Church St - N Left Ahead | U | 84 | 7.4 : 7.4\% | 9.7 |  | 0.5 |
| 1/3+1/4 | Church St - N Ahead Right | U+O | 915 | 72.3 : 79.9\% | 20.2 |  | 11.9 |
| $2 / 2+2 / 1$ | Grose St - E Left Ahead | U | 162 | 61.7 : 61.7\% | 58.6 |  | 5.0 |
| 2/3 | Grose St - E Right Ahead | 0 | 97 | 129.7\% | 571.1 |  | 16.5 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 80 | 6.9 : 6.9\% | 9.2 |  | 0.6 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U+O | 1240 | 98.7 : 98.7\% | 59.0 |  | 46.4 |
| 4/2+4/1 | Grose St - W Left Ahead | U | 268 | 95.4 : 95.4\% | 117.4 |  | 12.8 |
| 4/3 | Grose St - W Ahead Right | O | 95 | 86.1\% | 137.9 |  | 5.1 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P2 | 2 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P3 | 3 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P4 | 4 | - | 0 | 0.0\% | - |  | - |
| C1 - Church St/Factory St C2 - Church St/Pennant Hills Rd C3 - Church St/Grose St | PRC for Signalled Lanes (\%): <br> PRC for Signalled Lanes (\%): <br> PRC for Signalled Lanes (\%): <br> PRC Over All Lanes (\%): | $\begin{array}{r} 3.9 \\ -47.7 \\ -44.2 \\ -47.7 \end{array}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  | $\begin{array}{r} 12.03 \\ 166.99 \\ 56.29 \\ 235.35 \end{array}$ | Cycle Time (s): 106 <br> Cycle Time (s): 106 <br> Cycle Time (s): 106 |  |

GTA Basic Results Summary
Scenario 6: 'FUT-SAT OPT' (FG3: 'Future SAT + Dev (Sensitivity)', Plan 1: 'Network Control Plan 1')
Network Layout Diagram


GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - PHR | - | - | - | 92.6\% | - | - |
| J1: Church St/Factory St | - | - | - | 80.7\% | - | - |
| 1/2+1/1 | Church St - N Left Ahead | U | 34 | 3.2 : 3.2\% | 10.8 | 0.2 |
| 1/3+1/4 | Church St - N Ahead Right | U | 807 | 69.0 : 80.7\% | 21.9 | 15.8 |
| $2 / 2+2 / 1$ | Factory St -E Left Ahead | U | 49 | 14.3: 14.3\% | 41.3 | 0.7 |
| 2/3 | Factory St -E Right Ahead | O | 21 | 11.2\% | 49.3 | 0.6 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 25 | 2.3 : 2.3\% | 12.6 | 0.2 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U+O | 735 | 65.0 : 65.0\% | 11.8 | 5.8 |
| 4/2+4/1 | Factory St - W Left Ahead | U | 111 | 26.0 : 26.0\% | 40.2 | 2.9 |
| 4/3 | Factory St - W Ahead Right | 0 | 181 | 78.2\% | 78.8 | 7.0 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J2: Church St/Pennant Hills Rd | - | - | - | 92.6\% | - | - |
| 1/2+1/1 | Church St - N U-Turn Ahead | U+O | 18 | 2.0:0.0\% | 17.0 | 0.3 |
| 1/4+1/3 | Church St - N U-Turn Ahead | U+O | 946 | 92.6:92.6\% | 33.1 | 17.1 |
| 2/1 | Pennant Hills Rd-NE Ahead | U | 317 | 58.7\% | 37.1 | 7.2 |
| 2/2 | Pennant Hills Rd-NE Right | U | 434 | 88.4\% | 66.6 | 16.1 |
| 3/1 | Albert St - E Left | O | 29 | 10.9\% | 7.6 | 0.1 |
| 4/2+4/1 | Church St - S Ahead Left | U | 23 | 2.1 : 2.1\% | 9.0 | 0.2 |
| 4/3+4/4 | Church St - S Ahead Right | U | 923 | 75.1 : 86.9\% | 21.8 | 8.8 |
| 5/2+5/1 | Albert St - W Left Left2 | U | 134 | 27.0 : 27.0\% | 36.4 | 3.2 |
| 5/3 | Albert St - W Left | U | 129 | 26.3\% | 36.3 | 3.3 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |

GTA Basic Results Summary

| J3: Church St/Grose St | - | - | - | 83.7\% | - |  | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1/2+1/1 | Church St - N Left Ahead | U | 64 | 5.8 : 5.8\% | 10.3 |  | 0.5 |
| 1/3+1/4 | Church St - N Ahead Right | U+O | 1031 | 83.7 : 83.7\% | 19.9 |  | 15.0 |
| $2 / 2+2 / 1$ | Grose St - E Left Ahead | U | 113 | 39.6 : 39.6\% | 50.6 |  | 3.2 |
| 2/3 | Grose St - E Right Ahead | 0 | 95 | 78.8\% | 114.1 |  | 4.5 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 52 | 4.7 : 4.7\% | 9.9 |  | 0.3 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U+O | 801 | 69.1: 69.1\% | 17.6 |  | 15.2 |
| $4 / 2+4 / 1$ | Grose St - W Left Ahead | U | 159 | 52.6 : 52.6\% | 52.0 |  | 4.3 |
| 4/3 | Grose St - W Ahead Right | 0 | 60 | 35.2\% | 60.6 |  | 2.0 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P2 | 2 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P3 | 3 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P4 | 4 | - | 0 | 0.0\% | - |  | - |
| C1 - Church St/Factory St C2 - Church St/Pennant Hills Rd C3 - Church St/Grose St | $\begin{aligned} & \text { PRC for Signalled Lanes (\%): } \\ & \text { PRC for Signalled Lanes (\%): } \\ & \text { PRC for Signalled Lanes (\%): } \\ & \text { PRC Over All Lanes (\%): } \end{aligned}$ | $\begin{array}{r} 11.6 \\ -2.9 \\ 7.5 \\ -2.9 \end{array}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  | $\begin{aligned} & 13.55 \\ & 28.36 \\ & 17.87 \\ & 59.84 \end{aligned}$ | Cycle Time (s): 110 <br> Cycle Time (s): 110 <br> Cycle Time (s): 110 |  |

GTA Basic Results Summary GTA Basic Results Summary

## User and Project Details

| Project: | 14S1091200 PNUR - Rezoning |
| :--- | :--- |
| Title: | Parramatta North Modelling - Victoria Road |
| Location: | North Parramatta - Victoria Road |
| File name: | 141001Ing_Parramatta North modelling_South_FUT.Isg3x |

Scenario 4: 'FUT-AM OPT' (FG1: 'Future AM + Dev (Sensitivity)', Plan 1: 'Existing AM') Network Layout Diagram


GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - Victoria Road | - | - | - | 104.5\% | - | - |
| J1: O'Connell St/Albert St | - | - | - | 60.3\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 511 | 60.3:60.3\% | 28.6 | 13.1 |
| 1/3 | O'Connell St - N Ahead | U | 481 | 56.8\% | 27.7 | 12.0 |
| 2/1 | Albert St - E Left | U | 305 | 49.2\% | 29.6 | 7.3 |
| 2/2 | Albert St - E Right Ahead | 0 | 74 | 26.7\% | 48.1 | 2.2 |
| 3/2+3/1 | O'Connell St - S Ahead Left | U | 233 | 54.0:54.0\% | 15.5 | 3.0 |
| 3/3+3/4 | O'Connell St - S Ahead Right | U+O | 476 | 49.5:49.4\% | 17.7 | 6.3 |
| 4/2+4/1 | Albert St - W Left Ahead Right | O+U | 226 | 59.9 : 59.9\% | 54.3 | 7.4 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| J2: O'Connell St/Grose St | - | - | - | 72.5\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 705 | 58.9: 58.9\% | 9.8 | 7.5 |
| 1/3+1/4 | O'Connell St - N Ahead Right | U+O | 696 | 57.4:57.4\% | 10.0 | 7.6 |
| 2/2+2/1 | Grose St - E Left Ahead | U | 194 | 62.8:62.8\% | 59.1 | 6.1 |
| 2/3 | Grose St - E Right Ahead | $\bigcirc$ | 61 | 39.4\% | 69.7 | 2.2 |
| 3/2+3/1 | O'Connell St - S Ahead Left | U | 286 | 23.7 : $23.7 \%$ | 8.4 | 2.5 |
| 3/3+3/4 | O'Connell St - S Ahead Right | U+O | 684 | 72.5: 72.5\% | 22.4 | 5.8 |
| 4/2+4/1 | Grose St - W Left Ahead | U | 37 | 17.3: 17.3\% | 56.6 | 1.1 |
| 4/3 | Grose St - W Ahead Right | $\bigcirc$ | 37 | 68.0\% | 145.9 | 2.1 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J3: O'Connell St/Victoria Rd | - | - | - | 104.5\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 761 | 97.6:97.6\% | 69.3 | 34.4 |
| 1/3+1/4 | O'Connell St - N Ahead Right | U+O | 784 | 97.7: 97.7\% | 70.3 | 35.4 |


| 2/1 | Victoria Rd-E Left | U | 494 | 58.3\% | 25.9 | 16.6 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2/2 | Victoria Rd - E Right Ahead | U | 281 | 103.2\% | 168.4 | 20.4 |
| $3 / 2+3 / 1$ | O'Connell St - S Ahead Left | U | 1005 | 74.4: 74.4\% | 13.5 | 20.0 |
| $3 / 4+3 / 3$ | O'Connell St - S Ahead Right | U | 576 | 104.5:0.0\% | 176.0 | 40.1 |
| 4/2+4/1 | Victoria Rd - W Left Ahead | U | 53 | 19.3: 19.3\% | 52.2 | 1.6 |
| 4/3 | Victoria Rd - W Ahead | U | 77 | 28.3\% | 54.0 | 2.4 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J4: Marist PI/Victoria Rd | - | - | - | 70.8\% | - | - |
| 1/2+1/1 | Villiers St - N Left Ahead | U | 135 | 65.6 : 65.6\% | 75.6 | 4.4 |
| 1/3 | Villiers St - N Ahead Right | U | 109 | 60.1\% | 75.5 | 4.2 |
| $2 / 2+2 / 1$ | Victoria Rd - E Left Ahead | U | 598 | 55.2 : 55.2\% | 13.7 | 6.2 |
| 2/3 | Victoria Rd - E Right Ahead | 0 | 276 | 51.9\% | 24.1 | 7.1 |
| $3 / 2+3 / 1$ | Marist PI - S Ahead Left | U | 176 | 63.0 : 63.0\% | 64.3 | 6.0 |
| 3/3 | Marist PI - S Ahead Right | U | 168 | 61.7\% | 64.3 | 6.0 |
| $4 / 2+4 / 1$ | Victoria Rd - W Left Ahead | U | 538 | 48.3 : 48.0\% | 25.0 | 15.1 |
| 4/3 | Victoria Rd - W Ahead Right | 0 | 329 | 70.8\% | 33.4 | 7.8 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J5: Church St/Victoria Rd | - | - | - | 98.6\% | - | - |
| $1 / 2+1 / 1$ | Church St - N Left | U | 833 | 57.3 : 57.3\% | 18.5 | 9.0 |
| 1/3+1/4 | Church St - N Ahead Right | U | 537 | 97.1: 98.6\% | 106.6 | 20.9 |
| 2/2+2/1 | Victoria Rd - E Left Ahead | U | 495 | 58.1 : 58.1\% | 6.6 | 5.7 |
| $2 / 3+2 / 4$ | Victoria Rd - E Right Ahead | U | 667 | 92.6: 92.6\% | 52.8 | 29.4 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 51 | 11.5: 11.5\% | 39.7 | 1.3 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U | 240 | 60.3: 60.3\% | 54.9 | 5.3 |



GTA Basic Results Summary
Scenario 5: 'FUT-PM OPT' (FG2: 'Future PM + Dev (Sensitivity)', Plan 2: 'Existing PM')
Network Layout Diagram


GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU ( $\mathbf{s} / \mathrm{pcu}$ ) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - Victoria Road | - | - | - | 119.6\% | - | - |
| J1: O'Connell St/Albert St | - | - | - | 70.9\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 263 | 28.0 : 28.0\% | 18.6 | 5.0 |
| 1/3 | O'Connell St - N Ahead | U | 231 | 24.6\% | 18.2 | 4.3 |
| 2/1 | Albert St - E Left | U | 266 | 50.2\% | 36.5 | 6.6 |
| 2/2 | Albert St - E Right Ahead | 0 | 286 | 70.9\% | 56.4 | 9.7 |
| 3/2+3/1 | O'Connell St - S Ahead Left | U | 308 | 68.3 : 68.0\% | 21.0 | 7.6 |
| 3/3+3/4 | O'Connell St - S Ahead Right | U+O | 932 | 65.5 : 65.6\% | 12.8 | 7.1 |
| 4/2+4/1 | Albert St - W Left Ahead Right | O+U | 65 | 23.1 : $23.1 \%$ | 43.2 | 1.8 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| J2: O'Connell St/Grose St | - | - | - | 85.2\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 412 | 37.8:37.8\% | 12.4 | 6.1 |
| 1/3+1/4 | O'Connell St - N Ahead Right | U+O | 375 | 33.7 : 33.7\% | 13.0 | 5.4 |
| 2/2+2/1 | Grose St - E Left Ahead | U | 202 | 45.1: 45.1\% | 44.6 | 5.5 |
| 2/3 | Grose St - E Right Ahead | 0 | 57 | 27.3\% | 57.4 | 1.9 |
| 3/2+3/1 | O'Connell St - S Ahead Left | U | 362 | 32.9 : 33.1\% | 10.7 | 5.3 |
| 3/3+3/4 | O'Connell St - S Ahead Right | U+O | 978 | 85.2 : 85.1\% | 20.9 | 13.3 |
| 4/2+4/1 | Grose St - W Left Ahead | U | 111 | 29.5 : 29.5\% | 44.7 | 2.9 |
| 4/3 | Grose St - W Ahead Right | 0 | 103 | 71.9\% | 92.3 | 4.5 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J3: O'Connell St/Victoria Rd | - | - | - | 84.3\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 504 | 73.5 : 73.5\% | 37.4 | 16.7 |
| 1/3+1/4 | O'Connell St - N Ahead Right | U+O | 498 | 73.2 : 0.0\% | 39.5 | 16.9 |


| GTA Basic Results Sumpary |
| :--- |
| 2/1 |
| $2 / 2$ |
| $3 / 2+3 / 1$ |
| $3 / 4+3 / 3$ |



Scenario 6: 'FUT-SAT OPT' (FG3: 'Future SAT + Dev (Sensitivity)', Plan 3: 'Existing SAT') Network Layout Diagram


GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - Victoria Road | - | - | - | 95.4\% | - | - |
| J1: O'Connell St/Albert St | - | - | - | 44.8\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 300 | 32.5:32.5\% | 19.8 | 6.0 |
| 1/3 | O'Connell St - N Ahead | U | 325 | 35.2\% | 20.3 | 6.6 |
| 2/1 | Albert St - E Left | U | 227 | 41.7\% | 35.4 | 5.3 |
| 2/2 | Albert St - E Right Ahead | 0 | 179 | 41.9\% | 42.7 | 5.2 |
| 3/2+3/1 | O'Connell St - S Ahead Left | U | 124 | 26.7 : $26.7 \%$ | 11.4 | 0.7 |
| 3/3+3/4 | O'Connell St - S Ahead Right | U+O | 591 | 44.8:44.8\% | 11.5 | 4.3 |
| 4/2+4/1 | Albert St - W Left Ahead Right | O+U | 147 | 35.2:35.2\% | 40.1 | 4.0 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| J2: O'Connell St/Grose St | - | - | - | 51.1\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 453 | 37.4:37.4\% | 8.9 | 5.5 |
| 1/3+1/4 | O'Connell St - N Ahead Right | U+O | 460 | 37.2:37.2\% | 8.2 | 4.6 |
| 2/2+2/1 | Grose St - E Left Ahead | U | 128 | 35.2 : 35.2\% | 49.4 | 3.7 |
| 2/3 | Grose St - E Right Ahead | 0 | 29 | 16.4\% | 59.1 | 0.9 |
| 3/2+3/1 | O'Connell St - S Ahead Left | U | 188 | 15.6: 15.6\% | 7.1 | 1.6 |
| 3/3+3/4 | O'Connell St - S Ahead Right | U+O | 647 | 51.1:51.1\% | 10.3 | 7.1 |
| 4/2+4/1 | Grose St - W Left Ahead | U | 43 | 16.4:16.4\% | 51.1 | 0.8 |
| 4/3 | Grose St - W Ahead Right | 0 | 42 | 41.3\% | 79.1 | 1.6 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J3: O'Connell St/Victoria Rd | - | - | - | 71.2\% | - | - |
| 1/2+1/1 | O'Connell St - N Left Ahead | U | 500 | 65.5 : 65.5\% | 28.8 | 15.5 |
| 1/3+1/4 | O'Connell St - N Ahead Right | U+O | 512 | 67.2:67.2\% | 28.6 | 16.1 |


| 2/1 | Victoria Rd-E Left | U | 443 | 51.4\% | 22.4 | 14.9 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2/2 | Victoria Rd - E Right Ahead | U | 138 | 57.0\% | 48.2 | 4.7 |
| $3 / 2+3 / 1$ | O'Connell St - S Ahead Left | U | 695 | 50.5:50.5\% | 8.0 | 9.2 |
| $3 / 4+3 / 3$ | O'Connell St - S Ahead Right | U | 378 | 71.2: 71.2\% | 49.0 | 12.3 |
| 4/2+4/1 | Victoria Rd - W Left Ahead | U | 27 | 10.0: 10.0\% | 52.4 | 0.5 |
| 4/3 | Victoria Rd - W Ahead | U | 8 | 3.3\% | 52.7 | 0.2 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J4: Marist PI/Victoria Rd | - | - | - | 60.0\% | - | - |
| 1/2+1/1 | Villiers St - N Left Ahead | U | 85 | 42.7 : 42.7\% | 65.4 | 2.4 |
| 1/3 | Villiers St - N Ahead Right | U | 65 | 35.8\% | 65.3 | 2.3 |
| $2 / 2+2 / 1$ | Victoria Rd - E Left Ahead | U | 563 | 60.0: 60.0\% | 16.6 | 6.0 |
| 2/3 | Victoria Rd - E Right Ahead | 0 | 179 | 32.2\% | 28.3 | 4.3 |
| $3 / 2+3 / 1$ | Marist PI - S Ahead Left | U | 245 | 57.2 : 57.2\% | 49.7 | 7.6 |
| 3/3 | Marist PI - S Ahead Right | U | 244 | 57.6\% | 50.2 | 7.8 |
| $4 / 2+4 / 1$ | Victoria Rd - W Left Ahead | U | 366 | 39.6 : 39.6\% | 31.8 | 11.6 |
| 4/3 | Victoria Rd - W Ahead Right | 0 | 127 | 39.2\% | 31.0 | 3.2 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J5: Church St/Victoria Rd | - | - | - | 95.4\% | - | - |
| 1/2+1/1 | Church St - N Left | U | 426 | 26.5:26.5\% | 14.1 | 3.3 |
| 1/3+1/4 | Church St - N Ahead Right | U | 476 | 78.0 : 95.4\% | 64.4 | 11.4 |
| 2/2+2/1 | Victoria Rd - E Left Ahead | U | 511 | 58.8 : 58.8\% | 7.4 | 9.7 |
| 2/3+2/4 | Victoria Rd - E Right Ahead | U | 568 | 75.2 : 75.2\% | 36.3 | 22.4 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 42 | 9.2 : 9.2\% | 39.0 | 0.7 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U | 225 | 53.4 : 53.4\% | 51.6 | 4.1 |



GTA Basic Results Summary GTA Basic Results Summary

User and Project Details

| Project: | 14S1091200 PNUR - Rezoning |
| :--- | :--- |
| Title: | Parramatta North Modelling - North |
| File name: | 141008Ing_Parramatta North modelling_North_FUT-AM_upgrade.Isg3x |
| Company: | GTA Consultants Sydney |
| Address: | Lv6, 15 Help Street CHATSWOOD NSW 2067 |

GTA Basic Results Summary
Scenario 3: 'Fut-AM OPT Upgrade' (FG1: 'Future AM + Dev (Sensitivity)', Plan 1: 'Existing - AM') Network Layout Diagram


GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - North | - | - | - | 110.8\% | - | - |
| J1: Windsor Rd/James Rules Dr | - | - | - | 110.8\% | - | - |
| 1/1 | Windsor Rd - N Left | U | 1417 | 71.9\% | 5.3 | 16.6 |
| 1/3+1/2 | Windsor Rd - N Ahead | U | 657 | 95.6 : 95.6\% | 80.4 | 30.6 |
| 1/4+1/5 | Windsor Rd - N Ahead Right | U | 942 | 99.5 : 107.4\% | 131.2 | 46.1 |
| $2 / 2+2 / 1$ | James Rule Dr (off ramp) - E Right Left | U | 347 | 108.8 : 108.8\% | 260.8 | 34.0 |
| 2/3 | James Rule Dr (off ramp) - E Right | U | 197 | 104.3\% | 233.9 | 17.0 |
| 3/1+3/2 | Church St - S Ahead Left | U | 671 | 41.5 : 41.5\% | 2.3 | 0.7 |
| 3/3 | Church St - S Ahead | U | 537 | 102.5\% | 149.5 | 35.8 |
| $3 / 4+3 / 5$ | Church St - S Ahead Right | U | 113 | 0.0 : 105.2\% | 292.8 | 11.3 |
| 4/2+4/1 | Briens Rd (off ramp) - W Left Right | U | 342 | 110.8: 110.8\% | 281.0 | 31.3 |
| 4/3 | Briens Rd (off ramp) - W Right | U | 356 | 110.4\% | 287.3 | 35.5 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| Ped Link: P5 | P6 | - | 0 | 0.0\% | - | - |
| Ped Link: P6 | P7 | - | 0 | 0.0\% | - | - |
| Ped Link: P7 | P8 | - | 0 | 0.0\% | - | - |
| Ped Link: P8 | P5 | - | 0 | 0.0\% | - | - |
| J2: Windsor Rd/The Junction | - - | - | - | 80.1\% | - | - |
| 1/1 | Windsor Rd (N) Ahead | U | 18 | 1.2\% | 1.3 | 0.0 |
| 1/2 | Windsor Rd (N) Ahead | 0 | 1182 | 80.1\% | 9.6 | 18.9 |
| 1/3+1/4 | Windsor Rd (N) Ahead Right | U | 832 | 54.5 : 55.6\% | 3.3 | 1.6 |
| 2/2+2/1 | Windsor Rd-S Ahead Left | U+O | 708 | 53.3 : 53.3\% | 5.6 | 8.8 |
| 2/3 | Windsor Rd-S Ahead | U | 530 | 40.3\% | 4.1 | 4.0 |
| 2/4 | Windsor Rd-S Ahead | U | 113 | 8.6\% | 3.1 | 0.5 |
| $3 / 2+3 / 1$ | The Junction Access - W Left Right | U | 139 | 69.3 : 69.3\% | 85.1 | 5.4 |


| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| J3: Church St/North Rocks Rd | - | - | - | 97.8\% | - | - |
| 1/1 | Windsor Rd-N Left | U | 445 | 35.3\% | 6.1 | 3.5 |
| 1/2 | Windsor Rd-N Ahead | U | 18 | 2.1\% | 4.3 | 0.1 |
| 1/3 | Windsor Rd - N Ahead | U | 828 | 93.6\% | 47.6 | 33.0 |
| 1/4 | Windsor Rd - N Ahead | U | 833 | 93.5\% | 49.4 | 31.2 |
| $2 / 2+2 / 1$ | North Rocks Rd (E) Right Left | U | 569 | 97.8: 97.8\% | 96.5 | 24.9 |
| 2/3 | North Rocks Rd (E) Right | U | 256 | 90.8\% | 105.7 | 13.0 |
| 3/1 | Church St - S Ahead | U | 15 | 1.2\% | 8.2 | 0.1 |
| 3/2 | Church St - S Ahead | U | 474 | 37.1\% | 7.6 | 5.4 |
| 3/3 | Church St - S Ahead | U | 533 | 41.8\% | 6.9 | 5.3 |
| $3 / 5+3 / 4$ | Church St - S Ahead Right | U | 248 | 97.2 : 0.0\% | 123.1 | 13.5 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J4: Church St/Board St/Seville St | - | - | - | 56.2\% | - | - |
| 1/1 | Church St - N Left Ahead | U | 263 | 14.3\% | 1.2 | 0.1 |
| 1/2 | Church St - N Ahead | U | 786 | 42.8\% | 1.7 | 0.4 |
| 1/3 | Church St - N Ahead | U | 975 | 52.5\% | 2.7 | 1.5 |
| 2/1 | Seville St - E Left | O | 3 | 0.9\% | 5.4 | 0.0 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 29 | 2.8 : 2.8\% | 3.6 | 0.1 |
| 3/3 | Church St - S Ahead | U | 452 | 44.9\% | 10.5 | 5.1 |
| 3/4 | Church St - S Ahead | U | 446 | 44.3\% | 10.1 | 4.7 |
| 4/1 | Board St - W Left | U | 355 | 56.2\% | 41.7 | 11.3 |
| Ped Link: P1 | p1 | - | 0 | 0.0\% | - | - |
| J5: Church St/Barney St | - | - | - | 85.3\% | - | - |
| 1/2+1/1 | Church St - N Left Ahead | U | 97 | 7.6 : 7.4\% | 10.2 | 1.0 |
| 1/3 | Church St - N Ahead | U | 1066 | 78.1\% | 18.5 | 17.2 |
| 1/4+1/5 | Church St - N Right | U | 843 | 85.3 : 85.1\% | 24.3 | 9.7 |



GTA Basic Results Summary GTA Basic Results Summary

User and Project Details

| Project: | 14S1091200 PNUR - Rezoning |
| :--- | :--- |
| Title: | Parramatta North Modelling - North |
| File name: | 141008 Ing_Parramatta North <br> modelling_North_FUT-PM-SAT_upgrade.Isg3x |
| Company: | GTA Consultants Sydney |
| Address: | Lv6, 15 Help Street CHATSWOOD NSW 2067 |

## Scenario 1: 'Future PM OPT upgrade' (FG1: 'Future PM + Dev (Sensitivity)', Plan 2: 'Existing - PM') Network Layout Diagram



GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - North | - | - | - | 229.5\% | - | - |
| J1: Windsor Rd/James Rules Dr | - | - | - | 229.5\% | - | - |
| 1/1 | Windsor Rd - N Left | 0 | 817 | 50.2\% | 3.3 | 5.7 |
| 1/3+1/2 | Windsor Rd - N Ahead | U | 893 | 97.6:97.6\% | 77.5 | 35.2 |
| 1/4+1/5 | Windsor Rd - N Ahead Right | U | 185 | 0.0 : 229.5\% | 1218.7 | 66.0 |
| 2/2+2/1 | James Rule Dr (off ramp) - E Right Left | U | 390 | 106.6: 106.6\% | 224.6 | 34.5 |
| 2/3 | James Rule Dr (off ramp) - E Right | U | 381 | 105.0\% | 206.6 | 30.2 |
| 3/1+3/2 | Church St - S Ahead Left | O+U | 859 | 52.3 : 52.3\% | 2.8 | 0.9 |
| 3/3 | Church St - S Ahead | U | 786 | 99.4\% | 91.2 | 41.8 |
| $3 / 4+3 / 5$ | Church St - S Ahead Right | U | 805 | 98.3 : 100.5\% | 77.4 | 39.6 |
| 4/1+4/2 | Briens Rd (off ramp) - W Left Right | U | 561 | 96.5 : 136.8\% | 350.3 | 56.6 |
| 4/3 | Briens Rd (off ramp) - W Right | U | 352 | 137.9\% | 624.2 | 66.8 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| Ped Link: P5 | P6 | - | 0 | 0.0\% | - | - |
| Ped Link: P6 | P7 | - | 0 | 0.0\% | - | - |
| Ped Link: P7 | P8 | - | 0 | 0.0\% | - | - |
| Ped Link: P8 | P4 | - | 0 | 0.0\% | - | - |
| J2: Windsor Rd/The Junction | - | - | - | 74.9\% | - | - |
| 1/1 | Windsor Rd ( N ) Ahead | U | 678 | 44.7\% | 3.1 | 4.5 |
| 1/2 | Windsor Rd (N) Ahead | U | 602 | 37.0\% | 2.6 | 0.9 |
| 1/3+1/4 | Windsor Rd ( N ) Ahead Right | U | 299 | 55.4:61.0\% | 19.9 | 2.7 |
| 2/2+2/1 | Windsor Rd - S Ahead Left | U+O | 944 | 74.9 : 74.9\% | 10.7 | 9.4 |
| 2/3 | Windsor Rd - S Ahead | U | 690 | 55.8\% | 6.7 | 5.0 |
| 2/4 | Windsor Rd - S Ahead | U | 805 | 65.1\% | 11.0 | 12.0 |
| 3/2+3/1 | The Junction Access - W Left Right | U | 300 | 70.5 : 70.5\% | 64.5 | 6.9 |



GTA Basic Results Summary

| 2/1+2/2 | Barney St - E Right Left Ahead |  | U | 227 | 86.4 : 86.4\% | 97.6 |  | 10.8 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2/3 | Barney St - E Right |  | U | 268 | 90.7\% | 106.2 |  | 13.6 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left |  | U | 125 | 18.1 : 18.1\% | 30.5 |  | 3.0 |
| $3 / 3+3 / 4$ | Church St - S Ahead |  | U | 1066 | 89.6 : 89.5\% | 50.4 |  | 26.8 |
| 4/2+4/1 | Barney St - W Left Ahead |  | U | 425 | 76.8:76.8\% | 56.1 |  | 15.0 |
| Ped Link: P1 | P1 |  | - | 0 | 0.0\% | - |  | - |
| Ped Link: P2 | P2 |  | - | 0 | 0.0\% | - |  | - |
| Ped Link: P3 | P3 |  | - | 0 | 0.0\% | - |  | - |
| C1-TCS704 - Windsor Rd/Briens Rd C2 - TCS 3704 - Windsor Rd/The Junction Access C3 - TCS464 - North Rocks Rd/Church St C4 - TCS1085-Church St/Barney St C5 - Church St/Seville St | PRC for Signalled Lanes (\%): PRC for Signalled Lanes (\%): PRC for Signalled Lanes (\%): PRC for Signalled Lanes (\%): PRC for Signalled Lanes (\%): PRC Over All Lanes (\%): | $\begin{array}{r} -155.0 \\ 20.2 \\ 15.4 \\ -6.2 \\ 7.5 \\ -155.0 \end{array}$ |  | r Signal <br> Signal <br> r Signal <br> r Signall <br> Signal <br> lay Over | (pcuHr): 282 <br> pcuHr): 14 <br> pcuHr): 32 <br> pcuHr): 46 <br> pcuHr): 17 <br> pcuHr): 393 | Cycle Time (s): Cycle Time (s): Cycle Time (s): Cycle Time (s): Cycle Time (s): | $\begin{aligned} & 134 \\ & 134 \\ & 134 \\ & 134 \\ & 134 \end{aligned}$ |  |

GTA Basic Results Summary
Scenario 2: 'Future SAT OPT upgrade' (FG2: 'Future SAT + Dev (Sensitivity)', Plan 3: 'Existing - SAT') Network Layout Diagram


GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - North | - | - | - | 160.2\% | - | - |
| J1: Windsor Rd/James Rules Dr | - | - | - | 160.2\% | - | - |
| 1/1 | Windsor Rd - N Left | 0 | 954 | 59.1\% | 4.1 | 8.1 |
| 1/3+1/2 | Windsor Rd - N Ahead | U | 481 | 83.9 : 83.9\% | 63.9 | 14.8 |
| 1/4+1/5 | Windsor Rd - N Ahead Right | U | 633 | 90.6 : 160.2\% | 368.6 | 64.6 |
| 2/2+2/1 | James Rule Dr (off ramp) - E Right Left | U | 433 | 101.3: 101.3\% | 148.4 | 27.5 |
| 2/3 | James Rule Dr (off ramp) - E Right | U | 406 | 97.7\% | 120.6 | 22.5 |
| 3/1+3/2 | Church St - S Ahead Left | O+U | 773 | 47.7 : 47.7\% | 2.3 | 0.7 |
| 3/3 | Church St - S Ahead | U | 508 | 106.5\% | 215.6 | 40.9 |
| $3 / 4+3 / 5$ | Church St - S Ahead Right | U | 503 | 88.3: 85.0\% | 64.3 | 12.2 |
| 4/1+4/2 | Briens Rd (off ramp) - W Left Right | U | 604 | 59.3 : 99.8\% | 45.9 | 13.6 |
| 4/3 | Briens Rd (off ramp) - W Right | U | 395 | 98.4\% | 127.2 | 22.6 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| Ped Link: P5 | P6 | - | 0 | 0.0\% | - | - |
| Ped Link: P6 | P7 | - | 0 | 0.0\% | - | - |
| Ped Link: P7 | P8 | - | 0 | 0.0\% | - | - |
| Ped Link: P8 | P4 | - | 0 | 0.0\% | - | - |
| J2: Windsor Rd/The Junction | - | - | - | 70.8\% | - | - |
| 1/1 | Windsor Rd ( N ) Ahead | U | 847 | 58.7\% | 5.5 | 11.4 |
| 1/2 | Windsor Rd (N) Ahead | U | 733 | 50.9\% | 2.7 | 0.8 |
| 1/3+1/4 | Windsor Rd ( N ) Ahead Right | U | 149 | 0.0 : 67.2\% | 69.6 | 6.3 |
| 2/2+2/1 | Windsor Rd - S Ahead Left | U+O | 823 | 70.8 : 70.8\% | 12.0 | 11.0 |
| 2/3 | Windsor Rd - S Ahead | U | 425 | 37.4\% | 9.4 | 4.1 |
| 2/4 | Windsor Rd - S Ahead | U | 503 | 44.3\% | 9.7 | 8.8 |
| 3/2+3/1 | The Junction Access - W Left Right | U | 278 | 65.5 : 65.5\% | 57.7 | 5.3 |



GTA Basic Results Summary

| 2/1+2/2 | Barney St - E Right Left Ahead | U | 134 | 67.6:67.6\% | 80.2 | 5.1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2/3 | Barney St - E Right | U | 153 | 73.7\% | 87.0 | 6.6 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 86 | 16.7 : 16.7\% | 38.9 | 2.3 |
| $3 / 3+3 / 4$ | Church St - S Ahead | U | 765 | 83.1 : 84.5\% | 53.0 | 18.2 |
| 4/2+4/1 | Barney St - W Left Ahead | U | 173 | 20.6 : 20.6\% | 25.4 | 3.4 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| C1-TCS704 - Windsor Rd/Briens Rd C2 - TCS 3704 - Windsor Rd/The Junction Access C3 - TCS464 - North Rocks Rd/Church St C4-TCS1085-Church St/Barney St C5 - Church St/Seville St | PRC for Signalled Lanes (\%): -78.0 <br> PRC for Signalled Lanes (\%): 27.1 <br> PRC for Signalled Lanes (\%): 2.1 <br> PRC for Signalled Lanes (\%): 5.3 <br> PRC for Signalled Lanes (\%): 41.1 <br> PRC Over All Lanes (\%): -78.0 |  | Total Delay for Signalled Lanes Total Delay for Signalled Lanes Total Delay for Signalled Lanes Total Delay for Signalled Lanes Total Delay for Signalled Lanes Total Delay Over All Lanes | (pcuHr): 167.47 <br> (pcuHr): 14.38 <br> (pcuHr): 30.61 <br> (pcuHr): 29.62 <br> pcuHr): 7.11 <br> pcuHr): 249.88 | Cycle Time (s): 130 <br> Cycle Time (s): 130 <br> Cycle Time (s): 130 <br> Cycle Time (s): 130 <br> Cycle Time (s): 130 |  |

GTA Basic Results Summary GTA Basic Results Summary

User and Project Details

| Project: | 14S1091200 PNUR - Rezoning |
| :--- | :--- |
| Title: | Parramatta North Modelling - PHR |
| File name: | 141001Ing_Parramatta North modelling_PHR_FUT-AM_upgrade.Isg3x |
| Company: | GTA Consultants Sydney |
| Address: | Lv6, 15 Help Street CHATSWOOD NSW 2067 |

Scenario 1: 'FUT-AM OPT upgrade' (FG1: 'Future AM + Dev (Sensitivity)', Plan 1: 'Network Control Plan 1') Network Layout Diagram


GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - PHR | - | - | - | 97.6\% | - | - |
| J1: Church St/Factory St | - | - | - | 82.7\% | - | - |
| 1/2+1/1 | Church St - N Left Ahead | U | 539 | 60.8:60.8\% | 28.0 | 14.1 |
| 1/3+1/4 | Church St - N Ahead Right | U | 574 | 61.9: 61.9\% | 30.2 | 14.1 |
| $2 / 2+2 / 1$ | Factory St -E Left Ahead | U | 39 | 23.9:23.9\% | 65.1 | 0.9 |
| 2/3 | Factory St -E Right Ahead | O | 16 | 11.2\% | 67.4 | 0.6 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 40 | 4.5 : 4.5\% | 20.5 | 0.7 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U+O | 530 | 58.4 : 58.4\% | 24.4 | 7.6 |
| 4/1+4/2 | Factory St - W Left Ahead | U | 120 | 17.6: 17.6\% | 28.9 | 2.8 |
| 4/3 | Factory St - W Right | U | 288 | 82.7\% | 75.8 | 11.7 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J2: Church St/Pennant Hills Rd | - | - | - | 97.6\% | - | - |
| 1/2+1/1 | Church St - N U-Turn Ahead | U+O | 837 | 74.7 : 74.7\% | 17.7 | 8.8 |
| 1/3 | Church St - N Ahead | U | 532 | 64.3\% | 23.8 | 8.1 |
| 2/1 | Pennant Hills Rd-NE Ahead | U | 493 | 75.5\% | 43.9 | 13.3 |
| 2/2 | Pennant Hills Rd-NE Right | U | 454 | 72.7\% | 45.8 | 14.9 |
| 3/1 | Albert St - E Left | O | 37 | 13.5\% | 7.6 | 0.1 |
| 4/2+4/1 | Church St - S Ahead Left | U | 33 | 3.3 : 3.3\% | 13.2 | 0.4 |
| 4/3+4/4 | Church St - S Ahead Right | U | 700 | 62.4:97.6\% | 24.9 | 15.1 |
| 5/2+5/1 | Albert St - W Left Left2 | U | 188 | 30.0:30.0\% | 33.6 | 4.9 |
| 5/3 | Albert St - W Left | U | 185 | 29.6\% | 33.6 | 4.8 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |

GTA Basic Results Summary

| J3: Church St/Grose St | - | - | - | 71.3\% | - |  | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1/2+1/1 | Church St - N Left Ahead | U | 687 | 60.6:60.6\% | 11.8 |  | 9.4 |
| 1/3+1/4 | Church St - N Ahead Right | U+O | 974 | 71.3: 71.3\% | 11.4 |  | 13.6 |
| $2 / 2+2 / 1$ | Grose St - E Left Ahead | U | 199 | 59.1 : 59.1\% | 58.3 |  | 6.9 |
| 2/3 | Grose St - E Right Ahead | 0 | 59 | 62.2\% | 107.0 |  | 2.5 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 91 | 8.2 : 8.2\% | 11.3 |  | 0.7 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U+O | 626 | 54.6:54.6\% | 16.3 |  | 11.3 |
| 4/2+4/1 | Grose St - W Left Ahead | U | 238 | 70.0 : 70.0\% | 62.9 |  | 8.5 |
| 4/3 | Grose St - W Ahead Right | 0 | 57 | 40.6\% | 71.7 |  | 2.2 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P2 | 2 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P3 | 3 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P4 | 4 | - | 0 | 0.0\% | - |  | - |
| C1 - Church St/Factory St C2 - Church St/Pennant Hills Rd C3 - Church St/Grose St | $\begin{aligned} & \text { PRC for Signalled Lanes (\%): } \\ & \text { PRC for Signalled Lanes (\%): } \\ & \text { PRC for Signalled Lanes (\%): } \\ & \text { PRC Over All Lanes (\%): } \end{aligned}$ | $\begin{array}{r} 8.9 \\ -8.4 \\ 26.2 \\ -8.4 \end{array}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  | $\begin{aligned} & 20.86 \\ & 27.86 \\ & 18.73 \\ & 67.53 \end{aligned}$ | Cycle Time (s): 124 <br> Cycle Time (s): 124 <br> Cycle Time (s): 124 |  |

GTA Basic Results Summary GTA Basic Results Summary

User and Project Details

| Project: | 14S1091200 PNUR - Rezoning |
| :--- | :--- |
| Title: | Parramatta North Modelling - PHR |
| File name: | 141001Ing_Parramatta North modelling_PHR_FUT-PM_upgrade.Isg3x |
| Company: | GTA Consultants Sydney |
| Address: | Lv6, 15 Help Street CHATSWOOD NSW 2067 |

Scenario 1: 'FUT-PM OPT upgrade' (FG2: 'Future PM + Dev (Sensitivity)', Plan 1: 'Network Control Plan 1') Network Layout Diagram


GTA Basic Results Summary

## Network Results

| Item | Lane Description | Lane Type | Demand Flow (pcu) | Deg Sat (\%) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network: Parramatta North Modelling - PHR | - | - | - | 98.6\% | - | - |
| J1: Church St/Factory St | - | - | - | 86.7\% | - | - |
| 1/2+1/1 | Church St - N Left Ahead | U | 42 | 4.2 : 4.2\% | 11.4 | 0.4 |
| 1/3+1/4 | Church St - N Ahead Right | U | 776 | 72.1 : 86.7\% | 28.0 | 15.6 |
| $2 / 2+2 / 1$ | Factory St -E Left Ahead | U | 34 | 16.9 : 16.9\% | 50.8 | 0.6 |
| 2/3 | Factory St -E Right Ahead | O | 20 | 11.8\% | 56.3 | 0.6 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 553 | 56.1 : 56.1\% | 9.6 | 9.8 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U+O | 580 | 57.0 : 57.0\% | 9.0 | 7.7 |
| 4/1+4/2 | Factory St - W Left Ahead | U | 79 | 14.7 : 14.7\% | 31.9 | 1.6 |
| 4/3 | Factory St - W Right | U | 80 | 67.3\% | 92.5 | 3.3 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |
| J2: Church St/Pennant Hills Rd | - | - | - | 98.6\% | - | - |
| 1/2+1/1 | Church St - N U-Turn Ahead | U+O | 32 | 4.5 : 0.0\% | 17.3 | 0.6 |
| 1/4+1/3 | Church St - N U-Turn Ahead | U+O | 770 | 94.9 : 94.9\% | 62.5 | 26.3 |
| 2/1 | Pennant Hills Rd-NE Ahead | U | 315 | 44.2\% | 22.5 | 6.0 |
| 2/2 | Pennant Hills Rd-NE Right | U | 587 | 96.0\% | 80.1 | 24.3 |
| 3/1 | Albert St - E Left | O | 25 | 8.7\% | 6.9 | 0.1 |
| 4/2+4/1 | Church St - S Ahead Left | U | 651 | 66.1 : 66.1\% | 16.0 | 7.7 |
| 4/3+4/4 | Church St - S Ahead Right | U | 806 | 62.1:98.6\% | 26.6 | 24.5 |
| 5/2+5/1 | Albert St - W Left Left2 | U | 175 | 28.4 : 28.4\% | 29.7 | 3.8 |
| 5/3 | Albert St - W Left | U | 168 | 27.5\% | 29.6 | 3.8 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - | - |
| Ped Link: P2 | P2 | - | 0 | 0.0\% | - | - |
| Ped Link: P3 | P3 | - | 0 | 0.0\% | - | - |
| Ped Link: P4 | P4 | - | 0 | 0.0\% | - | - |

GTA Basic Results Summary

| J3: Church St/Grose St | - | - | - | 81.0\% | - |  | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1/2+1/1 | Church St - N Left Ahead | U | 84 | 8.4 : 8.4\% | 10.3 |  | 0.5 |
| 1/3+1/4 | Church St - N Ahead Right | U+O | 915 | 81.0:81.0\% | 23.2 |  | 9.9 |
| $2 / 2+2 / 1$ | Grose St - E Left Ahead | U | 162 | 40.6 : 40.6\% | 41.6 |  | 4.2 |
| 2/3 | Grose St - E Right Ahead | 0 | 97 | 75.2\% | 100.4 |  | 4.1 |
| $3 / 2+3 / 1$ | Church St - S Ahead Left | U | 615 | 60.3 : 60.3\% | 19.5 |  | 12.5 |
| $3 / 3+3 / 4$ | Church St - S Ahead Right | U+O | 705 | 63.2 : 63.2\% | 19.4 |  | 13.2 |
| 4/2+4/1 | Grose St - W Left Ahead | U | 266 | 63.8 : 63.8\% | 46.0 |  | 7.2 |
| 4/3 | Grose St - W Ahead Right | 0 | 97 | 40.3\% | 51.3 |  | 2.9 |
| Ped Link: P1 | P1 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P2 | 2 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P3 | 3 | - | 0 | 0.0\% | - |  | - |
| Ped Link: P4 | 4 | - | 0 | 0.0\% | - |  | - |
| C1 - Church St/Factory St C2 - Church St/Pennant Hills Rd C3 - Church St/Grose St | $\begin{aligned} & \text { PRC for Signalled Lanes (\%): } \\ & \text { PRC for Signalled Lanes (\%): } \\ & \text { PRC for Signalled Lanes (\%): } \\ & \text { PRC Over All Lanes (\%): } \end{aligned}$ | $\begin{array}{r} 3.9 \\ -9.6 \\ 11.1 \\ -9.6 \end{array}$ | Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): |  | $\begin{aligned} & 12.65 \\ & 40.24 \\ & 22.63 \\ & 75.57 \end{aligned}$ | Cycle Time (s): 106 <br> Cycle Time (s): 106 <br> Cycle Time (s): 106 |  |

## Melboume

A Level 25, 55 Collins Stree
PO Box 24055
MELBOURNE VIC 3000
P +61398519600
F +613 98519610
E melboume@gta.com.au

## Sydney

A Level 6, 15 Help Street CHATSWOOD NSW 2067 PO Box 5254 WESTCHATSWOOD NSW 1515

P +612 84481800
F +612 84481810
E sydney@gta.com.au

## Brisbane

A Level 4, 283 Eliza beth Street BRISBANE QLD 4000
GPO Box 115
BRISBANE QமD 4001
P +61731135000
F +617 31135010
E brisbane@gta.com.au

## Canbera

## Townsville

A Unit 4, Level 1, Sparta Building, 55 Woolley Street A Level 1, 25 Sturt Street
PO Box 62
PO Box 1064
TOWNSVIயF OLD 4810
P +61747222765
F +617 47222761
E townsville@gta.com.au

DICKSON ACT 2602
P +612 62434826
F +612 62434848
E canberra@gta.com.au

## Adelaide

A Suite 4, Level 1, 136 The Parade
PO Box 3421
NORWOOD SA 5067
P +618 83343600
F +618 83343610
E adelaide@gta.com.au

## Gold Coast

A Level 9, Comorate Centre 2
Box 37
1 Corporate Court
BUNDAL QLD 4217
P +6175510 4800
F +61755104814
E goldcoast@gta.com.au

GTAconsu


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